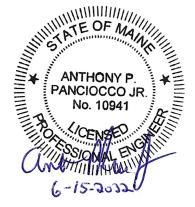
RAILROAD SQUARE RE-DEVELOPMENT

YARMOUTH, MAINE

STORMWATER MANAGEMENT REPORT



June 2022

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INTRODUCTION AND BACKGROUND

Atlantic Resource Consultants (ARC) has prepared the following stormwater management analysis for the redevelopment of Railroad Square. The project site encompasses 4.51 acres of land and is located off the southerly side of Main Street in Yarmouth. Access to the project site is provided off Main Street. The parcel is identified as Lots 29-00A and 30 on the on the Town of Yarmouth tax map 37. The majority of the parcel is developed with buildings, pavement areas, gravel areas, and lawn areas. The southern portion of the site is wooded and contains wetland areas and a stream.

The proposed redevelopment will include the construction a mixed-use neighborhood of residential, commercial and community uses and associated paving, landscaping, utilities, and stormwater management facilities.

Site History

The project site has been developed since at least the 1950's. Throughout the years it has served as a support lot for the adjacent rail station, an asphalt batching plant, Bickford Transportation facility, and is currently utilized by some small businesses and a display area for antique construction machinery. Based on the amount of redevelopment on the site it is anticipated that the project will require a Stormwater Permit from the MDEP. There is currently a VRAP No Action Assurance Letter associated with the project site.

Stormwater Management

Stormwater runoff from the central/northern portion of the site is collected in a shared drainage system with the property to the west (Downeast Energy property). Runoff from portions of both properties is conveyed via the existing catch basin system to the storm drainage system located in Main Street. The Main Street system conveys runoff northerly to the Royal River and on to the ocean. The southern portion of the site drains southerly to an existing railroad culvert, which conveys runoff easterly under the railroad tracks. Runoff eventually re-enters the Town drainage system and is conveyed to the Royal River. There are currently no stormwater quality or quantity control BMPs on the project site.

New stormwater Best Management Practices BMPs have been designed to capture and treat runoff from the new improvements associated with this project. These include roof drip line filters, underdrained pervious paver areas with sand filters, a subsurface sand filter with chambers, and a surface underdrained filtration basin. The new BMP's will provide detention and water quality treatment for runoff from new impervious and developed areas associated with new development.

The new stormwater management system will maintain the existing drainage patterns at the site, while protecting water quality and ensuring that there is no increase in peak runoff from the property during design storm conditions. This stormwater management analysis has been prepared in accordance with the Maine Department of Environmental Protection (MDEP) Chapter 500 Regulations for Basic, General and Flooding



Standards to ensure that the planned development will not result in a degradation of water quality or any other significant impacts to locations downstream of the development site because of stormwater runoff.

EXISTING CONDITIONS

Currently the project site is mostly developed with buildings, pavement areas, gravel areas, and lawn areas. The southern portion of the site is wooded and contains wetland areas and a stream. The project site consists of 2.44 acres of existing impervious surface area and 3.21 acres of existing developed area.

Receiving Waters

Runoff from northern portion of the site drains to the Town drainage system and onto the Royal River. Runoff from the southern portion of the site drains to an existing railroad culvert and onto the Town system and the Royal River. The royal river is tributary to the ocean. The Royal River is not identified as an Urban Impaired Stream watershed by the MDEP.

Historical Flooding

The site is not within Flood Zone as identified on the FIRM map for the Town of Yarmouth, Community Panel Number 230055 0005 B, Panel 5 of 12 dated November 15, 1984.

Alterations to Natural Drainageways

The project will not result in any alteration of natural drainageways. An existing manmade swale will be culverted as part of the project. A field determination from MDEP is included with this application indicating that the drainage way to be culverted is not a stream.

PROJECT DESCRIPTION

The proposed redevelopment will include the construction a mixed-use neighborhood of residential, commercial and community uses and associated paving, landscaping, utilities, and stormwater management facilities. The proposed project will connect to existing public water, sewer, gas, and electrical services to be installed as part of the project.

The redevelopment of the site will result in 2.53 acres of impervious surface area and 4.31 acres of developed area. As indicated in the attached calculations this is only a 0.10 acre increase in the impervious surface area of the site.

As part of the MDEP permitting the project will be required to provide treatment for 60% of the site's developed area. The MDEP re-development calculations are included in this report.

Stormwater Management

The stormwater BMPs proposed to capture and treat runoff from the new developed areas of the site include roof drip line filters, underdrained pervious paver areas with sand filters, a subsurface sand filter with chambers,



and a surface underdrained filtration basin. These BMP's have been sized and designed in accordance with current State of Maine Chapter 500 Stormwater Law. The stormwater BMPs are designed to capture, treat, and detain runoff from impervious and developed areas of the site and will discharge to existing receiving waters via riprap aprons designed to dissipate runoff velocities.

METHODOLOGY AND MODELING ASSUMPTIONS

Runoff and routing calculations have been performed for the watershed areas affected by the proposed development under pre-development and post-development conditions scenarios. Time of concentration and runoff curve number calculations have been performed using the method described in Natural Resource Conservation Service (NRCS) Technical Release 55 (TR-55) – Urban Hydrology for Small Watersheds. The TR-20 based HydroCAD modeling software has been utilized to perform the more complex runoff and routing calculations, some of which are beyond the scope of the TR-55 method. Time of concentration calculations have been amended where the value given by the TR-55 method is less than six minutes (0.1hr). In these cases, a standard minimum value of six minutes has been used to keep this parameter within the acceptable working range of the model and prevent computational errors.

Design rainfall events have been modeled using the SCS Type III Hydrograph for 24-hour duration storms. The rainfall depth for each return period is taken from Maine Department of environmental Protection Chapter 500 Stormwater Management, Appendix H (Cumberland County). The rainfall depth values for standard design storm frequencies are shown in the table below.

	TABLE I - 24-Hr Rainfall Depths for Cumberland County at Design Storm Frequencies						
	Maine Chapter 500: Stormwater Management, Appendix H						
Frequency	2-Year	10-Year	25-Year	100-Year			
Rainfall Depth	3.1 in	4.6 in	5.8 in	8.1 in			

Soil types around the site have been identified using the Natural Resource Conservation Service (NRCS) Web Soil Survey. The existing topography of the site was determined by field survey. and utilizing lidar contours. The existing vegetative cover was identified by site inspection and survey. The following table includes a list of the surficial soil types that were identified within the proposed development area on the project site, along with their associated Hydrologic Soil Group (HSG).

TABLE 2 – SOIL TYPES						
Soil Type Hydrologic Soil Grou						
Lamoine -BuB	C/D					
Elmwood - EmB	В					
Made Land - Md	С					
Suffield – SuC2	С					



Under pre-development conditions the site is represented by ten subcatchments that drain to two Points of Analysis, Study Point SP-I and SP-2. Study Point, SP-I is the location where site runoff, collected in the onsite drainage system, is conveyed into the Main Street system. Study Point, SP-2 represents the location of the railroad culvert where runoff from the site is conveyed offsite.

Under the post-development conditions scenario, the site is divided into twenty smaller subcatchment areas to model the impact of localized storage within the system. The overall routing configuration remains similar to predeveloped conditions. In this way, a direct comparison can be made of pre-development and post-development runoff values at the Design Points.

PROPOSED BMPS

New Stormwater Best Management Practices (BMPs) have been designed to effectively capture, detain, and treat runoff from the new impervious area associated with the new development at the site, before allowing it to discharge in a non-erosive manner to downstream areas.

The new BMP proposed for this project is as follows:

- Filtering Drip Strips Filtering drip edges at the exterior pitched roof segments, collect runoff from the building's roof. A stone layer is provided to store the water quality volume from the roofs. A sand filter media provides treatment for this runoff. Once through the soil media, the runoff is collected in a perforated underdrain pipe system and discharged downstream. The filter structure provides for the slow release of smaller storm events, minimizing stream channel erosion, and cooling of the discharge.
- Subsurface Sand Filter This BMP consists of a stormwater chamber system located over a subsurface sand filter. Stormwater rises in the chambers and is filtered vertically out of the open bottom chambers through the sand filter. Once through the sand filter runoff is collected in a perforated underdrain pipe and discharged downstream. This BMP is designed to accept and treat runoff from both the new impervious area of the site but also runoff from a portion of the new landscaped areas of the site. The filter structure provides for the slow release of smaller storm events, minimizing stream channel erosion, and cooling of the discharge.
- Pervious Paver Parking area. This BMP consists of a pervious paver section located over a stormwater reservoir, consisting of crushed stone, overlying a mineral soil filter layer. This runoff drains through the paver and the soil filter media into an underdrain system which conveys the runoff to the outlet.
- Underdrained Grass Filter The underdrained grass filter is a shallow grassed depression filled with a filtering soil media and planted with native grasses. Runoff directed to the BMPs is detained temporarily and passes slowly through the soil media and the root zone of the planted material before draining into an underdrain system that discharges to an outlet culvert. The soil media and root zone activity provide



water quality treatment for the runoff by removing suspended particles, and through uptake and binding of dissolved pollutants and nutrients.

STORMWATER QUANTITY ANALYSIS

Pre-development Conditions

The overall model for the site is represented by ten subcatchment areas tributary to two points of analysis, SP-Iand SP-2. Full details of the pre-development subcatchment areas, cover conditions and time of concentration flow paths are described in detail in the supporting HydroCAD documentation included in this report. A Predevelopment Conditions Watershed Plan is included in this report.

Post-Development Conditions

In the post-development condition, the overall site is divided into twenty watershed areas and the same two points of analysis, SP-1, and SP-2. Full details of the post-development subcatchment areas, cover conditions and time of concentration flow paths are described in detail in the supporting HydroCAD documentation included in this report. A Post-Development Conditions Watershed Plan is included as part of this report.

Water Quantity Analysis Summary

The table below summarizes the peak runoff values for predevelopment and post-development conditions during each of the analyzed design storm events.

Peak Rates of Runoff												
ΡΟΑ	-	tal rshed ea	•	eighted e No.	2-Y	2-Year 10-Year 25			25-1	/ear	100-	Year
	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post	Pre	Post
SP-1	2.38	2.37	87	87	5.24	5.22	8.23	8.18	10.77	10.70	15.75	15.64
SP-2	4.66	4.67	76	86	5.03	4.93	10.68	7.74	15.58	10.08	25.36	17.10

As indicated in the table above the post development peak rates of runoff are maintained at or below the predevelopment peak rates of runoff for all storm events at Study Points SP-1 and SP-2.



STORMWATER QUALITY ANALYSIS

Water Quality Treatment Measures

The project has been designed in accordance with the current Stormwater Law (Chapter 500) standards for a re-development site. Based on the redevelopment calculations the project will be required to provide stormwater quality treatment for 60% of the project's developed area. As indicated in the stormwater calculations the project will provide treatment for 62.8% of the projects developed area.

The project will result in approximately 2.53 acres of impervious surface area. An increase of 0.10 acres compared to the pre-development condition.

Water quality treatment will be provided by a subsurface sand filter, filtering drip strips, pervious pavers with a sand filter, and a surface underdrained filtration basin designed in accordance with the latest version of the Maine Department of Environmental Protection BMPs Technical Design Manual, to achieve the required stormwater quality treatment percentages.

SOIL EROSION AND SEDIMENT CONTROL

A comprehensive Soil Erosion and Sediment Control (SESC) narrative has been prepared that includes Best Management Practices (BMPs) associated with the proposed construction activities. The location of SESC BMPs is shown on the accompanying plans. These are further described on the details and notes sheets in the accompanying plan set.

The Erosion and Sediment Control plan and details outlines the required construction measures and techniques that will reduce potential degradation of the water quality at downstream locations. Temporary erosion control measures will be incorporated during construction, and long-term surface stabilization practices have been designed as part of the site development, thus minimizing the potential for erosion and sediment transport. These measures include the constructed BMPs for filtration of runoff from smaller storm events, riprap, permanent seeding, and other vegetative stabilization measures. Detailed information on the specific erosion and sedimentation control practices that are to be used on the site are provided on the following plan sheet, which will be included as part of the construction documents for the project.

STORMWATER MAINTENANCE PLAN

A Stormwater Management Inspection and Maintenance Manual has been prepared specifically for the project and is included in this report.

CONCLUSIONS

The stormwater management system designed for this project will mitigate impacts of development on stormwater runoff peak discharge rates and provide treatment of non-point source pollutants in the runoff in accordance with Maine's Stormwater Management Act and Regulations. Based on the analysis described herein, it is expected that runoff from the proposed development will not cause adverse impacts to downstream properties.



Limitations

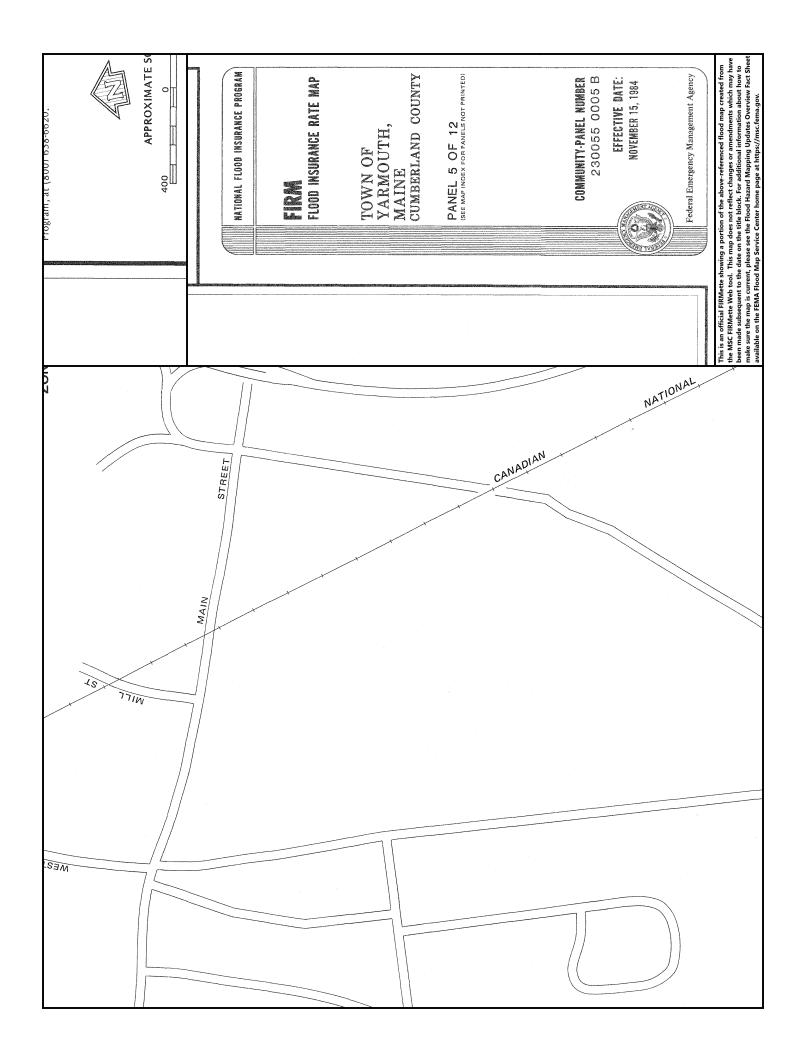
This analysis is based on the information available to the engineer on site conditions and has been conducted using standard industry software designed to analyze comparative changes in land cover conditions. The accuracy of the runoff and routing calculations is limited by the methodology used in the software and the results should be viewed as suitable for comparative studies only.

FIGURES AND ATTACHMENTS

- I. Figure 2 FEMA Flood Map
- 2. Figure 3 USDA SCS Web Soil Survey Map
- 3. Attachment A Stormwater Quality Calculations
- 4. Attachment B TR-20 Computations (HydroCAD)
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- 5. Attachment C Stormwater Operations and Maintenance Manual



FIGURES

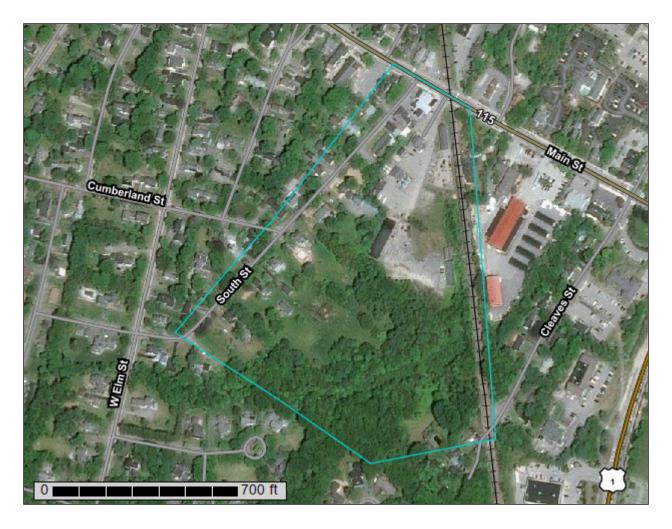




United States Department of Agriculture

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Cumberland County and Part of Oxford County, Maine

Railroad Square



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

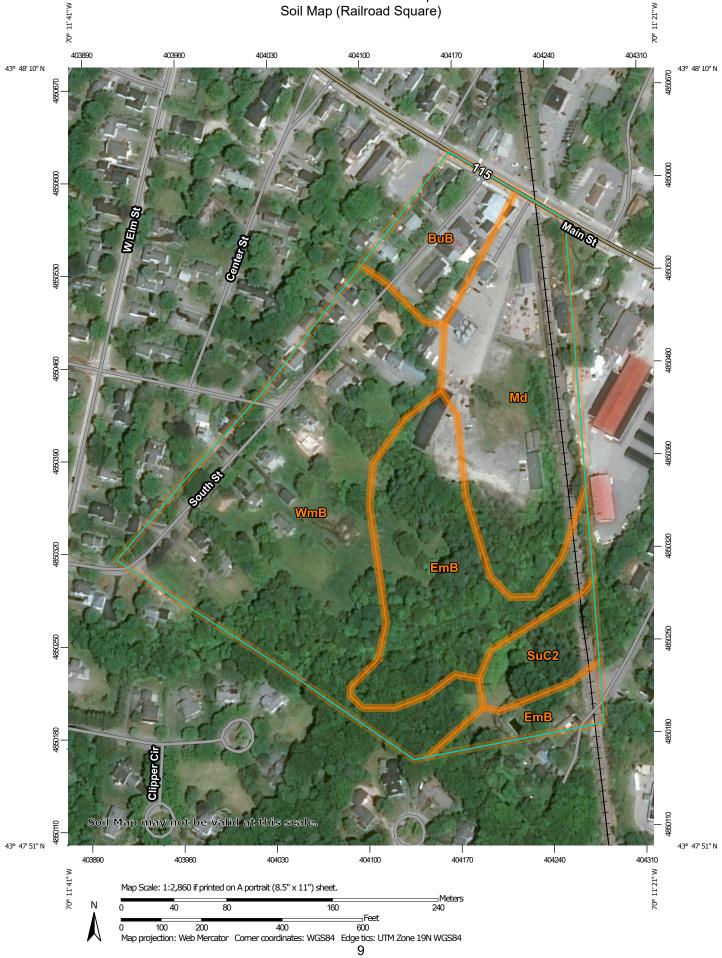
After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map (Railroad Square)



	MAP L	EGEND)	MAP INFORMATION
	terest (AOI) Area of Interest (AOI)	8	Spoil Area Stony Spot	The soil surveys that comprise your AOI were mapped at 1:24,000.
Soils	Soil Map Unit Polygons Soil Map Unit Lines	© ∜ △	Very Stony Spot Wet Spot Other	Warning: Soil Map may not be valid at this scale. Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil
అ	Soil Map Unit Points Point Features Blowout		Special Line Features atures Streams and Canals	line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.
⊠ × ◇	Borrow Pit Clay Spot Closed Depression	Transport	t ation Rails Interstate Highways	Please rely on the bar scale on each map sheet for map measurements.
**	Gravel Pit Gravelly Spot	~	US Routes Major Roads	Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
0 A 4	Landfill Lava Flow Marsh or swamp	Backgrou	Local Roads Ind Aerial Photography	Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
* 0 0	Mine or Quarry Miscellaneous Water Perennial Water			This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
× + ∷	Rock Outcrop Saline Spot Sandy Spot			Soil Survey Area: Cumberland County and Part of Oxford County, Maine Survey Area Data: Version 16, Sep 16, 2019
⊕ ◊	Severely Eroded Spot Sinkhole Slide or Slip			Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Jun 7, 2019—Jul 2,
¢ Ø	Sodic Spot			2019 The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
BuB	Lamoine silt loam, 3 to 8 percent slopes	1.9	7.7%
EmB	Elmwood fine sandy loam, 0 to 8 percent slopes	5.6	22.5%
Md	Made land	5.8	23.2%
SuC2	Suffield silt loam, 8 to 15 percent slopes, eroded	1.2	4.7%
WmB	Windsor loamy sand, 0 to 8 percent slopes	10.5	41.8%
Totals for Area of Interest		25.1	100.0%

Map Unit Legend (Railroad Square)

Map Unit Descriptions (Railroad Square)

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Cumberland County and Part of Oxford County, Maine

BuB—Lamoine silt loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2t0kc Elevation: 10 to 490 feet Mean annual precipitation: 33 to 60 inches Mean annual air temperature: 36 to 52 degrees F Frost-free period: 90 to 160 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Lamoine and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Lamoine

Setting

Landform: Marine terraces, river valleys Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope Down-slope shape: Linear Across-slope shape: Linear Parent material: Fine glaciomarine deposits

Typical profile

Ap - 0 to 7 inches: silt loam Bw - 7 to 13 inches: silt loam Bg - 13 to 24 inches: silty clay loam Cg - 24 to 65 inches: silty clay

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Somewhat poorly drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr)
Depth to water table: About 6 to 17 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water storage in profile: Moderate (about 7.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3w Hydrologic Soil Group: C/D Hydric soil rating: No

Minor Components

Scantic

Percent of map unit: 10 percent Landform: Marine terraces, river valleys Landform position (two-dimensional): Toeslope, footslope Landform position (three-dimensional): Base slope Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

Buxton

Percent of map unit: 3 percent Landform: Marine terraces, river valleys Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: No

Ragmuff

Percent of map unit: 1 percent Landform: Marine terraces, river valleys Landform position (two-dimensional): Backslope, shoulder Landform position (three-dimensional): Side slope, base slope Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

Biddeford

Percent of map unit: 1 percent Landform: Marine terraces, river valleys Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Concave Ecological site: Marine Terrace Depression (F144BY002ME) Hydric soil rating: Yes

EmB—Elmwood fine sandy loam, 0 to 8 percent slopes

Map Unit Setting

National map unit symbol: blh8 Elevation: 10 to 900 feet Mean annual precipitation: 36 to 55 inches Mean annual air temperature: 39 to 46 degrees F Frost-free period: 90 to 195 days Farmland classification: All areas are prime farmland

Map Unit Composition

Elmwood and similar soils: 88 percent *Minor components:* 12 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Elmwood

Setting

Landform: Stream terraces Landform position (two-dimensional): Footslope Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Parent material: Coarse-loamy glaciolacustrine deposits

Typical profile

H1 - 0 to 8 inches: fine sandy loam

H2 - 8 to 25 inches: sandy loam

H3 - 25 to 65 inches: silty clay loam

Properties and qualities

Slope: 0 to 8 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately high (0.00 to 0.20 in/hr)
Depth to water table: About 18 to 36 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: High (about 9.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2w Hydrologic Soil Group: B Hydric soil rating: No

Minor Components

Swanton

Percent of map unit: 7 percent Landform: Stream terraces Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: Yes

Melrose

Percent of map unit: 3 percent Landform: Stream terraces Landform position (two-dimensional): Backslope Landform position (three-dimensional): Riser Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Whately

Percent of map unit: 2 percent Landform: Stream terraces Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread *Down-slope shape:* Concave *Across-slope shape:* Concave *Hydric soil rating:* Yes

Md—Made land

Map Unit Setting

National map unit symbol: blj8 Elevation: 10 to 1,800 feet Mean annual precipitation: 30 to 50 inches Mean annual air temperature: 37 to 46 degrees F Frost-free period: 90 to 160 days Farmland classification: Not prime farmland

Map Unit Composition

Made land: 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Made Land

Setting

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear

Typical profile

H1 - 0 to 65 inches: variable

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8s Hydric soil rating: No

Minor Components

Buxton

Percent of map unit: 3 percent Landform: Coastal plains Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Scantic

Percent of map unit: 3 percent Landform: Coastal plains Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread *Down-slope shape:* Concave *Across-slope shape:* Concave *Hydric soil rating:* Yes

Belgrade

Percent of map unit: 3 percent Landform: Lakebeds Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Deerfield

Percent of map unit: 2 percent Landform: Outwash terraces Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Au gres

Percent of map unit: 2 percent Landform: Outwash terraces Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Hollis

Percent of map unit: 2 percent Landform: Till plains Landform position (two-dimensional): Footslope Landform position (three-dimensional): Rise Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

SuC2—Suffield silt loam, 8 to 15 percent slopes, eroded

Map Unit Setting

National map unit symbol: blk1 Elevation: 10 to 900 feet Mean annual precipitation: 34 to 48 inches Mean annual air temperature: 43 to 46 degrees F Frost-free period: 90 to 160 days Farmland classification: Not prime farmland

Map Unit Composition

Suffield and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Suffield

Setting

Landform: Coastal plains Landform position (two-dimensional): Backslope Landform position (three-dimensional): Riser Down-slope shape: Linear Across-slope shape: Linear Parent material: Fine glaciolacustrine deposits

Typical profile

H1 - 0 to 6 inches: silt loam *H2 - 6 to 23 inches:* silt loam *H3 - 23 to 33 inches:* silty clay *H4 - 33 to 65 inches:* silty clay

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately high (0.00 to 0.20 in/hr)
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: High (about 9.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4e Hydrologic Soil Group: C Hydric soil rating: No

Minor Components

Hartland

Percent of map unit: 6 percent Landform: Coastal plains Landform position (two-dimensional): Backslope Landform position (three-dimensional): Riser Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Belgrade

Percent of map unit: 5 percent Landform: Coastal plains Landform position (two-dimensional): Backslope Landform position (three-dimensional): Riser Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Suffield, slopes >15%

Percent of map unit: 2 percent Landform: Coastal plains Landform position (two-dimensional): Backslope Landform position (three-dimensional): Riser Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Suffield, slopes <8%

Percent of map unit: 2 percent Landform: Coastal plains Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

WmB—Windsor loamy sand, 0 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2w2x2 Elevation: 0 to 1,410 feet Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F Frost-free period: 140 to 240 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Windsor and similar soils: 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Windsor

Setting

Landform: Outwash terraces, deltas, outwash plains, dunes
Landform position (three-dimensional): Tread, riser
Down-slope shape: Linear, convex
Across-slope shape: Linear, convex
Parent material: Loose sandy glaciofluvial deposits derived from granite and/or loose sandy glaciofluvial deposits derived from schist and/or loose sandy glaciofluvial deposits derived from gneiss

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material

A - 1 to 3 inches: loamy sand

Bw - 3 to 25 inches: loamy sand

C - 25 to 65 inches: sand

Properties and qualities

Slope: 0 to 8 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Excessively drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water storage in profile: Low (about 4.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2s Hydrologic Soil Group: A Hydric soil rating: No

Minor Components

Hinckley

Percent of map unit: 5 percent Landform: Outwash plains, eskers, deltas, kames Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Nose slope, side slope, crest, head slope, rise Down-slope shape: Convex Across-slope shape: Linear, convex Hydric soil rating: No

Agawam

Percent of map unit: 5 percent

Landform: Kames, moraines, outwash terraces, kame terraces, outwash plains Landform position (two-dimensional): Footslope, summit, backslope, shoulder Landform position (three-dimensional): Side slope, crest, tread, riser, rise Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

Deerfield

Percent of map unit: 5 percent Landform: Outwash plains, deltas, terraces Landform position (two-dimensional): Footslope Landform position (three-dimensional): Tread, talf Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

ATTACHMENT A -STORMWATER QUALITY CALCULATIONS



Pre-Development Redevelopment Area Information Railroad Square Re-development

	298 Main Street		
Cover	Area (sf)	Area (acre)	Pollutant Ranking
Roads	0	0.00	5
Other Roads	4049	0.09	4
Other parking/drives	0	0.00	3
Other rooftops	5293	0.12	2
Non-grass landscaped	0	0.00	1
Forest/meadow	0	0.00	0
	9342	0.21	0.61

	North Parcel		
Cover	Area (sf)	Area (acre)	Pollutant Ranking
Roads	0	0.00	5
Other Roads	18557	0.43	4
Other parking/drives	0	0.00	3
Other rooftops	26231	0.60	2
Non-grass landscaped	10157	0.23	1
Forest/meadow	6301	0.14	0
	61246	1.41	3.14

	South Parcel		
Cover	Area (sf)	Area (acre)	Pollutant Ranking
Roads	0	0.00	5
Other Roads	28993	0.67	4
Other parking/drives	0	0.00	3
Other rooftops	47115	1.08	2
Non-grass landscaped	0	0.00	1
Forest/meadow	50005	1.15	0
	126113	2.90	4.83

TOTAL AREA (ACI	RES) 4.52	
TOTAL AREA (S.F	5.) 196,891	
EXISTING P	OLLUTANT IMPACT RATING	8.58

EXISTING IMPACT RATING PER ACRE 1.90

Post-Development Redevelopment Area Information Railroad Square Re-Development

298 Main Street

Cover	Area (sf)	Area (acre)	Pollutant Ranking
Roads	0	0.00	5
Other Roads	0	0.00	4
Other parking/drives	0	0.00	3
Other rooftops	9057	0.21	2
Non-grass landscaped	285	0.01	1
Forest/meadow	0	0.00	0
	9342	0.21	0.42

North Parcel

Cover	Area (sf)	Area (acre)	Pollutant Ranking
Roads	0	0.00	5
Other Roads	10577	0.24	4
Other parking/drives	8371	0.19	3
Other rooftops	42471	0.98	2
Non-grass landscaped	0	0.00	1
Forest/meadow	0	0.00	0
	61419	1.41	3.50

	South Parcel		
Cover	Area (sf)	Area (acre)	Pollutant Ranking
Roads	0	0.00	5
Other Roads	19721	0.45	4
Other parking/drives	0	0.00	3
Other rooftops	67920	1.56	2
Non-grass landscaped	6000	0.14	1
Forest/meadow	32592	0.75	0
	126233	2.90	4.93

TOTAL AREA (ACRES)
TOTAL AREA (S.F.)

POST POLLUTANT IMPACT RATING	8.85	
POST DEVELOPMENT IMPACT RATING PER ACRE	1.96	
PRE-DEVELOPMENT IMPACT RATING PER ACRE	1.90	
RANKED IMPACT CHANGE DUE TO DEVELOPMENT	0.06	

Per Chapter 500 a ranked impact change = to 0.06 equates to 60% developed area treatment See Chapter 500 Table 3 below.

Table 3			
Treatment Levels for Redevelopment Projects			

Ranked Impact Change Due to Redevelopment	Percentage of Developed Area that Must be Treated
0.0 or less	0% (Stormwater projects) 50% (Site projects)
$\geq 0.0 \text{ to} \leq 1.0$	60%
> 1.0 to ≤ 2.0	70%
> 2.0 to ≤ 3.0	80%
> 3.0	Same treatment level as for new development

Railroad Square Pre-Development Areas 20-001.1

SP	Subcatchment		Cover	Areas	Existing Treatment
					reaching the
			Landscaped	50	
7	AP-1 SC-1		Woods/Brush	0	
AP			Ex. Imperv.	9,292	
			Offsite Imp	10,475	
		Total		19,817	
			Landscaped	0	
	SC-2		Woods/Brush	0	_
	x		Ex. Imperv.	784	_
			Offsite Imp	853	_
		Total		1,637	
			Offiste Landscpaed	746	
			landscape	847	_
	SC-3		Woods/Brush	0	_
	Ś		Ex. Imperv.	2,403	_
			Offsite Imp	13,779	_
	+	Total	Off-it-landar	17,775	
			Offsite landscape	1,646	
	-		Landscaped	914	_
	SC-4		Woods/Brush	1,338	
	S		Ex. Imperv.	1,670	_
		<i>T</i> • 1	Offsite Imp	7,341	_
		Total		12,909	
			Landscaped	0	-
	'n		Woods/Brush	0	_
	SC-5		Ex. Imperv.	2,744	_
			Offsite Imp	5,773	_
		Total		8,517	
			Landscaped	0	
	SC-6		Woods/Brush	0	
	sc		Ex. Imperv.	630	
			Offsite Imp	3,268	
		Total		3,898	
				- (_
	N .		Offsite Landscape	2,635	_
	sc-7		Woods/Brush	1,040	_
	s		Ex. Imperv. Offsite Imp	0	
		Total	onsite mip	9,239 12,914	
		Total		12,914	
			Offsite Landscape	2,284	
	×		Woods/Brush	0	
	SC-8		Ex. Imperv.	0	
			Offsite Imp	9,056	
		Total		11,340	
					_
	SC-9		Offsite Landscaped	11,072	_
			Woods/Brush	0	
			Ex. Imperv.	0	_
			Offsite Imp	4,183	_
	 	Total		15,255	
			Offsite Landscape	14,742	_
			Landscaped	31,907	_
AP-2	SC-10		Woods/Brush	56,297	_
SC N	sc		Ex. Imperv.	88,906	_
			Offsite Imp	10,759	4
		Total		202,611	

TOTAL

7.04

lotals	Landscaped	33,718	0.77	Parcel Size
	Woods/Brush	56,297	1.29	4.51
	Impervious	106,429	2.44	
	Offsite Imp	74,726	1.72	
Offsite Landscap		35,503	0.82	

306,673

Railroad Square Post-Development Areas 20-001.1

SP	Subcatchmnet		Cover	Areas	Existing Treatment
					, i i i i i i i i i i i i i i i i i i i
AP-1			Landscaped	303	
	SC-10		Woods/Brush	0	
A	A) SC		Imperv.	9,039	
		Total	Offsite Imp	10,475	-
		10101		19,817	
			Landscaped	0	
	50		Woods/Brush	0	
	SC-20		Imperv.	784	
	•2		Offsite Imp	853	
		Total		1,637	
			Offsite Landscape	746	
	•		Landscaped	754	
	SC-30		Woods/Brush	0	
	x		Imperv. Offsite Imp	2,496	4
		Total	Unsite mip	13,779 <i>17,775</i>	-
		10101	Offsite landscape	1,646	
			Landscaped	0	1
	40		Woods/Brush	1,338	1
	SC-40		Imperv.	806]
	51		Offsite Imp	7,341	
		Total		11,131	
	<u> </u>		Landscaped	0	
	SC-50		Woods/Brush	0	
	sc		Imperv.	1,775	-
		Total	Offsite Imp	5,773	4
		Total		7,548	
			Landscaped	0	1
	60		Woods/Brush	0	
	SC-60		Imperv.	2,700	
	•2		Offsite Imp	3,268	
		Total		5,968	
	•		Offsite Landscape	2,635	4
	SC-70		Woods/Brush	1,040	
	Š		Imperv. Offsite Imp	0	-
		Total	Offsite http	9,239 12,914	
 	1 1	20141	1	1=,714	
			Offsite Landscape	2,284	1
	80		Woods/Brush	0]
	SC-80		Ex. Imperv.	0	
			Offsite Imp	9,056	
L	ļ	Total	ļ	11,340	
					4
	Q		Offsite Landscaped Woods/Brush	11,072	1
	C-9	SC-90	Imperv.	0	
	Š		Offsite Imp	4,183	1
		Total	top	15,255	1
	1		Offsite Landscape	16,896	
			Landscaped	41,443]
AP-2	SC-100		Woods/Brush	0	
Ŧ	sc-		Imperv.	0	
			Offsite Imp	10,765	4
	 	Total		69,104	
			Offsite Landscape	= 019	-
N	1		Landscaped Woods/Brush	7,218	4
AP-2	SC-101		Imperv.	3,766 8,017	1
4			imperv.	0,01/	1

Railroad Square Post-Development Areas 20-001.1

			Offsite Imp		1
		Total	F	19,001	
			Offsite Landscape	~	
			Landscaped	3,387	
Ņ	SC-102		Woods/Brush	0.0 /	
AP-2	- S		Imperv.	4,611	_
, v		Offsite Imp	-0.0		
		Total	r i r	7,998	-
			Offsite Landscape	1,55%	
			Landscaped	1,185	-
ର୍	SC-103		Woods/Brush	1,100	
AP-2	통		Imperv.	17,688	-
ł	Š		Offsite Imp	17,000	-
		Total	onsite imp	18,873	
		10101	Offsite Landscape	10,0/3	
		-	Landscaped	1,786	
0	4		-	1,/00	-
AP-2	SC-104		Woods/Brush	19 900	-
A	sc		Imperv.	18,803	-
		T-+1	Offsite Imp	00 -0°	4
		Total		20,589	4
			Offsite Landscape	0.1	-
	o U		Landscaped	896	
AP-2	SC-105		Woods/Brush		
A	sc		Imperv.	8,810	_
			Offsite Imp		_
		Total		9,706	
			Offsite Landscape	0	
			Landscaped	15,887	
AP-2	SC-106		Woods/Brush	0	
W		sc		Imperv.	24,393
	•		Offsite Imp	0	
		Total		40,280	
			Offsite Landscape		
	5		Landscaped	2,145	
AP-2	SC-107		Woods/Brush		
AP	ς'		Imperv.	3,214	
	02		Offsite Imp		
		Total		5,359]
			Offsite Landscape		
			Landscaped	2,447]
Ņ	SC-108		Woods/Brush	2,448	1
AP-2			Imperv.	0	1
	so		Offsite Imp		1
		Total		4,895	1
		1	Offsite Landscape		1
			Landscaped	250	1
Ņ	SC-109		Woods/Brush	0	1
AP-2	5		Imperv.	2,326	
,	Ś		Offsite Imp	, , ,–~	1
		Total	r	2,576	1
			Offsite Landscape	-,0/ ~	1
			Landscaped	0	1
0	10		Woods/Brush	0	1
AP-2	SC-110		Imperv.		1
P.	š			4,907	4
		The Avel	Offsite Imp		-
	L	Total		4,907	1

TOTAL

7.04

	Landscaped	77,701	1.78	Site Area
als	Woods/Brush	8,592	0.20	4.51
Tot	Impervious	110,369	2.53	
-	Offsite Imp	74,732	1.72	
Offsite Landscar		35,279	0.81	

306,673

STORMWATER TREATMENT SUMMARY RAILROAD SQUARE

	Square Feet	Acres
Total Area	196,455	4.51

Predeveloped Site Summary

_	Square Feet	Acres
Developed Area	140,147	3.22
Impervious Area	106,429	2.44
Forrested Area	56,308	1.29

onsite

Proposed Site Summary

	Square Feet	Acres	Percentage of Total Area
Developed Area	188,070	4.32	96%
Impervious Area	110,369	2.53	56%
Forrested Area	8,592	0.2	4%

Proposed Treatment Summary

	Impervious Area Treated		Developed A	Area Treated	BMP
	Square Feet	Percent of Total Impervious	Square Feet	Percent of Total Developed	
SC-10	520	0.5%	520	0.3%	Pervious Pavers
SC-101	8,017	7%	15,235	8%	Filter Basin
SC-102	0	0%	0	0%	
SC-103	17,688	16%	18,873	10%	Pervious pavers and Sand Filter
SC-104	18,803	17%	20,589	11%	Drip Strip
SC-105	8,810	8%	9,706	5%	Drip Strip
SC-106	24,393	22%	40,280	21%	R-Tank and Sand filter
SC-107	3,214	3%	5,359	3%	Drip Strip
SC-108		0%		0%	
SC-109	2,326	2%	2,576	1%	Pervious Pavers and Sand Flter
SC-110	4,907	4%	4,907	3%	Pervious Pavers and Sand Filter
PROJECT	88,678	80.3%	118,045	62.8%	

		Units
Impervious Area	24,393	SF
Landscaped Area	15,887	SF
Storage Volume Required	2,562	CF
Surface Area Required	1,537	SF
Ponding Depth for Water Quality Volume	18	In
Bed Surface Area Provided	1,616	SF
Total Water Quality Storage Volume Provided	4,332	CF

		Units
Impervious Area	8,017	SF
Landscaped Area	7,218	SF
Storage Volume Required	909	CF
Surface Area Required	545	SF
Ponding Depth for Water Quality Volume	18	In
Bed Surface Area Provided	691	SF
Total Water Quality Storage Volume Provided	1,400	CF

Roof Drip Line Filter	DF-1 Sizing	
Lot 4		Units
Impervious Area	18,803	SF
Landscaped Area	1,786	SF
Storage Volume Required	1,626	CF
Storage Volume Provided		
Length	446.0	FT
Width	4.0	FT
Depth	2.8	\mathbf{FT}
Stone Porosity	0.4	
Length	0.0	FT
Width	0.0	\mathbf{FT}
Depth	0.0	FT
Stone Porosity	0.4	
Total Water Quality Storage Volume Provided	1,962	CF

Roof Drip Line Filter DF-2 Sizing				
Lot 5	-	Units		
Impervious Area	8,810	SF		
Landscaped Area	896	SF		
Storage Volume Required	764	CF		
Storage Volume Provided				
Length	224.0	\mathbf{FT}		
Width	4.0	\mathbf{FT}		
Depth	2.5	\mathbf{FT}		
Stone Porosity	0.4			
Length	0.0	FT		
Width	0.0	\mathbf{FT}		
Depth	0.0	FT		
Stone Porosity	0.4			
Total Water Quality Storage Volume Provided	896	CF		

Roof Drip Line Filter	DF-3 Sizing	
Lot 6/7	_	Units
Impervious Area	3,214	SF
Landscaped Area	2,145	SF
Storage Volume Required	339	CF
Storage Volume Provided		
Length	160.0	FT
Width	3.0	FT
Depth	2.00	FT
Stone Porosity	0.4	
Length	0.0	FT
Width	0.0	FT
Depth	0.0	FT
Stone Porosity	0.0	
Total Water Quality Storage Volume Provided	384	CF

Pervious Pavers Area 1					
Watershed 103S		Units			
Impervious Area	17,688	SF			
Landscaped Area	1,185	SF			
Storage Volume Required	1,514	CF			
Stormwater Reservoir					
<u>Storage Volume Provided</u>					
Area	3978.0	SF			
Depth	1.00	FT			
Stone Porosity	0.4				
Length	0.0	FT			
Width	0.0	FT			
Depth	0.0	FT			
Stone Porosity	0.0				
Total Water Quality Storage Volume Provided	1,591	CF			

Pervious Pavers	Area 2	
Watershed 109S		Units
Impervious Area	2,326	SF
Landscaped Area	250	SF
Storage Volume Required	202	CF
Stormwater Reservoir		
Storage Volume Provided		
Area	1023.0	SF
Depth	1.00	FT
Stone Porosity	0.4	
Length	0.0	FT
Width	0.0	FT
Depth	0.0	FT
Stone Porosity	0.0	
Total Water Quality Storage Volume Provided	409	CF

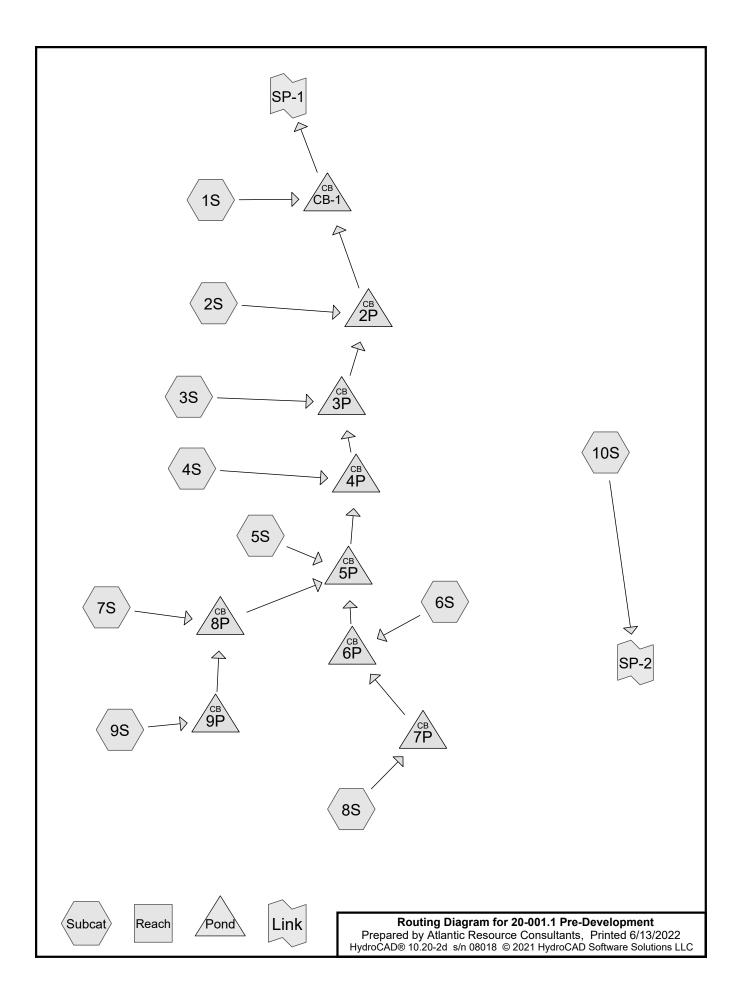
Pervious Pavers	Area 3	
Watershed 110S		Units
Impervious Area	4,907	SF
Landscaped Area	0	SF
Storage Volume Required	409	CF
Stormwater Reservoir		
Storage Volume Provided		
Area	3446.0	SF
Depth	1.00	FT
Stone Porosity	0.4	
Length	0.0	FT
Width	0.0	FT
Depth	0.0	FT
Stone Porosity	0.0	
Total Water Quality Storage Volume Provided	1,378	CF

ATTACHMENT B -HYDROCAD RUNOFF AND ROUTING CALCULATIONS



ATTACHMENT B (I) -PRE-DEVELOPMENT MODEL RESULTS





Summary for Subcatchment 1S:

Runoff = 1.36 cfs @ 12.08 hrs, Volume= 0.109 af, Depth= 2.87" Routed to Pond CB-1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

	Area (sf)	CN	Description				
*	10,475	98	Offsite impe	ervious			
*	9,292	98	Existing Im	pervious			
	50	74	>75% Gras	s cover, Go	ood, HSG C		
	19,817	98	Weighted A	verage			
	50		0.25% Perv	vious Area			
	19,767		99.75% lmp	pervious Ar	rea		
	Tc Length			Capacity			
(r	nin) (feet)	(ft/f	t) (ft/sec)	(cfs)			
	6.0				Direct Entry,		

Summary for Subcatchment 2S:

Runoff = 0.11 cfs @ 12.08 hrs, Volume= 0.009 af, Depth= 2.87" Routed to Pond 2P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

	A	rea (sf)	CN	Description				
*		784	98	Existing Imp	pervious			
*		853	98	Offsite Impe	ervious			
		1,637	98	Weighted Average				
		1,637		100.00% Im	npervious A	Area		
	Тс	Length	Slope	e Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
	6.0					Direct Entry,		

Summary for Subcatchment 3S:

Runoff = 1.18 cfs @ 12.09 hrs, Volume= 0.090 af, Depth= 2.65" Routed to Pond 3P :

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022

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	А	rea (sf)	CN	Description					
*		13,779	98	Offsite Impe	ervious				
*		2,403	98	Existing Im	pervious				
		1,593	74	>75% Gras	s cover, Go	bod, HSG C			
		17,775	96	Weighted A	verage				
		1,593		8.96% Perv	vious Area				
		16,182		91.04% Imp	pervious Ar	ea			
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
	6.0					Direct Entry,			

Summary for Subcatchment 4S:

Runoff = 0.71 cfs @ 12.09 hrs, Volume= 0.051 af, Depth= 2.08" Routed to Pond 4P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

	Area (sf)) CN	Description	Description				
*	7,341	98	Offsite Impe	ervious				
*	1,670	98	Existing Im	pervious				
	2,560) 74	>75% Gras	s cover, Go	lood, HSG C			
	1,338	65	Brush, Goo	d, HSG C				
	12,909	90	Weighted A	Weighted Average				
	3,898	}	30.20% Per	vious Area	а			
	9,011		69.80% lmp	pervious Ar	rea			
	Tc Lengt			Capacity				
(r	min) (fee	t) (ft/	ft) (ft/sec)	(cfs)				
	6.0				Direct Entry,			

Summary for Subcatchment 5S:

Runoff = 0.58 cfs @ 12.08 hrs, Volume= 0.047 af, Depth= 2.87" Routed to Pond 5P :

	Area (sf)	CN	Description			
*	5,773	98	Offsite Impervious			
*	2,744	98	Existing Impervious			
	8,517	98	Weighted Average			
	8,517		100.00% Impervious Area			

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			08018 © 202	lutions LLC		Page 4			
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description				
6.0					Direct Entr	ъy,			
			Su	immary fo	or Subcate	hment 6S:			
Runoff Route	Runoff = 0.27 cfs @ 12.08 hrs, Volume= 0.021 af, Depth= 2.87" Routed to Pond 6P :								
			ethod, UH=: n Rainfall=3		ited-CN, Time	e Span= 0.00-72.00 ł	nrs, dt= 0.03 hrs		
A	rea (sf)	CN	Description	<u>ו</u>					
*	3,268 630		Offsite Imp Existing Im						
	3,898 3,898	98	Weighted / 100.00% li	Average npervious A	Area				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description				
6.0					Direct Entr	у,			
			Su	immary fo	or Subcatc	hment 7S:			
Runoff Route	= ed to Pone		cfs @ 12.0)9 hrs, Volu	ıme=	0.051 af, Depth= 2	08"		
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"									
А	rea (sf)	CN	Descriptior	l					
*	9,239 2.635		Offsite Imp >75% Gras		ood. HSG C				

Type III 24-hr 2-Yr Storm Rainfall=3.10"

		011	Beechbach						
*	9,239	98	Offsite Impervious						
	2,635	74	>75% Grass cover, Good, HSG C						
	1,040	65	Brush, Good, HSG C						
	12,914	90	Weighted Average						
	3,675		28.46% Pervious Area						
	9,239		71.54% Impervious Area						
	Tc Length	Slop	e Velocity Capacity Description						
(min) (feet)	(ft/							

6.0

20-001.1 Pre-Development

Direct Entry,

Summary for Subcatchment 8S:

Runoff = 0.58 cfs @ 12.00 hrs, Volume= 0.035 af, Depth= 1.60" Routed to Pond 7P :

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022 tions LLC Page 5

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	Area (sf)	CN	Description
*	9,056	98	Offsite Impervious
	2,284	30	Brush, Good, HSG A
	11,340	84	Weighted Average
	2,284		20.14% Pervious Area
	9,056		79.86% Impervious Area

Summary for Subcatchment 9S:

Runoff	=	0.00 cfs @	13.80 hrs,	Volume=	0.003 af,	Depth= 0.09"
Routed	to Pond	19P:				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

	Area (sf)	CN	Description					
*	4,183	98	Offsite Impe	ervious area	a			
	11,072	30	Brush, Goo	d, HSG A				
	15,255	49	Weighted A	Weighted Average				
	11,072		72.58% Pe	72.58% Pervious Area				
	4,183		27.42% lmp	pervious Ar	rea			
	Tc Length			Capacity	Description			
	(min) (feet)) (ft/	ft) (ft/sec)	(cfs)				
	6.0				Direct Entry,			
					•			

Summary for Subcatchment 10S:

Runoff = 5.03 cfs @ 12.14 hrs, Volume= 0.420 af, Depth= 1.08" Routed to Link SP-2 :

	Area (sf)	CN	Description			
*	10,759	98	Offsite Impervious			
*	68,433	98	Existing Impervious			
	36,503	39	>75% Grass cover, Good, HSG A			
	6,316	30	Brush, Good, HSG A			
	49,266	70	Noods, Good, HSG C			
	10,861	55	Woods, Good, HSG B			
*	20,473	96	Existing Gravel surface, HSG D			
	202,611	76	Weighted Average			
	123,419		60.91% Pervious Area			
	79,192		39.09% Impervious Area			

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022 tions LLC Page 6

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.0	75	0.0200	1.28		Sheet Flow, A-B
					Smooth surfaces n= 0.011 P2= 3.10"
2.2	314	0.0210	2.33		Shallow Concentrated Flow, B-C
					Unpaved Kv= 16.1 fps
0.4	35	0.3400	1.46		Shallow Concentrated Flow, C-D
					Forest w/Heavy Litter Kv= 2.5 fps
5.0	170	0.0520	0.57		Shallow Concentrated Flow, D-E
					Forest w/Heavy Litter Kv= 2.5 fps
0.7	80	0.0100	1.95	1.95	Parabolic Channel, E-F
					W=3.00' D=0.50' Area=1.0 sf Perim=3.2'
					n= 0.035 Earth, dense weeds

9.3 674 Total

Summary for Pond 2P:

Inflow Area =	1.934 ac, 73.27% Impervious, Inflo	w Depth = 1.91" for 2-Yr Storm event					
Inflow =	3.88 cfs @ 12.08 hrs, Volume=	0.307 af					
Outflow =	3.88 cfs @ 12.08 hrs, Volume=	0.307 af, Atten= 0%, Lag= 0.0 min					
Primary =	3.88 cfs @ 12.08 hrs, Volume=	0.307 af					
Routed to Pond CB-1 :							

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.84' @ 12.08 hrs Flood Elev= 95.86'

Device	Routing	Invert	Outlet Devices		
#1	Primary	90.65'	12.0" Round Culvert L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.65' / 90.00' S= 0.0103 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf		

Primary OutFlow Max=3.84 cfs @ 12.08 hrs HW=92.80' (Free Discharge) **1=Culvert** (Inlet Controls 3.84 cfs @ 4.89 fps)

Summary for Pond 3P:

Inflow Are	a =	1.896 ac, 7	72.74% Impervious	, Inflow Depth =	1.89"	for 2-Yr Storm event
Inflow	=	3.77 cfs @	12.08 hrs, Volum	ie= 0.298	af	
Outflow	=	3.77 cfs @	12.08 hrs, Volum	ie= 0.298	af, Atte	en= 0%, Lag= 0.0 min
Primary	=	3.77 cfs @	12.08 hrs, Volum	ie= 0.298	af	-
Routed	to Pond	d 2P :				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.84' @ 12.08 hrs Flood Elev= 95.40'

Device	Routing	Invert	Outlet Devices		
#1	Primary	90.75'	12.0" Round Culvert		
	-		L= 50.0' CPP, projecting, no headwall, Ke= 0.900		

Inlet / Outlet Invert= 90.29' / 90.75' S= -0.0092 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=3.73 cfs @ 12.08 hrs HW=92.81' (Free Discharge) -1=Culvert (Inlet Controls 3.73 cfs @ 4.74 fps)

Summary for Pond 4P:

1.488 ac, 67.72% Impervious, Inflow Depth = 1.68" for 2-Yr Storm event Inflow Area = 2.60 cfs @ 12.07 hrs, Volume= Inflow 0.208 af = 2.60 cfs @ 12.07 hrs, Volume= 0.208 af, Atten= 0%, Lag= 0.0 min Outflow = 2.60 cfs @ 12.07 hrs, Volume= = 0.208 af Primary Routed to Pond 3P :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.11' @ 12.07 hrs Flood Elev= 96.14'

Device	Routing	Invert	Outlet Devices		
#1	Primary	90.85'	12.0" Round Culvert L= 77.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.85' / 90.29' S= 0.0073 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf		

Primary OutFlow Max=2.57 cfs @ 12.07 hrs HW=92.09' (Free Discharge) -1=Culvert (Inlet Controls 2.57 cfs @ 3.27 fps)

Summary for Pond 5P:

 Inflow Area =
 1.192 ac, 67.20% Impervious, Inflow Depth =
 1.58" for 2-Yr Storm event

 Inflow =
 1.91 cfs @
 12.07 hrs, Volume=
 0.157 af

 Outflow =
 1.91 cfs @
 12.07 hrs, Volume=
 0.157 af, Atten= 0%, Lag= 0.0 min

 Primary =
 1.91 cfs @
 12.07 hrs, Volume=
 0.157 af

 Routed to Pond 4P :
 12.07 hrs, Volume=
 0.157 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.09' @ 12.07 hrs Flood Elev= 95.65'

Device	Routing	Invert	Outlet Devices
#1	Primary	91.13'	12.0" Round Culvert
			L= 70.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 91.13' / 90.85' S= 0.0040 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf
#2	Primary	95.65'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=1.89 cfs @ 12.07 hrs HW=92.08' (Free Discharge)

-2=Orifice/Grate (Controls 0.00 cfs)

¹⁼Culvert (Barrel Controls 1.89 cfs @ 3.14 fps)

Summary for Pond 6P:

Inflow Area = 0.350 ac, 85.01% Impervious, Inflow Depth = 1.92" for 2-Yr Storm event Inflow 0.76 cfs @ 12.01 hrs. Volume= 0.056 af = 0.76 cfs @ 12.01 hrs, Volume= Outflow = 0.056 af, Atten= 0%, Lag= 0.0 min 0.76 cfs @ 12.01 hrs, Volume= Primary = 0.056 af Routed to Pond 5P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.33' @ 12.01 hrs Flood Elev= 96.08' Device Routing Invert **Outlet Devices** #1 Primary 91.83' 12.0" Round Culvert L= 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=0.74 cfs @ 12.01 hrs HW=92.33' (Free Discharge) **1=Culvert** (Inlet Controls 0.74 cfs @ 1.90 fps) Summary for Pond 7P: 0.260 ac, 79.86% Impervious, Inflow Depth = 1.60" for 2-Yr Storm event Inflow Area = Inflow 0.58 cfs @ 12.00 hrs, Volume= 0.035 af = 0.58 cfs @ 12.00 hrs, Volume= Outflow = 0.035 af, Atten= 0%, Lag= 0.0 min Primary = 0.58 cfs @ 12.00 hrs, Volume= 0.035 af Routed to Pond 6P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.89' @ 12.00 hrs Flood Elev= 97.40' Routing Invert Outlet Devices Device 92.43' 12.0" Round Culvert #1 Primary L= 105.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.43' / 92.03' S= 0.0038 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=0.57 cfs @ 12.00 hrs HW=92.89' (Free Discharge) -1=Culvert (Barrel Controls 0.57 cfs @ 2.37 fps) Summary for Pond 8P: Inflow Area = 0.647 ac, 47.65% Impervious, Inflow Depth = 1.00" for 2-Yr Storm event Inflow = 0.71 cfs @ 12.09 hrs, Volume= 0.054 af

 Outflow
 =
 0.71 cfs @
 12.09 hrs, Volume=
 0.054 af, Atten= 0%, Lag= 0.0 min

 Primary
 =
 0.71 cfs @
 12.09 hrs, Volume=
 0.054 af

 Primary
 =
 0.71 cfs @
 12.09 hrs, Volume=
 0.054 af

Routed to Pond 5P :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Peak Elev= 92.65' @ 12.09 hrs Flood Elev= 97.36'

1 1000 Elev - 97.30								
Device Routing Invert Outlet Devices								
#1 Primary 92.16' 12.0" Round Culvert L= 85.0' CPP, projecting, no headwall, K Inlet / Outlet Invert= 92.16' / 91.23' S= 0.0 n= 0.012, Flow Area= 0.79 sf								
Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=92.64' (Free Discharge) ☐ 1=Culvert (Inlet Controls 0.71 cfs @ 1.87 fps)								
Summary for Pond 9P:								
Inflow Area = 0.350 ac, 27.42% Impervious, Inflow Depth = 0.09" fo Inflow = 0.00 cfs @ 13.80 hrs, Volume= 0.003 af Outflow = 0.00 cfs @ 13.80 hrs, Volume= 0.003 af, Atten= Primary = 0.00 cfs @ 13.80 hrs, Volume= 0.003 af Routed to Pond 8P : 0.003 af 0.003 af	r 2-Yr Storm event : 0%, Lag= 0.0 min							
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.09' @ 13.80 hrs Flood Elev= 96.37'	Peak Elev= 93.09' @ 13.80 hrs							
Device Routing Invert Outlet Devices								
#1 Primary 93.06' 12.0'' Round Culvert L= 72.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 93.06' / 92.16' S= 0.0125 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf								
Primary OutFlow Max=0.00 cfs @ 13.80 hrs HW=93.09' (Free Discharge) ☐ 1=Culvert (Inlet Controls 0.00 cfs @ 0.50 fps)								
Summary for Pond CB-1:								
Inflow Area = 2.389 ac, 78.31% Impervious, Inflow Depth = 2.09" fo Inflow = 5.24 cfs @ 12.08 hrs, Volume= 0.416 af Outflow = 5.24 cfs @ 12.08 hrs, Volume= 0.416 af, Atten= Primary = 5.24 cfs @ 12.08 hrs, Volume= 0.416 af Routed to Link SP-1 : 5.24 cfs @ 12.08 hrs, Volume= 0.416 af	r 2-Yr Storm event : 0%, Lag= 0.0 min							
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 91.84' @ 12.08 hrs Flood Elev= 95.55'								
Device Routing Invert Outlet Devices								
#1 Primary 89.95' 15.0" Round Culvert L= 35.0' CPP, projecting, no headwall, K Inlet / Outlet Invert= 89.95' / 89.50' S= 0.0 n= 0.012, Flow Area= 1.23 sf								

Primary OutFlow Max=5.18 cfs @ 12.08 hrs HW=91.81' (Free Discharge) **1=Culvert** (Inlet Controls 5.18 cfs @ 4.22 fps)

Summary for Link SP-1:

 Inflow Area =
 2.389 ac, 78.31% Impervious, Inflow Depth = 2.09" for 2-Yr Storm event

 Inflow =
 5.24 cfs @ 12.08 hrs, Volume=
 0.416 af

 Primary =
 5.24 cfs @ 12.08 hrs, Volume=
 0.416 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Link SP-2:

Inflow Are	a =	4.651 ac, 39.09% Impervious, Inflow Depth = 1.08" for 2-Yr Storm ev	ent
Inflow	=	5.03 cfs @ 12.14 hrs, Volume= 0.420 af	
Primary	=	5.03 cfs @ 12.14 hrs, Volume= 0.420 af, Atten= 0%, Lag= 0.0 r	min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Subcatchment 1S:

Runoff = 2.03 cfs @ 12.08 hrs, Volume= 0.165 af, Depth= 4.36" Routed to Pond CB-1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

	A	rea (sf)	CN	Description				
*		10,475	98	Offsite impe	ervious			
*		9,292	98	Existing Im	pervious			
		50	74	>75% Gras	s cover, Go	ood, HSG C		
		19,817	98	Weighted Average				
		50		0.25% Pervious Area				
		19,767		99.75% Impervious Area				
	Тс	Length	Slope		Capacity			
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.0					Direct Entry,		
						-		

Summary for Subcatchment 2S:

Runoff = 0.17 cfs @ 12.08 hrs, Volume= 0.014 af, Depth= 4.36" Routed to Pond 2P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

	A	rea (sf)	CN	Description				
*		784	98	Existing Imp	pervious			
*		853	98	Offsite Impe	ervious			
		1,637	98	98 Weighted Average				
		1,637		100.00% Im	npervious A	Area		
	Тс	Length	Slope	e Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
	6.0					Direct Entry,		

Summary for Subcatchment 3S:

Runoff = 1.79 cfs @ 12.08 hrs, Volume= 0.141 af, Depth= 4.14" Routed to Pond 3P :

Type III 24-hr 10-Yr Storm Rainfall=4.60"

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	A	rea (sf)	CN	Description						
*		13,779	98	Offsite Impe	ervious					
*		2,403	98	Existing Imp	pervious					
		1,593	74	>75% Gras	s cover, Go	lood, HSG C				
		17,775	96	Weighted A	Veighted Average					
		1,593		8.96 [°] % Perv	ious Area					
		16,182		91.04% Imp	pervious Ar	rea				
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	•				
_	6.0					Direct Entry,				

Summary for Subcatchment 4S:

Runoff = 1.17 cfs @ 12.09 hrs, Volume= 0.086 af, Depth= 3.49" Routed to Pond 4P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

	Area (sf)	CN	Description	Description					
*	7,341	98	Offsite Impe	ervious					
*	1,670	98	Existing Im	pervious					
	2,560	74	>75% Gras	s cover, Go	ood, HSG C				
	1,338	65	Brush, Goo	d, HSG C					
	12,909	90	90 Weighted Average						
	3,898		30.20% Pe	rvious Area	3				
	9,011		69.80% Imp	pervious Ar	rea				
	Tc Lengt			Capacity	Description				
(n	nin) (feet	t) (ft/	ft) (ft/sec)	(cfs)					
	6.0				Direct Entry,				

Summary for Subcatchment 5S:

Runoff = 0.87 cfs @ 12.08 hrs, Volume= 0.071 af, Depth= 4.36" Routed to Pond 5P :

	Area (sf)	CN	Description	
*	5,773	98	Offsite Impervious	
*	2,744	98	Existing Impervious	
	8,517 8,517	98	Weighted Average 100.00% Impervious Area	

Prepare	.1 Pre-D d by Atla D® 10.20-;	ntic Re	esourc	e Coi	Type III 24-hr	10-Yr Storm Rainfall=4.60" Printed 6/13/2022 Page 13		
Tc (min)	Length (feet)	Slop (ft/ft		ocity sec)	Capacity (cfs)	Descriptior	I	
6.0						Direct Ent	ry,	
				Sur	nmary fo	or Subcato	hment 6S:	
Runoff Route	= ed to Pone		cfs @	12.08	3 hrs, Volu	ime=	0.033 af, Dep	th= 4.36"
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"							
A	rea (sf)	CN	Descri	ption				
*	3,268	98	Offsite					
*	630	98			ervious			
	3,898 3,898	98			verage pervious A	vrea		
Tc (min)	Length (feet)	Slop (ft/ft		ocity sec)	Capacity (cfs)	Descriptior	1	
6.0						Direct Ent	ry,	
				Sur	nmary fo	or Subcate	hment 7S:	
Runoff Route	= ed to Pone		cfs @	12.09	9 hrs, Volu	ime=	0.086 af, Dep	th= 3.49"
	y SCS TF 24-hr 10-					ited-CN, Tim	e Span= 0.00-7	2.00 hrs, dt= 0.03 hrs
А	rea (sf)	CN	Descri	ption				
*	9,239	98	Offsite		rvious			
	2,635	74	>75%	Grass	s cover, Go	ood, HSG C		
	1,040	65			d, HSG C			
	12,914	90			verage			
	3,675		28.40		vious Area			

						Description	
-	(min) 6 0	(feet)	(ft/ft)	(ft/sec)	(cfs)	Direct Entry	
	0.0					Direct Entry,	

71.54% Impervious Area

9,239

Direct Entry,

Summary for Subcatchment 8S:

1.05 cfs @ 12.00 hrs, Volume= Runoff 0.063 af, Depth= 2.91" = Routed to Pond 7P :

Type III 24-hr 10-Yr Storm Rainfall=4.60" Printed 6/13/2022

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	Area (sf)	CN	Description	
*	9,056	98	Offsite Impervious	
	2,284	30	Brush, Good, HSG A	
	11,340	84	Weighted Average	
	2,284		20.14% Pervious Area	
	9,056		79.86% Impervious Area	

Summary for Subcatchment 9S:

Runoff	=	0.09 cfs @	12.15 hrs,	Volume=	0.014 af,	Depth= 0	.49"
Routed	I to Pond	19P:				-	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

	Area	ı (sf)	CN	Description			
*	4,	,183	98	Offsite Impe	ervious area	a	
	11,	,072	30	Brush, Goo	d, HSG A		
	15,	,255	49	Weighted Average			
	11,	,072		72.58% Pei	vious Area		
	4,	,183		27.42% Imp	pervious Are	ea	
					-		
		ength	Slope		Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	6.0					Direct Entry,	
						•	

Summary for Subcatchment 10S:

Runoff = 10.68 cfs @ 12.13 hrs, Volume= 0.857 af, Depth= 2.21" Routed to Link SP-2 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

	Area (sf)	CN	Description
*	10,759	98	Offsite Impervious
*	68,433	98	Existing Impervious
	36,503	39	>75% Grass cover, Good, HSG A
	6,316	30	Brush, Good, HSG A
	49,266	70	Woods, Good, HSG C
	10,861	55	Woods, Good, HSG B
*	20,473	96	Existing Gravel surface, HSG D
	202,611	76	Weighted Average
	123,419		60.91% Pervious Area
	79,192		39.09% Impervious Area

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Type III 24-hr 10-Yr Storm Rainfall=4.60" Printed 6/13/2022

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.0	75	0.0200	1.28		Sheet Flow, A-B
					Smooth surfaces n= 0.011 P2= 3.10"
2.2	314	0.0210	2.33		Shallow Concentrated Flow, B-C
					Unpaved Kv= 16.1 fps
0.4	35	0.3400	1.46		Shallow Concentrated Flow, C-D
					Forest w/Heavy Litter Kv= 2.5 fps
5.0	170	0.0520	0.57		Shallow Concentrated Flow, D-E
					Forest w/Heavy Litter Kv= 2.5 fps
0.7	80	0.0100	1.95	1.95	Parabolic Channel, E-F
					W=3.00' D=0.50' Area=1.0 sf Perim=3.2'
					n= 0.035 Earth, dense weeds

9.3 674 Total

Summary for Pond 2P:

Inflow Area =	1.934 ac,	73.27% Impervious, Inf	flow Depth = 3.15" for 10-Yr Storm event
Inflow =	6.20 cfs @	12.08 hrs, Volume=	0.508 af
Outflow =	6.20 cfs @	12.08 hrs, Volume=	0.508 af, Atten= 0%, Lag= 0.0 min
Primary =	6.20 cfs @	12.08 hrs, Volume=	0.508 af
Routed to	Pond CB-1 :		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 95.46' @ 12.08 hrs Flood Elev= 95.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.65'	12.0" Round Culvert L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.65' / 90.00' S= 0.0103 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=6.13 cfs @ 12.08 hrs HW=95.36' (Free Discharge) **1=Culvert** (Inlet Controls 6.13 cfs @ 7.80 fps)

Summary for Pond 3P:

Inflow Area =	1.896 ac, 7	2.74% Impervious,	Inflow Depth = 3	.13" for 10-Yr Storm event
Inflow =	6.03 cfs @	12.08 hrs, Volume	= 0.494 af	
Outflow =	6.03 cfs @	12.08 hrs, Volume	= 0.494 af	, Atten= 0%, Lag= 0.0 min
Primary =	6.03 cfs @	12.08 hrs, Volume	= 0.494 af	_
Routed to P	ond 2P :			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 95.33' @ 12.08 hrs Flood Elev= 95.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.75'	12.0" Round Culvert
			L= 50.0' CPP, projecting, no headwall, Ke= 0.900

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Inlet / Outlet Invert= 90.29' / 90.75' S= -0.0092 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=5.96 cfs @ 12.08 hrs HW=95.24' (Free Discharge) —1=Culvert (Inlet Controls 5.96 cfs @ 7.59 fps)

Summary for Pond 4P:

1.488 ac, 67.72% Impervious, Inflow Depth = 2.85" Inflow Area = for 10-Yr Storm event 4.26 cfs @ 12.07 hrs, Volume= Inflow 0.354 af = 4.26 cfs @ 12.07 hrs, Volume= 0.354 af, Atten= 0%, Lag= 0.0 min Outflow = 4.26 cfs @ 12.07 hrs, Volume= = 0.354 af Primary Routed to Pond 3P :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.38' @ 12.07 hrs Flood Elev= 96.14'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.85'	12.0" Round Culvert L= 77.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.85' / 90.29' S= 0.0073 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=4.21 cfs @ 12.07 hrs HW=93.34' (Free Discharge) -1=Culvert (Inlet Controls 4.21 cfs @ 5.36 fps)

Summary for Pond 5P:

 Inflow Area =
 1.192 ac, 67.20% Impervious, Inflow Depth =
 2.69" for 10-Yr Storm event

 Inflow =
 3.11 cfs @
 12.07 hrs, Volume=
 0.267 af

 Outflow =
 3.11 cfs @
 12.07 hrs, Volume=
 0.267 af, Atten= 0%, Lag= 0.0 min

 Primary =
 3.11 cfs @
 12.07 hrs, Volume=
 0.267 af

 Routed to Pond 4P :
 12.07 hrs, Volume=
 0.267 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.77' @ 12.07 hrs Flood Elev= 95.65'

Device	Routing	Invert	Outlet Devices
#1	Primary	91.13'	12.0" Round Culvert
			L= 70.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.13' / 90.85' S= 0.0040 '/' Cc= 0.900
	. .		n= 0.012, Flow Area= 0.79 sf
#2	Primary	95.65'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=3.08 cfs @ 12.07 hrs HW=92.75' (Free Discharge)

1=Culvert (Barrel Controls 3.08 cfs @ 3.93 fps)

-2=Orifice/Grate (Controls 0.00 cfs)

Inflow Area = 0.350 ac, 85.01% Impervious, Inflow Depth = 3.28" for 10-Yr Storm event Inflow 1.31 cfs @ 12.01 hrs. Volume= 0.096 af = 1.31 cfs @ 12.01 hrs, Volume= Outflow 0.096 af, Atten= 0%, Lag= 0.0 min = 1.31 cfs @ 12.01 hrs, Volume= Primary = 0.096 af Routed to Pond 5P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.53' @ 12.01 hrs Flood Elev= 96.08' Invert Device Routing **Outlet Devices** #1 Primary 91.83' 12.0" Round Culvert L= 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=1.28 cfs @ 12.01 hrs HW=92.52' (Free Discharge) **1=Culvert** (Inlet Controls 1.28 cfs @ 2.23 fps) Summary for Pond 7P: 0.260 ac, 79.86% Impervious, Inflow Depth = 2.91" for 10-Yr Storm event Inflow Area = Inflow 1.05 cfs @ 12.00 hrs, Volume= 0.063 af = 1.05 cfs @ 12.00 hrs, Volume= Outflow = 0.063 af, Atten= 0%, Lag= 0.0 min Primary = 1.05 cfs @ 12.00 hrs, Volume= 0.063 af Routed to Pond 6P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.08' @ 12.00 hrs Flood Elev= 97.40' Routing Invert Outlet Devices Device 92.43' 12.0" Round Culvert #1 Primary L= 105.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.43' / 92.03' S= 0.0038 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=1.04 cfs @ 12.00 hrs HW=93.08' (Free Discharge) **1=Culvert** (Barrel Controls 1.04 cfs @ 2.75 fps) Summary for Pond 8P:

 Inflow Area =
 0.647 ac, 47.65% Impervious, Inflow Depth =
 1.87" for 10-Yr Storm event

 Inflow =
 1.24 cfs @
 12.09 hrs, Volume=
 0.101 af

 Outflow =
 1.24 cfs @
 12.09 hrs, Volume=
 0.101 af, Atten= 0%, Lag= 0.0 min

 Primary =
 1.24 cfs @
 12.09 hrs, Volume=
 0.101 af, Atten= 0%, Lag= 0.0 min

 Primary =
 1.24 cfs @
 12.09 hrs, Volume=
 0.101 af

 Routed to Pond 5P :
 12.09 hrs, Volume=
 0.101 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Peak Elev= 92.83' @ 12.09 hrs Flood Elev= 97.36'

Device Routing Invert Outlet Devices #1 Primary 92.16' 12.0'' Round Culvert L= 85.0' CPP, projecting, no headwall, Ke= 0.900									
Inlet / Outlet Invert= 92.16' / 91.23' S= 0.0109 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf									
Primary OutFlow Max=1.24 cfs @ 12.09 hrs HW=92.83' (Free Discharge) ☐1=Culvert (Inlet Controls 1.24 cfs @ 2.20 fps)									
Summary for Pond 9P:									
Inflow Area = 0.350 ac, 27.42% Impervious, Inflow Depth = 0.49" for 10-Yr Storm event Inflow = 0.09 cfs @ 12.15 hrs, Volume= 0.014 af Outflow = 0.09 cfs @ 12.15 hrs, Volume= 0.014 af, Atten= 0%, Lag= 0.0 min Primary = 0.09 cfs @ 12.15 hrs, Volume= 0.014 af Routed to Pond 8P : 0.014 af 0.014 af									
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.22' @ 12.15 hrs Flood Elev= 96.37'									
Device Routing Invert Outlet Devices									
#1 Primary 93.06' 12.0'' Round Culvert L= 72.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 93.06' / 92.16' S= 0.0125 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf									
Primary OutFlow Max=0.09 cfs @ 12.15 hrs HW=93.22' (Free Discharge) [●] —1=Culvert (Inlet Controls 0.09 cfs @ 1.09 fps)									
Summary for Pond CB-1:									
Inflow Area = 2.389 ac, 78.31% Impervious, Inflow Depth = 3.38" for 10-Yr Storm event Inflow = 8.23 cfs @ 12.08 hrs, Volume= 0.673 af Outflow = 8.23 cfs @ 12.08 hrs, Volume= 0.673 af, Atten= 0%, Lag= 0.0 min Primary = 8.23 cfs @ 12.08 hrs, Volume= 0.673 af Routed to Link SP-1 : 0.673 af									
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.68' @ 12.08 hrs Flood Elev= 95.55'									

Device	Routing	Invert	Outlet Devices
#1	Primary		15.0" Round Culvert L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 89.95' / 89.50' S= 0.0129 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=8.14 cfs @ 12.08 hrs HW=93.62' (Free Discharge) **1=Culvert** (Inlet Controls 8.14 cfs @ 6.63 fps)

Summary for Link SP-1:

 Inflow Area =
 2.389 ac, 78.31% Impervious, Inflow Depth =
 3.38" for 10-Yr Storm event

 Inflow =
 8.23 cfs @
 12.08 hrs, Volume=
 0.673 af

 Primary =
 8.23 cfs @
 12.08 hrs, Volume=
 0.673 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Link SP-2:

Inflow Are	a =	4.651 ac, 39.09% Impervious, Inflow Depth = 2.21" for 10-Yr Storm event
Inflow	=	10.68 cfs @ 12.13 hrs, Volume= 0.857 af
Primary	=	10.68 cfs @ 12.13 hrs, Volume= 0.857 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Subcatchment 1S:

Runoff = 2.57 cfs @ 12.08 hrs, Volume= 0.211 af, Depth= 5.56" Routed to Pond CB-1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

	Area (s	f) CN	Description				
*	10,47	5 98	Offsite impe	ervious			
*	9,29	2 98	Existing Im	pervious			
	5	0 74	>75% Gras	s cover, Go	ood, HSG C		
	19,81	7 98	Weighted A	verage			
	5	0	0.25% Pervious Area				
	19,76	7	99.75% lm	pervious Ar	rea		
	Tc Leng			Capacity			
(n	nin) (fee	et) (ft/	(ft) (ft/sec)	(cfs)			
	6.0				Direct Entry,		

Summary for Subcatchment 2S:

Runoff = 0.21 cfs @ 12.08 hrs, Volume= 0.017 af, Depth= 5.56" Routed to Pond 2P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

	A	rea (sf)	CN	Description		
*		784	98	Existing Imp	pervious	
*		853	98	Offsite Impe	ervious	
		1,637	98	Weighted A	verage	
		1,637		100.00% Im	npervious A	Area
	Тс	Length	Slope	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	6.0					Direct Entry,

Summary for Subcatchment 3S:

Runoff = 2.28 cfs @ 12.08 hrs, Volume= 0.181 af, Depth= 5.33" Routed to Pond 3P :

Type III 24-hr 25-Yr Storm Rainfall=5.80" Printed 6/13/2022

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	А	rea (sf)	CN	Description				
*		13,779	98	Offsite Impe	ervious			
*		2,403	98	Existing Imp	pervious			
		1,593	74	>75% Gras	s cover, Go	lood, HSG C		
		17,775	96	Weighted Average				
		1,593		8.96% Pervious Area				
		16,182		91.04% Impervious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	•		
	6.0					Direct Entry,		

Summary for Subcatchment 4S:

Runoff = 1.53 cfs @ 12.09 hrs, Volume= 0.115 af, Depth= 4.65" Routed to Pond 4P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

	Area (sf)	CN	Description		
*	7,341	98	Offsite Impe	ervious	
*	1,670	98	Existing Im	pervious	
	2,560	74	>75% Gras	s cover, Go	ood, HSG C
	1,338	65	Brush, Goo	d, HSG C	
	12,909	90	Weighted A	verage	
	3,898		30.20% Pe	rvious Area	3
	9,011		69.80% Imp	pervious Ar	rea
	Tc Lengt			Capacity	Description
(n	nin) (feet	t) (ft/	ft) (ft/sec)	(cfs)	
	6.0				Direct Entry,

Summary for Subcatchment 5S:

Runoff = 1.10 cfs @ 12.08 hrs, Volume= 0.091 af, Depth= 5.56" Routed to Pond 5P :

	Area (sf)	CN	Description			
*	5,773	98	Offsite Impervious			
*	2,744	98	Existing Impervious			
	8,517	98	Weighted Average			
	8,517		100.00% Impervious Area			

Prepare	20-001.1 Pre-Development Type III 24-hr25-Yr Storm Rainfall=5.80"Prepared by Atlantic Resource ConsultantsPrinted 6/13/2022HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLCPage 22									
Tc (min)	Length (feet)	Slop (ft/f		locity /sec)	Capacity (cfs)	Descriptior	1			
6.0						Direct Ent	ry,			
				Su	mmary fo	or Subcatc	hment 6S:			
Runoff Route	= ed to Pond		cfs @	12.0	8 hrs, Volu	ıme=	0.041 af, Dep	th= 5.56"		
	y SCS TF 24-hr 25-					ited-CN, Tim	e Span= 0.00-7	2.00 hrs, dt= 0.03 hrs		
A	rea (sf)	CN	Desc	ription						
*	3,268 630	98 98			ervious pervious					
	3,898 3,898	98			verage pervious A	vrea				
Tc (min)	Length (feet)	Slop (ft/f		locity /sec)	Capacity (cfs)	Descriptior	ı			
6.0						Direct Ent	ry,			
				Su	mmary fo	or Subcatc	hment 7S:			
Runoff Route	= ed to Pond		cfs @	12.0	9 hrs, Volu	ıme=	0.115 af, Dep	th= 4.65"		
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"										
A	rea (sf)	CN	Desc	ription						
*	9,239 2,635 1,040	98 74 65	>75%	Gras		ood, HSG C				
	1,040 65 Brush, Good, HSG C 12,914 90 Weighted Average									

9,239 71.54% Impervious Area Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)

28.46% Pervious Area

6.0

3,675

Direct Entry,

Summary for Subcatchment 8S:

Runoff = 1.43 cfs @ 12.00 hrs, Volume= 0.087 af, Depth= 4.01" Routed to Pond 7P :

Type III 24-hr 25-Yr Storm Rainfall=5.80" Printed 6/13/2022 utions LLC Page 23

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	Area (sf)	CN	Description
*	9,056	98	Offsite Impervious
	2,284	30	Brush, Good, HSG A
	11,340	84	Weighted Average
	2,284		20.14% Pervious Area
	9,056		79.86% Impervious Area

Summary for Subcatchment 9S:

Runoff	=	0.29 cfs @	12.11 hrs,	Volume=	0.029 af, Depth= 0.98"
Routed	to Por	nd 9P :			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

	Area	ı (sf)	CN	Description			
*	4,	,183	98	Offsite Impe	ervious area	a	
	11,	,072	30	Brush, Goo	d, HSG A		
	15,	,255	49	Weighted Average			
	11,	,072		72.58% Pei	vious Area		
	4,	,183		27.42% Imp	pervious Are	ea	
					-		
		ength	Slope		Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	6.0					Direct Entry,	
						•	

Summary for Subcatchment 10S:

Runoff = 15.59 cfs @ 12.13 hrs, Volume= 1.244 af, Depth= 3.21" Routed to Link SP-2 :

	Area (sf)	CN	Description
*	10,759	98	Offsite Impervious
*	68,433	98	Existing Impervious
	36,503	39	>75% Grass cover, Good, HSG A
	6,316	30	Brush, Good, HSG A
	49,266	70	Woods, Good, HSG C
	10,861	55	Woods, Good, HSG B
*	20,473	96	Existing Gravel surface, HSG D
	202,611	76	Weighted Average
	123,419		60.91% Pervious Area
	79,192		39.09% Impervious Area

Type III 24-hr 25-Yr Storm Rainfall=5.80" Printed 6/13/2022

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.0	75	0.0200	1.28		Sheet Flow, A-B
					Smooth surfaces n= 0.011 P2= 3.10"
2.2	314	0.0210	2.33		Shallow Concentrated Flow, B-C
					Unpaved Kv= 16.1 fps
0.4	35	0.3400	1.46		Shallow Concentrated Flow, C-D
					Forest w/Heavy Litter Kv= 2.5 fps
5.0	170	0.0520	0.57		Shallow Concentrated Flow, D-E
					Forest w/Heavy Litter Kv= 2.5 fps
0.7	80	0.0100	1.95	1.95	Parabolic Channel, E-F
					W=3.00' D=0.50' Area=1.0 sf Perim=3.2'
					n= 0.035 Earth, dense weeds

9.3 674 Total

Summary for Pond 2P:

Inflow Area =		1.934 ac, 73	3.27% Impervious,	Inflow Depth = 4	4.19" for 25	-Yr Storm event		
Inflow	=	8.21 cfs @	12.08 hrs, Volume	e= 0.676 a	f			
Outflow	=	8.21 cfs @	12.08 hrs, Volume	e= 0.676 a	f, Atten= 0%	,Lag= 0.0 min		
Primary	=	8.21 cfs @	12.08 hrs, Volume	e= 0.676 a	f	-		
Routed to Pond CB-1 :								

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 98.70' @ 12.08 hrs Flood Elev= 95.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.65'	12.0" Round Culvert L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.65' / 90.00' S= 0.0103 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=8.11 cfs @ 12.08 hrs HW=98.53' (Free Discharge) **1=Culvert** (Inlet Controls 8.11 cfs @ 10.33 fps)

Summary for Pond 3P:

Inflow Area	=	1.896 ac, 7	72.74% Imperviou	s, Inflow Depth =	4.17"	for 25-Yr Storm event	
Inflow	=	8.00 cfs @	12.08 hrs, Volur	ne= 0.659) af		
Outflow	=	8.00 cfs @	12.08 hrs, Volur	ne= 0.659	af, Atte	en= 0%, Lag= 0.0 min	
Primary	=	8.00 cfs @	12.08 hrs, Volur	ne= 0.659) af	-	
Routed to Pond 2P :							

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 98.41' @ 12.08 hrs Flood Elev= 95.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.75'	12.0" Round Culvert
			L= 50.0' CPP, projecting, no headwall, Ke= 0.900

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Inlet / Outlet Invert= 90.29' / 90.75' S= -0.0092 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=7.90 cfs @ 12.08 hrs HW=98.26' (Free Discharge) -1=Culvert (Inlet Controls 7.90 cfs @ 10.06 fps)

Summary for Pond 4P:

1.488 ac, 67.72% Impervious, Inflow Depth = 3.85" for 25-Yr Storm event Inflow Area = 5.74 cfs @ 12.07 hrs, Volume= Inflow 0.477 af = 5.74 cfs @ 12.07 hrs, Volume= 0.477 af, Atten= 0%, Lag= 0.0 min Outflow = 5.74 cfs @ 12.07 hrs, Volume= = 0.477 af Primary Routed to Pond 3P :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 95.04' @ 12.07 hrs Flood Elev= 96.14'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.85'	12.0" Round Culvert L= 77.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.85' / 90.29' S= 0.0073 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=5.67 cfs @ 12.07 hrs HW=94.96' (Free Discharge) **1=Culvert** (Inlet Controls 5.67 cfs @ 7.22 fps)

Summary for Pond 5P:

Inflow Area = 1.192 ac, 67.20% Impervious, Inflow Depth = 3.65" for 25-Yr Storm event 4.23 cfs @ 12.07 hrs, Volume= Inflow = 0.363 af 4.23 cfs @ 12.07 hrs, Volume= Outflow = 0.363 af, Atten= 0%, Lag= 0.0 min 4.23 cfs @ 12.07 hrs, Volume= Primarv = 0.363 af Routed to Pond 4P :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.64' @ 12.07 hrs Flood Elev= 95.65'

Device	Routing	Invert	Outlet Devices
#1	Primary	91.13'	12.0" Round Culvert L= 70.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 91.13' / 90.85' S= 0.0040 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf
#2	Primary	95.65'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=4.20 cfs @ 12.07 hrs HW=93.60' (Free Discharge)

-1=Culvert (Inlet Controls 4.20 cfs @ 5.34 fps)

-2=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 6P:

Inflow Area =

0.350 ac, 85.01% Impervious, Inflow Depth = 4.41" for 25-Yr Storm event Inflow 1.76 cfs @ 12.01 hrs. Volume= 0.128 af = 1.76 cfs @ 12.01 hrs, Volume= Outflow 0.128 af, Atten= 0%, Lag= 0.0 min = 1.76 cfs @ 12.01 hrs, Volume= Primary = 0.128 af Routed to Pond 5P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.68' @ 12.01 hrs Flood Elev= 96.08' Invert Device Routing **Outlet Devices** #1 Primary 91.83' 12.0" Round Culvert L= 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=1.73 cfs @ 12.01 hrs HW=92.67' (Free Discharge) **1=Culvert** (Inlet Controls 1.73 cfs @ 2.46 fps) Summary for Pond 7P: 0.260 ac, 79.86% Impervious, Inflow Depth = 4.01" for 25-Yr Storm event Inflow Area = Inflow 1.43 cfs @ 12.00 hrs, Volume= 0.087 af = 1.43 cfs @ 12.00 hrs, Volume= Outflow = 0.087 af, Atten= 0%, Lag= 0.0 min Primary = 1.43 cfs @ 12.00 hrs, Volume= 0.087 af Routed to Pond 6P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.22' @ 12.00 hrs Flood Elev= 97.40' Routing Invert Outlet Devices Device 92.43' 12.0" Round Culvert #1 Primary L= 105.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.43' / 92.03' S= 0.0038 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=1.42 cfs @ 12.00 hrs HW=93.21' (Free Discharge) **1=Culvert** (Barrel Controls 1.42 cfs @ 2.96 fps) Summary for Pond 8P: Inflow Area = 0.647 ac, 47.65% Impervious, Inflow Depth = 2.66" for 25-Yr Storm event Inflow = 1.81 cfs @ 12.09 hrs, Volume= 0.143 af 1.81 cfs @ 12.09 hrs, Volume= Outflow = 0.143 af, Atten= 0%, Lag= 0.0 min 1.81 cfs @ 12.09 hrs, Volume= 0.143 af Primary = Routed to Pond 5P:

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

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Peak Elev= 93.03' @ 12.09 hrs Flood Elev= 97.36'

Device Device Invest Outlet Device								
DeviceRoutingInvertOutlet Devices#1Primary92.16' 12.0" Round Culvert L= 85.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.16' / 91.23' S= 0.0109 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf								
Primary OutFlow Max=1.81 cfs @ 12.09 hrs HW=93.03' (Free Discharge) 1=Culvert (Inlet Controls 1.81 cfs @ 2.50 fps)								
Summary for Pond 9P:								
Inflow Area = 0.350 ac, 27.42% Impervious, Inflow Depth = 0.98" for 25-Yr Storm event Inflow = 0.29 cfs @ 12.11 hrs, Volume= 0.029 af Outflow = 0.29 cfs @ 12.11 hrs, Volume= 0.029 af, Atten= 0%, Lag= 0.0 min Primary = 0.29 cfs @ 12.11 hrs, Volume= 0.029 af Routed to Pond 8P : 0.029 cfs 12.11 hrs, Volume=								
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.36' @ 12.11 hrs Flood Elev= 96.37'								
Device Routing Invert Outlet Devices								
#1 Primary 93.06' 12.0'' Round Culvert L= 72.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 93.06' / 92.16' S= 0.0125 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf								
Primary OutFlow Max=0.29 cfs @ 12.11 hrs HW=93.36' (Free Discharge)								
Summary for Pond CB-1:								
Inflow Area = 2.389 ac, 78.31% Impervious, Inflow Depth = 4.46" for 25-Yr Storm event Inflow = 10.77 cfs @ 12.08 hrs, Volume= 0.887 af Outflow = 10.77 cfs @ 12.08 hrs, Volume= 0.887 af, Atten= 0%, Lag= 0.0 min Primary = 10.77 cfs @ 12.08 hrs, Volume= 0.887 af Routed to Link SP-1 : 12.08 hrs, Volume= 0.887 af								
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 95.90' @ 12.08 hrs Flood Elev= 95.55'								

Device	Routing	Invert	Outlet Devices
#1	Primary	89.95'	15.0" Round Culvert
			L= 35.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 89.95' / 89.50' S= 0.0129 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=10.65 cfs @ 12.08 hrs HW=95.79' (Free Discharge) ☐ 1=Culvert (Inlet Controls 10.65 cfs @ 8.68 fps)

Summary for Link SP-1:

 Inflow Area =
 2.389 ac, 78.31% Impervious, Inflow Depth = 4.46" for 25-Yr Storm event

 Inflow =
 10.77 cfs @ 12.08 hrs, Volume=
 0.887 af

 Primary =
 10.77 cfs @ 12.08 hrs, Volume=
 0.887 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Link SP-2:

Inflow Are	a =	4.651 ac, 39.09% Impervious, Inflow Depth = 3.21" for 25-Yr Storm event
Inflow	=	15.59 cfs @ 12.13 hrs, Volume= 1.244 af
Primary	=	15.59 cfs @ 12.13 hrs, Volume= 1.244 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Subcatchment 1S:

Runoff = 3.60 cfs @ 12.08 hrs, Volume= 0.298 af, Depth= 7.86" Routed to Pond CB-1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

	Area (sf)	CN	Description				
*	10,475	98	Offsite impe	ervious			
*	9,292	98	Existing Im	pervious			
	50	74	>75% Gras	s cover, Go	ood, HSG C		
	19,817	98	Weighted A	verage			
	50		0.25% Pervious Area				
	19,767		99.75% Impervious Area				
	Tc Length			Capacity			
(r	nin) (feet)	(ft/f	t) (ft/sec)	(cfs)			
	6.0				Direct Entry,		

Summary for Subcatchment 2S:

Runoff = 0.30 cfs @ 12.08 hrs, Volume= 0.025 af, Depth= 7.86" Routed to Pond 2P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

	A	rea (sf)	CN	Description			
*		784	98	Existing Imp	pervious		
*		853	98	Offsite Impe	ervious		
		1,637	98	Weighted A	verage		
		1,637		100.00% Impervious Area			
	Тс	Length	Slope	e Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)		
	6.0					Direct Entry,	

Summary for Subcatchment 3S:

Runoff = 3.20 cfs @ 12.08 hrs, Volume= 0.259 af, Depth= 7.62" Routed to Pond 3P :

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	А	rea (sf)	CN	Description				
*		13,779	98	Offsite Impe	ervious			
*		2,403	98	Existing Im	pervious			
		1,593	74	>75% Gras	s cover, Go	iood, HSG C		
		17,775	96	Neighted A	verage			
		1,593	1	3.96% Perv	ious Area			
		16,182	9	91.04% Impervious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)			
	6.0					Direct Entry,		

Summary for Subcatchment 4S:

Runoff = 2.23 cfs @ 12.09 hrs, Volume= 0.171 af, Depth= 6.90" Routed to Pond 4P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

	Area (sf)	CN	Description			
*	7,341	98	Offsite Impe	ervious		
*	1,670	98	Existing Imp	pervious		
	2,560	74	>75% Gras	s cover, Go	ood, HSG C	
	1,338	65	Brush, Goo	d, HSG C		
	12,909	90	Weighted A	verage		
	3,898		30.20% Per	vious Area	3	
	9,011		69.80% Impervious Area			
	Tc Length			Capacity	Description	
(m	nin) (feet)	(ft/1	ft) (ft/sec)	(cfs)		
	6.0				Direct Entry,	

Summary for Subcatchment 5S:

Runoff = 1.55 cfs @ 12.08 hrs, Volume= 0.128 af, Depth= 7.86" Routed to Pond 5P :

	Area (sf)	CN	Description	
*	5,773	98	Offsite Impervious	
*	2,744	98	Existing Impervious	
	8,517	98	Weighted Average	
	8,517		100.00% Impervious Area	

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
6.0					Direct Ent	ry,			
	Summary for Subcatchment 6S:								
Runoff Routed	= to Pond		cfs @ 12.0)8 hrs, Volu	ume=	0.059 af, De	epth= 7.86"		
Runoff by S Type III 24-					nted-CN, Tim	e Span= 0.00	-72.00 hrs, dt= 0.03 hrs		
Area	a (sf)	CN	Description	า					
* 3	8,268	98	Offsite Imp						
<u>*</u>	630	98	Existing Im						
	3,898 3,898	98	Weighted A 100.00% In	average npervious A	Area				
		<u>.</u>		·					
	ength (feet)	Slop (ft/fl		Capacity (cfs)	Descriptior	1			
6.0	(1001)	(101)	.) (14000)	(010)	Direct Ent	ry,			
			6.			hmant 70.			
			30	ininary ic	or Subcald	hment 7S:			
Runoff Routed	= to Pond		cfs @ 12.0)9 hrs, Volu	ume=	0.171 af, De	epth= 6.90"		
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"									
Area	a (sf)	CN	Description	ı					
-),239	98	Offsite Imp	ervious					
2	2,635	74	>75% Gra	ss cover, Go	ood, HSG C				
	,040	65	Brush, Go						
12	2.914	90	Weighted /	-veraue					

_		1,040	65	Brush, Good, HSG C
_		12,914	90	Weighted Average
		3,675		28.46% Pervious Area
		9,239		71.54% Impervious Area
	Тс	Lenath	Slop	e Velocity Capacity Description

-	6.0	(1001)	(1011)	(14000)	(010)	Direct Entry,	-
		(feet)		(ft/sec)	(cfs)	Description	
	Tr	Lenath	Slone	Velocity	Canacity	Description	

Direct Entry,

Summary for Subcatchment 8S:

2.17 cfs @ 12.00 hrs, Volume= Runoff 0.134 af, Depth= 6.19" = Routed to Pond 7P :

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	Area (sf)	CN	Description
*	9,056	98	Offsite Impervious
	2,284	30	Brush, Good, HSG A
	11,340	84	Weighted Average
	2,284		20.14% Pervious Area
	9,056		79.86% Impervious Area

Summary for Subcatchment 9S:

Runoff	=	0.82 cfs @	12.10 hrs,	Volume=	0.064 af,	Depth= 2.21
Routed	to Pond	19P:				-

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

	Area (sf)	CN	Description		
*	4,183	98	Offsite Impe	ervious area	a
	11,072	30	Brush, Goo	d, HSG A	
	15,255	49	Weighted A	verage	
	11,072		72.58% Pervious Area		
	4,183		27.42% lmp	pervious Ar	rea
	Tc Length			Capacity	Description
	(min) (feet)) (ft/	ft) (ft/sec)	(cfs)	
	6.0				Direct Entry,
					•

Summary for Subcatchment 10S:

25.36 cfs @ 12.13 hrs, Volume= 2.035 af, Depth= 5.25" Runoff = Routed to Link SP-2 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

	Area (sf)	CN	Description
*	10,759	98	Offsite Impervious
*	68,433	98	Existing Impervious
	36,503	39	>75% Grass cover, Good, HSG A
	6,316	30	Brush, Good, HSG A
	49,266	70	Woods, Good, HSG C
	10,861	55	Woods, Good, HSG B
*	20,473	96	Existing Gravel surface, HSG D
	202,611	76	Weighted Average
	123,419		60.91% Pervious Area
	79,192		39.09% Impervious Area

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Type III 24-hr 100-Yr Storm Rainfall=8.10" Printed 6/13/2022

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.0	75	0.0200	1.28		Sheet Flow, A-B
					Smooth surfaces n= 0.011 P2= 3.10"
2.2	314	0.0210	2.33		Shallow Concentrated Flow, B-C
					Unpaved Kv= 16.1 fps
0.4	35	0.3400	1.46		Shallow Concentrated Flow, C-D
					Forest w/Heavy Litter Kv= 2.5 fps
5.0	170	0.0520	0.57		Shallow Concentrated Flow, D-E
					Forest w/Heavy Litter Kv= 2.5 fps
0.7	80	0.0100	1.95	1.95	Parabolic Channel, E-F
					W=3.00' D=0.50' Area=1.0 sf Perim=3.2'
					n= 0.035 Earth, dense weeds

9.3 674 Total

Summary for Pond 2P:

Inflow Are	a =	1.934 ac, 7	3.27% Impervious,	Inflow Depth =	6.27" for 100-Yr Storm even	t
Inflow	=	12.16 cfs @	12.08 hrs, Volume	= 1.010	af	
Outflow	=	12.16 cfs @	12.08 hrs, Volume	= 1.010	af, Atten= 0%, Lag= 0.0 min	
Primary	=	12.16 cfs @	12.08 hrs, Volume	= 1.010	af	
Routed to Pond CB-1 :						

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 107.72' @ 12.08 hrs Flood Elev= 95.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.65'	12.0" Round Culvert L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.65' / 90.00' S= 0.0103 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=12.02 cfs @ 12.08 hrs HW=107.36' (Free Discharge) ☐ 1=Culvert (Inlet Controls 12.02 cfs @ 15.31 fps)

Summary for Pond 3P:

Inflow Are	a =	1.896 ac, 7	2.74% Impervious,	Inflow Depth = 6.2	24" for 100-Yr Storm event
Inflow	=	11.86 cfs @	12.08 hrs, Volume	= 0.986 af	
Outflow	=	11.86 cfs @	12.08 hrs, Volume	= 0.986 af,	Atten= 0%, Lag= 0.0 min
Primary	=	11.86 cfs @	12.08 hrs, Volume	= 0.986 af	-
Routed to Pond 2P :					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 107.03' @ 12.08 hrs Flood Elev= 95.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.75'	12.0" Round Culvert
			L= 50.0' CPP, projecting, no headwall, Ke= 0.900

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Inlet / Outlet Invert= 90.29' / 90.75' S= -0.0092 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=11.73 cfs @ 12.08 hrs HW=106.68' (Free Discharge) -1=Culvert (Inlet Controls 11.73 cfs @ 14.93 fps)

Summary for Pond 4P:

1.488 ac, 67.72% Impervious, Inflow Depth = 5.86" for 100-Yr Storm event Inflow Area = 8.70 cfs @ 12.07 hrs, Volume= Inflow 0.726 af = 8.70 cfs @ 12.07 hrs, Volume= 0.726 af, Atten= 0%, Lag= 0.0 min Outflow = = 8.70 cfs @ 12.07 hrs, Volume= 0.726 af Primary Routed to Pond 3P :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 99.82' @ 12.07 hrs Flood Elev= 96.14'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.85'	12.0" Round Culvert L= 77.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.85' / 90.29' S= 0.0073 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=8.59 cfs @ 12.07 hrs HW=99.63' (Free Discharge) **1=Culvert** (Inlet Controls 8.59 cfs @ 10.94 fps)

Summary for Pond 5P:

Inflow Area = 1.192 ac, 67.20% Impervious, Inflow Depth = 5.60" for 100-Yr Storm event 6.50 cfs @ 12.07 hrs, Volume= Inflow = 0.556 af 6.50 cfs @ 12.07 hrs, Volume= Outflow = 0.556 af, Atten= 0%, Lag= 0.0 min 6.50 cfs @ 12.07 hrs, Volume= Primarv = 0.556 af Routed to Pond 4P :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 95.72' @ 12.07 hrs Flood Elev= 95.65'

Device	Routing	Invert	Outlet Devices	
#1	Primary	91.13'	12.0" Round Culvert L= 70.0' CPP, projecting, no headwall, Ke= 0.900	
			Inlet / Outlet Invert= 91.13' / 90.85' S= 0.0040 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf	
#2	Primary	95.65'	24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads	

Primary OutFlow Max=6.43 cfs @ 12.07 hrs HW=95.71' (Free Discharge)

-1=Culvert (Inlet Controls 6.03 cfs @ 7.68 fps)

-2=Orifice/Grate (Weir Controls 0.40 cfs @ 0.81 fps)

Summary for Pond 6P:

0.350 ac, 85.01% Impervious, Inflow Depth = 6.62" for 100-Yr Storm event

Inflow Area =

Inflow 2.63 cfs @ 12.01 hrs. Volume= 0.193 af = 2.63 cfs @ 12.01 hrs, Volume= Outflow = 0.193 af, Atten= 0%, Lag= 0.0 min 2.63 cfs @ 12.01 hrs, Volume= Primary = 0.193 af Routed to Pond 5P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.10' @ 12.01 hrs Flood Elev= 96.08' Invert Device Routing **Outlet Devices** #1 Primary 91.83' 12.0" Round Culvert L= 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=2.58 cfs @ 12.01 hrs HW=93.08' (Free Discharge) 1=Culvert (Inlet Controls 2.58 cfs @ 3.29 fps) Summary for Pond 7P: 0.260 ac, 79.86% Impervious, Inflow Depth = 6.19" for 100-Yr Storm event Inflow Area = Inflow 2.17 cfs @ 12.00 hrs, Volume= 0.134 af = 2.17 cfs @ 12.00 hrs, Volume= Outflow = 0.134 af, Atten= 0%, Lag= 0.0 min Primarv = 2.17 cfs @ 12.00 hrs, Volume= 0.134 af Routed to Pond 6P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.49' @ 12.00 hrs Flood Elev= 97.40' Routing Invert Outlet Devices Device 92.43' 12.0" Round Culvert #1 Primary L= 105.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.43' / 92.03' S= 0.0038 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=2.16 cfs @ 12.00 hrs HW=93.48' (Free Discharge) **1=Culvert** (Barrel Controls 2.16 cfs @ 3.24 fps) Summary for Pond 8P: Inflow Area = 0.647 ac, 47.65% Impervious, Inflow Depth = 4.36" for 100-Yr Storm event Inflow = 3.04 cfs @ 12.09 hrs, Volume= 0.235 af = 3.04 cfs @ 12.09 hrs, Volume= Outflow 0.235 af, Atten= 0%, Lag= 0.0 min 3.04 cfs @ 12.09 hrs, Volume= Primary = 0.235 af Routed to Pond 5P:

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Peak Elev= 93.70' @ 12.09 hrs Flood Elev= 97.36'

	Routing		Outlet Devices
#1	Primary	92.16'	
			L= 85.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.16' / 91.23' S= 0.0109 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf
Duine			
	Ivert (Inlet Cont		② 12.09 hrs HW=93.69' (Free Discharge) cfs @ 3.86 fps)
			Summary for Pond 9P:
Inflow A	rea = 0.35	0 ac, 27.	42% Impervious, Inflow Depth = 2.21" for 100-Yr Storm event
Inflow	= 0.82	cfs @ 12	2.10 hrs, Volume= 0.064 af
Outflow Primary			2.10 hrs, Volume= 0.064 af, Atten= 0%, Lag= 0.0 min 2.10 hrs, Volume= 0.064 af
	ed to Pond 8P :		
•	by Stor-Ind meth ev= 93.59' @ 12.		Span= 0.00-72.00 hrs, dt= 0.03 hrs
	lev= 95.39 @ 12.	101113	
-	Routing	Invert	
#1	Primary	93.06'	12.0" Round Culvert L= 72.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 93.06' / 92.16' S= 0.0125 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf
Primarv	OutFlow Max=	0 81 cfs (ᡚ 12.10 hrs HW=93.58' (Free Discharge)
	Ivert (Inlet Cont		
			Summary for Pond CB-1:
Inflow A	rea = 2.38	9 ac. 78.	31% Impervious, Inflow Depth = 6.57" for 100-Yr Storm event
Inflow	= 15.75	cfs @ 12	2.08 hrs, Volume= 1.308 af
Outflow	= 15.75	cfs @ 12	2.08 hrs, Volume= 1.308 af, Atten= 0%, Lag= 0.0 min
Primary Route	ed to Link SP-1 :	cts @ 12	2.08 hrs, Volume= 1.308 af
			Span= 0.00-72.00 hrs, dt= 0.03 hrs
	ev= 101.96' @ 12 lev= 95.55'	2.08 nrs	
	00.00		
Device	Routing	Invert	
#1	Primary	89.95'	15.0" Round Culvert
			L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 89.95' / 89.50' S= 0.0129 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=15.57 cfs @ 12.08 hrs HW=101.71' (Free Discharge) **1=Culvert** (Inlet Controls 15.57 cfs @ 12.68 fps)

Summary for Link SP-1:

 Inflow Area =
 2.389 ac, 78.31% Impervious, Inflow Depth =
 6.57" for 100-Yr Storm event

 Inflow =
 15.75 cfs @
 12.08 hrs, Volume=
 1.308 af

 Primary =
 15.75 cfs @
 12.08 hrs, Volume=
 1.308 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

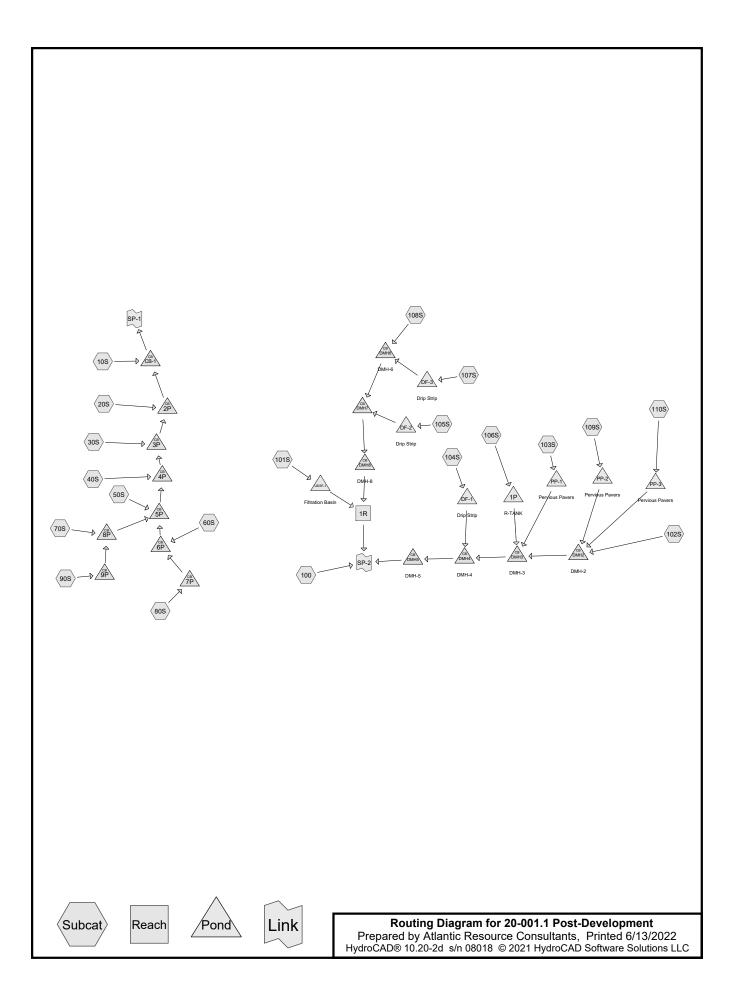
Summary for Link SP-2:

Inflow Are	a =	4.651 ac, 39.09% Impervious, Inflow Depth = 5.25" for 100-Yr Storm event
Inflow	=	25.36 cfs @ 12.13 hrs, Volume= 2.035 af
Primary	=	25.36 cfs @ 12.13 hrs, Volume= 2.035 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

ATTACHMENT B (II) -POST-DEVELOPMENT MODEL RESULTS





Summary for Subcatchment 10S:

Runoff = 1.36 cfs @ 12.08 hrs, Volume= 0.109 af, Depth= 2.87" Routed to Pond CB-1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

	A	rea (sf)	CN	Description		
*		10,475	98	Offsite impe	ervious	
*		9,057	98	Existing Im	pervious	
		285	74	>75% Gras	s cover, Go	ood, HSG C
		19,817		Weighted A		
		285		1.44% Perv		
		19,532		98.56% lmp	pervious Ar	rea
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	
	6.0					Direct Entry,

Summary for Subcatchment 20S:

Runoff = 0.11 cfs @ 12.08 hrs, Volume= 0.009 af, Depth= 2.87" Routed to Pond 2P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

	A	rea (sf)	CN	Description		
*		784	98	Existing Imp	pervious	
*		853	98	Offsite Impe	ervious	
		1,637	98	Weighted A	verage	
		1,637		100.00% Im	npervious A	Area
	Тс	Length	Slope	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	6.0					Direct Entry,

Summary for Subcatchment 30S:

Runoff = 1.18 cfs @ 12.09 hrs, Volume= 0.090 af, Depth= 2.65" Routed to Pond 3P :

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022

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	Area (sf)	CN	Description					
*	13,779	98	Offsite Impervious					
*	2,496	98	Impervious					
*	746	74	Offsite Landscaped					
	754	74	>75% Grass cover, Good, HSG C					
	17,775	96	Weighted Average					
	1,500		8.44% Pervious Area					
	16,275		91.56% Impervious Area					
	Tc Length							
((min) (feet)	(ft/	ft) (ft/sec) (cfs)					
	6.0		Dive at Fratma					

6.0

Direct Entry,

Summary for Subcatchment 40S:

Runoff	=	0.61 cfs @	12.09 hrs,	Volume=	0.044 af,	Depth= 2.08"
Routed	to Pond	4P:				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

	A	rea (sf)	CN	CN Description					
*		7,341	98	Offsite Impe	ervious				
*		806	98	Existing Im	pervious				
		1,646	74	>75% Gras	s cover, Go	ood, HSG C			
		1,338	65	65 Brush, Good, HSG C					
		11,131	90	Weighted A	verage				
		2,984		26.81% Pervious Area					
		8,147		73.19% lmp	pervious Ar	ea			
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
	6.0					Direct Entry,			

Summary for Subcatchment 50S:

Runoff 0.52 cfs @ 12.08 hrs, Volume= 0.041 af, Depth= 2.87" = Routed to Pond 5P :

	Area (sf)	CN	Description
*	5,773	98	Offsite Impervious
*	1,775	98	Existing Impervious
	7,548	98	Weighted Average
	7,548		100.00% Impervious Area

20-001.1 Post-Development Type III 24-hr2-Yr Storm Rainfall=3.10"Prepared by Atlantic Resource ConsultantsPrinted 6/13/2022HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLCPage 4					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
6.0 Direct Entry,					
Summary for Subcatchment 60S:					
Runoff = 0.41 cfs @ 12.08 hrs, Volume= 0.033 af, Depth= 2.87" Routed to Pond 6P :					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"					
Area (sf) CN Description					
 * 3,268 98 Offsite Impervious * 2,700 98 Existing Impervious 					
5,96898Weighted Average5,968100.00% Impervious Area					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
6.0 Direct Entry,					
Summary for Subcatchment 70S:					
Runoff = 0.71 cfs @ 12.09 hrs, Volume= 0.051 af, Depth= 2.08" Routed to Pond 8P :					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"					
Area (sf) CN Description					
* 9,239 98 Offsite Impervious 2,635 74 >75% Grass cover, Good, HSG C 1.040 65 Brush, Good, HSG C					

_	1,040	65	Brush, Good, HSG C	
	12,914	90	Weighted Average	
	3,675		28.46% Pervious Area	
	9,239		71.54% Impervious Area	
	Tc Length	Slop	be Velocity Capacity Description	

(min) (feet) (ft/ft) (ft/sec) (cfs)
Tc Length Slope Velocity Capacity Description

Summary for Subcatchment 80S:

0.58 cfs @ 12.00 hrs, Volume= 0.035 af, Depth= 1.60" Runoff = Routed to Pond 7P :

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022 tions LLC Page 5

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	Area (sf)	CN	Description
*	9,056	98	Offsite Impervious
	2,284	30	Brush, Good, HSG A
	11,340	84	Weighted Average
	2,284		20.14% Pervious Area
	9,056		79.86% Impervious Area

Summary for Subcatchment 90S:

Runoff	=	0.00 cfs @	13.80 hrs,	Volume=	0.003 af,	Depth= 0.09"
Routed	to Pond	19P:				•

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

	Area (s	sf) CN		Description		
*	4,18	33 98	3 C	Offsite Impe	ervious area	a
_	11,07	72 30) B	Brush, Goo	d, HSG A	
	15,25	55 49	9 V	Veighted A	verage	
	11,07	72	7	2.58% Per	vious Area	1
	4,18	33	2	7.42% Imp	pervious Are	ea
	Tc Len		lope	Velocity	Capacity	Description
_	(min) (fe	et) (ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,
						•

Summary for Subcatchment 100:

Runoff = 1.46 cfs @ 12.29 hrs, Volume= 0.159 af, Depth= 1.20" Routed to Link SP-2 :

	Area (sf)	CN	Description
	41,443	74	>75% Grass cover, Good, HSG C
*	16,896	74	OFSITE AREA TO BE UPDATED
*	10,765	98	Offsite Impervious
	69,104	78	Weighted Average
	58,339		84.42% Pervious Area
	10,765		15.58% Impervious Area

20-001.1 Post-Development	Type III 24-hr 2-Yr Storm Rainfall=3.10"
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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.3	75	0.0930	0.08		Sheet Flow, A-B
3.2	110	0.0540	0.58		Woods: Dense underbrush n= 0.800 P2= 3.10" Shallow Concentrated Flow, B-C Forest w/Heavy Litter Kv= 2.5 fps
0.6	70	0.0100	1.95	1.95	Parabolic Channel, C-D W=3.00' D=0.50' Area=1.0 sf Perim=3.2' n= 0.035
20.1	255	Total			

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Summary for Subcatchment 101S:

Runoff = 0.78 cfs @ 12.09 hrs, Volume= 0.056 af, Depth= 1.53" Routed to Pond UDSF-1 : Filtration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

Are	ea (sf)	CN [Description						
8	8,017	98 F	Paved parki	ing, HSG D	D				
	3,766	70 \	Noods, Goo	od, HSG C					
	7,218	74 >	>75% Grass	s cover, Go	ood, HSG C				
19	9,001	83 \	Veighted A	verage					
10	0,984		57.81% Per		a				
8	8,017	2	42.19% Impervious Area						
Tc L (min)	_ength (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0			Direct Entry,						

Summary for Subcatchment 102S:

Runoff 0.41 cfs @ 12.09 hrs, Volume= 0.029 af, Depth= 1.91" = Routed to Pond DMH2 : DMH-2

A	rea (sf)	CN	Description					
	4,611	98	Roofs, HSG	5 D				
	3,387	74	>75% Gras	s cover, Go	ood, HSG C			
	7,998	88	Weighted Average					
	3,387		42.35% Per	vious Area	3			
	4,611		57.65% Impervious Area					
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment 103S:

Runoff = 1.25 cfs @ 12.09 hrs, Volume= 0.096 af, Depth= 2.65" Routed to Pond PP-1 : Pervious Pavers

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

A	rea (sf)	CN I	Description					
	17,688	98 I	Paved park	ing, HSG D				
	1,185	74 >	>75% Gras	s cover, Go	ood, HSG C			
	18,873	96 \	Neighted A	verage				
	1,185	6	6.28% Perv	ious Area				
	17,688	ę	93.72% Imp	ervious Ar	ea			
Tc	Length	Slope		Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	ft) (ft/sec) (cfs)					
6.0			Direct Entry,					
					-			

Summary for Subcatchment 104S:

Runoff = 1.41 cfs @ 12.08 hrs, Volume= Routed to Pond DF-1 : Drip Strip 0.113 af, Depth= 2.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

	A	rea (sf)	CN	Description		
		18,803	98	Roofs, HSG	G D	
*		1,786	98	Stone		
		20,589	98	Weighted A	verage	
		20,589		100.00% In	npervious A	Area
	Tc	Length	Slope	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	6.0					Direct Entry,
						-

Summary for Subcatchment 105S:

Runoff = 0.67 cfs @ 12.08 hrs, Volume= 0.053 af, Depth= 2.87" Routed to Pond DF-2 : Drip Strip

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022

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	A	rea (sf)	CN	Description					
		8,810	98	Roofs, HSG	G D				
*		896	98	Stone					
		9,706 9,706	98	Weighted Average 100.00% Impervious Area					
	Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description			
_	6.0					Direct Entry,			

Summary for Subcatchment 106S:

Runoff = 2.13 cfs @ 12.09 hrs, Volume= 0.153 af, Depth= 1.99" Routed to Pond 1P : R-TANK

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

A	rea (sf)	CN	Description		
	21,170	98	Paved parki	ing, HSG D)
	3,223	98	Roofs, HSG	δĎ	
	15,887	74	>75% Grass	s cover, Go	ood, HSG C
	40,280	89	Weighted A	verage	
	15,887		39.44% Per	vious Area	a
	24,393		60.56% Imp	ervious Ar	rea
_		-		- ··	
Тс	Length	Slope		Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,
					•

Summary for Subcatchment 107S:

Runoff = 0.31 cfs @ 12.09 hrs, Volume= 0.022 af, Depth= 2.16" Routed to Pond DF-3 : Drip Strip

	Area (sf)	CN	Description
	3,214	98	Roofs, HSG D
*	480	98	Stone
	1,665	74	>75% Grass cover, Good, HSG C
	5,359	91	Weighted Average
	1,665		31.07% Pervious Area
	3,694		68.93% Impervious Area

20-001.1 Post-Development Type III 24-hr2-Yr Storm Rainfall=3.10"Prepared by Atlantic Resource ConsultantsPrinted 6/13/2022HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLCPage 9
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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
6.0 Direct Entry,
Summary for Subcatchment 108S:
Runoff = 0.10 cfs @ 12.10 hrs, Volume= 0.008 af, Depth= 0.87" Routed to Pond DMH6 : DMH-6
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"
Area (sf) CN Description
2,447 74 >75% Grass cover, Good, HSG C
2,448 70 Woods, Good, HSG C
4,895 72 Weighted Average
4,895 100.00% Pervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
6.0 Direct Entry,

Summary for Subcatchment 109S:

0.17 cfs @ 12.09 hrs, Volume= 0.013 af, Depth= 2.65" Runoff = Routed to Pond PP-2 : Pervious Pavers

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 2-Yr Storm Rainfall=3.10"

A	rea (sf)	CN	Description		
	2,326	98	Paved park	ing, HSG D	D
	250	74	>75% Ġras	s cover, Go	lood, HSG C
	2,576	96	Weighted A	verage	
	250		9.70% Perv	rious Area	
	2,326		90.30% Imp	pervious Ar	rea
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	I
6.0				()	Direct Entry,

Summary for Subcatchment 110S:

Runoff	=	0.34 cfs @	12.08 hrs,	Volume=	0.0)27 af,	Depth= 2.87"	
Routed	l to Pond	d PP-3 : Pervi	ous Pavers					

Type III 24-hr2-Yr Storm Rainfall=3.10"Printed6/13/2022tions LLCPage 10

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Area (sf) CN Description
4,907 98 Paved parking, HSG D
4,907 100.00% Impervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
6.0 Direct Entry,
Oursen and fan Daaak 4D
Summary for Reach 1R:
Inflow Area = 0.894 ac, 54.97% Impervious, Inflow Depth = 1.84" for 2-Yr Storm event Inflow = 1.25 cfs @ 12.16 hrs, Volume= 0.137 af
Outflow = 1.24 cfs @ 12.19 hrs, Volume= 0.137 af, Atten= 1%, Lag= 1.7 min Routed to Link SP-2 :
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Max. Velocity= 1.23 fps, Min. Travel Time= 2.4 min Avg. Velocity = 0.35 fps, Avg. Travel Time= 8.3 min
Peak Storage= 175 cf @ 12.19 hrs Average Depth at Peak Storage= 0.05' , Surface Width= 20.50' Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 60.16 cfs
20.00' x 0.50' deep channel, n= 0.035 Earth, dense weeds Side Slope Z-value= 5.0 '/' Top Width= 25.00' Length= 173.0' Slope= 0.0462 '/' Inlet Invert= 80.00', Outlet Invert= 72.00'
±
Summary for Pond 1P: R-TANK
Inflow Area = 0.925 ac. 60.56% Impervious. Inflow Depth = 1.99" for 2-Yr Storm event

Inflow Area	a =	0.925 ac, 6	0.56% Imp	ervious,	Inflow Depth =	1.99'	' for 2-Yr	Storm event
Inflow	=	2.13 cfs @	12.09 hrs,	Volume	= 0.153	af		
Outflow	=	0.74 cfs @	12.38 hrs,	Volume	= 0.153	af, A	tten= 65%,	Lag= 17.3 min
Primary	=	0.74 cfs @	12.38 hrs,	Volume	= 0.153	af		-
Routed	to Pond	I DMH3 : DMI	H-3					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 88.60' @ 12.38 hrs Surf.Area= 3,593 sf Storage= 1,542 cf Flood Elev= 91.30' Surf.Area= 3,593 sf Storage= 6,075 cf

Plug-Flow detention time= 27.5 min calculated for 0.153 af (100% of inflow) Center-of-Mass det. time= 27.6 min (839.9 - 812.3)

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022

Prepared by Atlantic Resource Consultants HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLC

Volume	Invert	Avail.Storage	Storage Description
#1	85.30'	1,131 cf	Custom Stage Data (Prismatic)Listed below (Recalc)
#2A	88.22'	1,396 cf	31.56'W x 62.65'L x 4.07'H Field A
			8,049 cf Overall - 4,560 cf Embedded = 3,489 cf x 40.0% Voids
#3A	88.47'	4,332 cf	ACF R-Tank HD 2 x 525 Inside #2
			Inside= 15.7"W x 33.9"H => 3.52 sf x 2.35'L = 8.3 cf
			Outside= 15.7"W x 33.9"H => 3.70 sf x 2.35'L = 8.7 cf
			525 Chambers in 21 Rows
		6 859 cf	Total Available Storage

6,859 ct I otal Available Storage

Storage Group A created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.30	1,616	0.0	0	0
85.63	1,616	40.0	213	213
85.97	1,616	20.0	110	323
86.47	1,616	20.0	162	485
86.97	1,616	20.0	162	646
87.47	1,616	20.0	162	808
87.97	1,616	20.0	162	970
88.47	1,616	20.0	162	1,131

Device	Routing	Invert	Outlet Devices
#1	Primary	85.10'	15.0" Round Culvert
			L= 126.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 85.10' / 83.50' S= 0.0127 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.23 sf
#2	Device 1	85.30'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	90.60'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
			4.0' Crest Height
#4	Device 1	90.00'	12.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.74 cfs @ 12.38 hrs HW=88.60' (Free Discharge)

-**1=Culvert** (Passes 0.74 cfs of 7.90 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.74 cfs @ 8.52 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 2P:

Inflow Area = 1.918 ac, 74.25% Impervious, Inflow Depth = 1.91" for 2-Yr Storm event Inflow = 3.86 cfs @ 12.08 hrs, Volume= 0.306 af

 3.86 cfs @
 12.08 hrs, Volume=
 0.306 af, Atten= 0%, Lag= 0.0 min

 3.86 cfs @
 12.08 hrs, Volume=
 0.306 af

 Outflow = Primary = Routed to Pond CB-1:

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.82' @ 12.08 hrs Flood Elev= 95.86'

20-001.1 Post-Development Prepared by Atlantic Resource Consultants

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022 HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLC Page 12

Device Routing Invert Outlet Devices
#1 Primary 90.65' 12.0" Round Culvert L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.65' / 90.00' S= 0.0103 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf
Primary OutFlow Max=3.82 cfs @ 12.08 hrs HW=92.78' (Free Discharge) 1=Culvert (Inlet Controls 3.82 cfs @ 4.86 fps)
Summary for Pond 3P:
Inflow Area = 1.881 ac, 73.74% Impervious, Inflow Depth = 1.90" for 2-Yr Storm event Inflow = 3.75 cfs @ 12.08 hrs, Volume= 0.297 af Outflow = 3.75 cfs @ 12.08 hrs, Volume= 0.297 af, Atten= 0%, Lag= 0.0 min Primary = 3.75 cfs @ 12.08 hrs, Volume= 0.297 af Routed to Pond 2P : 12.08 hrs, Volume= 0.297 af
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.82' @ 12.08 hrs Flood Elev= 95.40'
Device Routing Invert Outlet Devices
#1 Primary 90.75' 12.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.29' / 90.75' S= -0.0092 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf
Primary OutFlow Max=3.70 cfs @ 12.08 hrs HW=92.79' (Free Discharge) ↑ 1=Culvert (Inlet Controls 3.70 cfs @ 4.72 fps)
Summary for Pond 4P:
Inflow Area = 1.473 ac, 68.80% Impervious, Inflow Depth = 1.69" for 2-Yr Storm event Inflow = 2.58 cfs @ 12.07 hrs, Volume= 0.207 af Outflow = 2.58 cfs @ 12.07 hrs, Volume= 0.207 af, Atten= 0%, Lag= 0.0 min Primary = 2.58 cfs @ 12.07 hrs, Volume= 0.207 af Routed to Pond 3P : 12.07 hrs, Volume= 0.207 af
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.10' @ 12.07 hrs Flood Elev= 96.14'
Device Routing Invert Outlet Devices
#1 Primary 90.85' 12.0'' Round Culvert L= 77.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.85' / 90.29' S= 0.0073 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=2.55 cfs @ 12.07 hrs HW=92.08' (Free Discharge) **1=Culvert** (Inlet Controls 2.55 cfs @ 3.25 fps)

HydroCADE 10 20-2d sin 08018 © 2021 HydroCAD Software Solutions LLC Page 13 Summary for Pond 5P: Inflow Area = 1.217 ac, 67.88% Impervious, Inflow Depth = 1.60" for 2-Yr Storm event Inflow = 1.98 cfs @ 12.07 hrs, Volume= 0.163 af Outflow = 1.98 cfs @ 12.07 hrs, Volume= 0.163 af Outlow = 1.98 cfs @ 12.07 hrs, Volume= 0.163 af Routed to Pond 4P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.12' @ 12.07 hrs Flood Elev= 95.65' Device Routing Invert Outlet Devices #1 Primary 91.13' 12.0" Round Culvert L= 70.0'' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.13' / 90.85'' S= 0.0040 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf #2 Primary 95.65' 24.0'' x 24.0'' Moriz. Orifice/Grate C = 0.600 Limited to weir flow at low heads Primary OutFlow Max=1.96 cfs @ 12.07 hrs, HW=92.11' (Free Discharge) -1=Culvert (Barrel Controls 1.96 cfs @ 3.17 fps) -2=Orifice/Grate (Controls 0.00 cfs) Summary for Pond 6P: -2Culvert (Barrel Controls 1.96 cfs @ 12.02 hrs, Volume= 0.067 af	20-001.1 Post-Developmen Prepared by Atlantic Resource		Type III 24-hr 2-Yr St	<i>corm Rainfall=3.10"</i> Printed 6/13/2022
Inflow Area = 1.217 ac, 67.88% Impervious, Inflow Depth = 1.60" for 2-Yr Storm event Inflow = 1.98 cfs @ 12.07 hrs, Volume= 0.163 af Outflow = 1.98 cfs @ 12.07 hrs, Volume= 0.163 af Routed to Pond 4P : Routed to Pond 4P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.12' @ 12.07 hrs Flood Elev= 95.65' Device Routing Invert Outlet Devices #1 Primary 91.13' 12.0" Round Culvert L= 70.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.13' 90.85' S= 0.0040 '/ Cc= 0.900 Intel / Outlet Invert= 91.13' 90.85' S= 0.0040 '/ Cc= 0.900 Inlet / Outlet Invert= 91.13' 90.85' S= 0.0040 '/ Cc= 0.900 #2 Primary 95.55' 24.0" x24.0" Horz. Orifice/Grate C = 0.600 Limited to weir flow at low heads Primary OutFlow Max=1.96 cfs @ 12.07 hrs HW=92.11' (Free Discharge) H=Culvert (Barrel Controls 1.96 cfs @ 3.17 fps) 220'rifice/Grate (Controls 0.00 cfs) Summary for Pond 6P: Inflow Area = 0.397 ac, 86 80% Impervious, Inflow Depth = 2.04" for 2-Yr Storm event Inflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Outf			utions LLC	
Inflow = 1.98 cfs @ 12.07 hrs, Volume= 0.163 af Outflow = 1.98 cfs @ 12.07 hrs, Volume= 0.163 af Routed to Pond 4P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.12' @ 12.07 hrs Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.12' @ 12.07 hrs Primary 91.13' 12.0" Round Culvert E = 70.0' CPP, projecting, no headwall, Ke= 0.900 Intel / Outlet Invert= 91.13' / 90.85' S= 0.0040 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf #2 Primary 95.65' 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads Primary OutFlow Max=1.96 cfs @ 12.07 hrs HW=92.11' (Free Discharge) -1=Culvert (Barrel Controls 1.96 cfs @ 3.17 fps) -2=Orifice/Grate (Controls 0.00 cfs) Summary for Pond 6P: Inflow a 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Inflow = 0.397 ac, 86.80% Impervious, Inflow Depth = 2.04" for 2-Yr Storm event Inflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Outled to Pond 5P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.37' @ 12.02 hrs Pioed Elev= 96.08' Device Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 96.0		Summary for Pond	1 5P:	
Peak Elev= 92.12 @ 12.07 hrs Flood Elev= 95.65'DeviceRoutingInvertOutlet Devices#1Primary91.13' 12.0" Round Culvert L= 70.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.13' / 90.85' S= 0.0040 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf#2Primary95.65' 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low headsPrimary OutFlow Max=1.96 cfs @ 12.07 hrsHW=92.11' (Free Discharge)-1=Culvert(Barrel Controls 1.96 cfs @ 3.17 fps)-2=Orifice/Grate(Controls 0.00 cfs)Summary for Pond 6P:Inflow Area = 0.397 ac, 86.80% Impervious, Inflow Depth = 2.04" for 2-Yr Storm event Inflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Routed to Pond 5P :Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.37' @ 12.02 hrs Flood Elev= 96.08'Device Routing Invert Outlet Devices #1#1Primary91.83'1.2.0" Round Culvert L= 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sfPrimary OutFlow Max=0.84 cfs @ 12.02 hrsPrimary OutFlow Max=0.84 cfs @ 12.02 hrsPrimary 0.0167 Cc= 0.900 n = 0.012, Flow Area= 0.79 sfPrimary 0.02 from the colspan="2">Primary 0.02 for	Inflow = 1.98 cfs @ 1.98 cfs @ Outflow = 1.98 cfs @ 1.98 cfs @ Primary = 1.98 cfs @ 1.98 cfs @	12.07 hrs, Volume= 12.07 hrs, Volume=	0.163 af 0.163 af, Atten= 0%, L	
#1 Primary 91.13' 12.0" Round Culvert L= 70.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.13' / 90.85' S= 0.0040 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf #2 Primary 95.65' 24.0" x 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads Primary OutFlow Max=1.96 cfs @ 12.07 hrs HW=92.11' (Free Discharge) 1=Culvert (Barrel Controls 1.96 cfs @ 3.17 fps) 2=Orifice/Grate (Controls 0.00 cfs) Summary for Pond 6P: Inflow Area = 0.397 ac, 86.80% Impervious, Inflow Depth = 2.04" for 2-Yr Storm event Inflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Outflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Outflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Routed to Pond 5P : Routed to Pond 5P : Routed to Pond 5P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.37' @ 12.02 hrs Flood Elev= 96.08' Device Routing Invert Outlet Devices #1 Primary 91.83' 12.0" Round Culvert L = 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Inve	Peak Elev= 92.12' @ 12.07 hrs	e Span= 0.00-72.00 hrs, dt=	0.03 hrs	
L = 70.0° CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.13' / 90.85' S= 0.0040 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf #2 Primary 95.65' 24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads Primary OutFlow Max=1.96 cfs @ 12.07 hrs HW=92.11' (Free Discharge) 1=Culvert (Barrel Controls 1.96 cfs @ 3.17 fps) 2=Orifice/Grate (Controls 0.00 cfs) Summary for Pond 6P: Inflow Area = 0.397 ac, 86.80% Impervious, Inflow Depth = 2.04" for 2-Yr Storm event Inflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Outflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Outflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Notice to Pond 5P: Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.37' @ 12.02 hrs Flood Elev= 96.08' Device Routing Invert Outlet Devices #1 Primary 91.83' 12.0" Round Culvert L = 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S=	Device Routing Invert	Outlet Devices		
L1=Culvert (Barrel Controls 1.96 cfs @ 3.17 fps) 2=Orifice/Grate (Controls 0.00 cfs) Summary for Pond 6P: Inflow Area = 0.397 ac, 86.80% Impervious, Inflow Depth = 2.04" for 2-Yr Storm event Inflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Outflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af, Atten= 0%, Lag= 0.0 min Primary = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.37' @ 12.02 hrs Flood Elev= 96.08' Device Routing Device Routing Invert Outlet Devices #1 Primary 91.83' 12.0" Round Culvert L= 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=0.84 cfs @ 12.02 hrs HW=92.37' (Free Discharge)	5	L= 70.0' CPP, projecting Inlet / Outlet Invert= 91.13 n= 0.012, Flow Area= 0.7 24.0'' x 24.0'' Horiz. Orifi	'/90.85' S= 0.0040 '/' 9 sf ce/Grate C= 0.600	
Inflow Area = 0.397 ac, 86.80% Impervious, Inflow Depth = 2.04" for 2-Yr Storm event Inflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Outflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af, Atten= 0%, Lag= 0.0 min Primary = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Routed to Pond 5P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.37' @ 12.02 hrs Flood Elev= 96.08' $\frac{Device}{\#1} \frac{Invert}{Primary} \frac{Outlet Devices}{91.83'} \frac{12.0" Round Culvert}{L= 64.0' CPP, projecting, no headwall, Ke= 0.900}{Inlet / Outlet Invert= 91.83' / 91.23' S = 0.0094 '/' Cc= 0.900}{Inlet / Outlet Invert= 91.83' / 91.23' S = 0.0094 '/' Cc= 0.900}$	1=Culvert (Barrel Controls 1.9	96 cfs @ 3.17 fps)	ree Discharge)	
Inflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Outflow = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af, Atten= 0%, Lag= 0.0 min Primary = 0.85 cfs @ 12.02 hrs, Volume= 0.067 af Routed to Pond 5P : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs 0.067 af Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.37' @ 12.02 hrs Flood Elev= 96.08'		Summary for Pone	d 6P:	
Peak Elev= 92.37' @ 12.02 hrs Flood Elev= 96.08' Device Routing Invert Outlet Devices #1 Primary 91.83' 12.0" Round Culvert L= 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=0.84 cfs @ 12.02 hrs HW=92.37' (Free Discharge)	Inflow = 0.85 cfs @ Outflow = 0.85 cfs @ Primary = 0.85 cfs @ Routed to Pond 5P :	12.02 hrs, Volume= 12.02 hrs, Volume= 12.02 hrs, Volume=	0.067 af 0.067 af, Atten= 0%, L 0.067 af	
#1 Primary 91.83' 12.0" Round Culvert L= 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=0.84 cfs @ 12.02 hrs HW=92.37' (Free Discharge)	Peak Elev= 92.37' @ 12.02 hrs	5 opan - 0.00-72.00 m3, dt-	0.00 113	
L= 64.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=0.84 cfs @ 12.02 hrs HW=92.37' (Free Discharge)	Device Routing Invert	Outlet Devices		
	#1 Primary 91.83'	L= 64.0' CPP, projecting Inlet / Outlet Invert= 91.83	'/ 91.23' S= 0.0094 '/'	
			ree Discharge)	

Summary for Pond 7P:

20-001.1 Post-Development Type III 24-hr2-Yr Storm Rainfall=3.10Prepared by Atlantic Resource ConsultantsPrinted 6/13/202HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLCPage 1					
HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLC Page 14 Inflow Area = 0.260 ac, 79.86% Impervious, Inflow Depth = 1.60" for 2-Yr Storm event Inflow = 0.58 cfs @ 12.00 hrs, Volume= 0.035 af Outflow = 0.58 cfs @ 12.00 hrs, Volume= 0.035 af, Atten= 0%, Lag= 0.0 min Primary = 0.58 cfs @ 12.00 hrs, Volume= 0.035 af Routed to Pond 6P : 0.035 af 0.035 af					
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.89' @ 12.00 hrs Flood Elev= 97.40'					
Device Routing Invert Outlet Devices #1 Primary 92.43' 12.0" Round Culvert L= 105.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.43' / 92.03' S= 0.0038 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=0.57 cfs @ 12.00 hrs HW=92.89' (Free Discharge)					
^T —1=Culvert (Barrel Controls 0.57 cfs @ 2.37 fps) Summary for Pon	nd 8P:				
Inflow Area = 0.647 ac , 47.65% Impervious, Inflow DInflow = 0.71 cfs @ 12.09 hrs , Volume=Outflow = 0.71 cfs @ 12.09 hrs , Volume=Primary = 0.71 cfs @ 12.09 hrs , Volume=Routed to Pond 5P :	0.054 af				
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.65' @ 12.09 hrs Flood Elev= 97.36'					

Device	Routing	Invert	Outlet Devices
#1	Primary	92.16'	12.0" Round Culvert L= 85.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.16' / 91.23' S= 0.0109 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=92.64' (Free Discharge) -1=Culvert (Inlet Controls 0.71 cfs @ 1.87 fps)

Summary for Pond 9P:

 Inflow Area =
 0.350 ac, 27.42% Impervious, Inflow Depth =
 0.09" for 2-Yr Storm event

 Inflow =
 0.00 cfs @
 13.80 hrs, Volume=
 0.003 af

 Outflow =
 0.00 cfs @
 13.80 hrs, Volume=
 0.003 af, Atten= 0%, Lag= 0.0 min

 Primary =
 0.00 cfs @
 13.80 hrs, Volume=
 0.003 af

 Routed to Pond 8P :
 0.00
 13.80 hrs, Volume=

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.09' @ 13.80 hrs Flood Elev= 96.37' 20-001.1 Post-Development Prepared by Atlantic Resource Consultants

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022 HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLC Page 15

Device	Routing	Invert	Outlet Devices		
#1	Primary	93.06'	12.0" Round Culvert L= 72.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 93.06' / 92.16' S= 0.0125 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf		
		Max=0.00 cfs @ t Controls 0.00 d) 13.80 hrs HW=93.09' (Free Discharge) cfs @ 0.50 fps)		
			Summary for Pond CB-1:		
Inflow Ai Inflow Outflow Primary Route	= =	5.22 cfs @ 12 5.22 cfs @ 12 5.22 cfs @ 12	91% Impervious, Inflow Depth = 2.10" for 2-Yr Storm event 2.08 hrs, Volume= 0.415 af 2.08 hrs, Volume= 0.415 af, Atten= 0%, Lag= 0.0 min 2.08 hrs, Volume= 0.415 af		
Peak Ele		l method, Time @ 12.08 hrs	Span= 0.00-72.00 hrs, dt= 0.03 hrs		
Device	Routing	Invert	Outlet Devices		
#1	Primary	89.95'	15.0" Round Culvert L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 89.95' / 89.50' S= 0.0129 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf		
	Primary OutFlow Max=5.16 cfs @ 12.08 hrs HW=91.80' (Free Discharge) ←1=Culvert (Inlet Controls 5.16 cfs @ 4.20 fps)				
		Su	Immary for Pond DF-1: Drip Strip		
Inflow Ai Inflow Outflow Primary Route	= = =	1.41 cfs @ 12 0.78 cfs @ 12	00% Impervious, Inflow Depth = 2.87" for 2-Yr Storm event 2.08 hrs, Volume= 0.113 af 2.21 hrs, Volume= 0.109 af, Atten= 45%, Lag= 7.4 min 2.21 hrs, Volume= 0.109 af		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 83.50' @ 12.21 hrs Surf.Area= 1,786 sf Storage= 894 cf

Plug-Flow detention time= 58.8 min calculated for 0.109 af (96% of inflow) Center-of-Mass det. time= 36.7 min (793.8 - 757.1)

Volume	Invert	Avail.Storage	Storage Description
#1	81.83'	8,485 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Device

#1

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Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.83	1,786	0.0	0	0
82.16	1,786	30.0	177	177
94.00	1,786	30.0	6,344	6,521
94.25	1,786	40.0	179	6,699
94.75	1,786	40.0	357	7,056
95.75	1,786	40.0	714	7,771
96.75	1,786	40.0	714	8,485

 Routing
 Invert
 Outlet Devices

 Primary
 82.16'
 6.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.16' / 81.00' S= 0.0232 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=0.78 cfs @ 12.21 hrs HW=83.50' (Free Discharge) -1=Culvert (Inlet Controls 0.78 cfs @ 3.97 fps)

Summary for Pond DF-2: Drip Strip

Inflow Are	a =	0.223 ac,10	0.00% Impervio	ous, Inflow De	pth = 2.87	7" for 2-Yr	Storm event
Inflow	=	0.67 cfs @	12.08 hrs, Vol	ume=	0.053 af		
Outflow	=	0.51 cfs @	12.15 hrs, Vol	ume=	0.051 af, /	Atten= 23%,	Lag= 4.0 min
Primary	=	0.51 cfs @	12.15 hrs, Vol	ume=	0.051 af		
Routed	l to Ponc	DMH7 :					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 82.89' @ 12.15 hrs Surf.Area= 896 sf Storage= 284 cf

Plug-Flow detention time= 52.5 min calculated for 0.051 af (96% of inflow) Center-of-Mass det. time= 29.5 min (786.5 - 757.1)

Volume	Inver	t Ava	il.Storage	Storage Descri	ption	
#1	81.83	5"	4,257 cf	Custom Stage	Data (Prismatio)Listed below (Recalc)
Elevation (feet)	S	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
81.83		896	0.0	0	0	
82.16		896	30.0	89	89	
94.00		896	30.0	3,183	3,271	
94.25		896	40.0	90	3,361	
94.75		896	40.0	179	3,540	
95.75		896	40.0	358	3,898	
96.75		896	40.0	358	4,257	
<u>Device R</u>	outing	In	vert Out	et Devices		
#1 P	rimary	82	L= 9 Inlet		cting, no headwa 82.16' / 80.00' S	all, Ke= 0.900 S= 0.0227 '/' Cc= 0.900

Primary OutFlow Max=0.52 cfs @ 12.15 hrs HW=82.89' (Free Discharge) -1=Culvert (Inlet Controls 0.52 cfs @ 2.62 fps)

Summary for Pond DF-3: Drip Strip

Inflow Area =	0.123 ac, 68.93% Impervious, Inflov	v Depth = 2.16" for 2-Yr Storm event
Inflow =	0.31 cfs @ 12.09 hrs, Volume=	0.022 af
Outflow =	0.21 cfs @ 12.18 hrs, Volume=	0.022 af, Atten= 32%, Lag= 5.4 min
Primary =	0.21 cfs @ 12.18 hrs, Volume=	0.022 af
Routed to Por	nd DMH6 : DMH-6	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.65' @ 12.18 hrs Surf.Area= 1,786 sf Storage= 174 cf

Plug-Flow detention time= 37.4 min calculated for 0.022 af (100% of inflow) Center-of-Mass det. time= 37.5 min (841.2 - 803.7)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	93.	33'	2,859 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevatio (fee 93.3 93.6 94.0 95.0 95.5 96.5 97.5	et) 33 66 00 00 50 50	Surf.Area (sq-ft) 1,786 1,786 1,786 1,786 1,786 1,786 1,786 1,786	Voids (%) 30.0 30.0 40.0 40.0 40.0 40.0 40.0	Inc.Store (cubic-feet) 0 177 182 714 357 714 357 714 714	Cum.Store (cubic-feet) 0 177 359 1,073 1,431 2,145 2,859	
Device	Routing	In	vert Outl	et Devices		
#1	Primary	93	L= 5 Inlet	Round Culvert 50.0' CPP, project / Outlet Invert= \$ 0.012, Flow Area	cting, no headwa 93.33' / 92.00' S	II, Ke= 0.900 = 0.0266 '/' Cc= 0.900

Primary OutFlow Max=0.21 cfs @ 12.18 hrs HW=93.65' (Free Discharge) ←1=Culvert (Inlet Controls 0.21 cfs @ 1.53 fps)

Summary for Pond DMH2: DMH-2

 Inflow Area =
 0.355 ac, 76.51% Impervious, Inflow Depth =
 2.34" for 2-Yr Storm event

 Inflow =
 0.89 cfs @
 12.10 hrs, Volume=
 0.069 af

 Outflow =
 0.89 cfs @
 12.10 hrs, Volume=
 0.069 af, Atten= 0%, Lag= 0.0 min

 Primary =
 0.89 cfs @
 12.10 hrs, Volume=
 0.069 af

 Routed to Pond DMH3 : DMH-3
 0.010 hrs, Volume=
 0.069 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Peak Elev= 86.47' @ 12.10 hrs Flood Elev= 94.25'

DeviceRoutingInvertOutlet Devices#1Primary86.00'**18.0" Round Culvert**
L= 150.0' CPP, projecting, no headwall, Ke= 0.900
Inlet / Outlet Invert= 86.00' / 83.50' S= 0.0167 '/' Cc= 0.900
n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=0.87 cfs @ 12.10 hrs HW=86.47' (Free Discharge) -1=Culvert (Inlet Controls 0.87 cfs @ 1.84 fps)

Summary for Pond DMH3: DMH-3

Inflow Area =1.713 ac, 72.25% Impervious, Inflow Depth =2.23" for 2-Yr Storm eventInflow =1.87 cfs @12.11 hrs, Volume=0.318 afOutflow =1.87 cfs @12.11 hrs, Volume=0.318 af, Atten= 0%, Lag= 0.0 minPrimary =1.87 cfs @12.11 hrs, Volume=0.318 afRouted to Pond DMH4 : DMH-40.318 af0.318 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 82.23' @ 12.11 hrs Flood Elev= 96.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	81.51'	18.0" Round Culvert L= 82.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 81.51' / 81.10' S= 0.0050 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=1.86 cfs @ 12.11 hrs HW=82.23' (Free Discharge) -1=Culvert (Barrel Controls 1.86 cfs @ 3.23 fps)

Summary for Pond DMH4: DMH-4

 Inflow Area =
 2.186 ac, 78.25% Impervious, Inflow Depth =
 2.34" for 2-Yr Storm event

 Inflow =
 2.59 cfs @
 12.12 hrs, Volume=
 0.427 af

 Outflow =
 2.59 cfs @
 12.12 hrs, Volume=
 0.427 af, Atten= 0%, Lag= 0.0 min

 Primary =
 2.59 cfs @
 12.12 hrs, Volume=
 0.427 af

 Routed to Pond DMH5 : DMH-5
 0.427 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 81.85' @ 12.12 hrs Flood Elev= 96.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	81.00'	18.0" Round Culvert
			L= 213.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 81.00' / 79.90' S= 0.0052 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=2.57 cfs @ 12.12 hrs HW=81.85' (Free Discharge) -1=Culvert (Inlet Controls 2.57 cfs @ 2.48 fps)

Summary for Pond DMH5: DMH-5

 Inflow Area =
 2.186 ac, 78.25% Impervious, Inflow Depth =
 2.34" for 2-Yr Storm event

 Inflow =
 2.59 cfs @
 12.12 hrs, Volume=
 0.427 af

 Outflow =
 2.59 cfs @
 12.12 hrs, Volume=
 0.427 af, Atten= 0%, Lag= 0.0 min

 Primary =
 2.59 cfs @
 12.12 hrs, Volume=
 0.427 af

 Routed to Link SP-2 :
 12.12 hrs, Volume=
 0.427 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 80.65' @ 12.12 hrs Flood Elev= 96.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	79.80'	18.0" Round Culvert L= 55.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 79.80' / 78.00' S= 0.0327 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=2.58 cfs @ 12.12 hrs HW=80.65' (Free Discharge) -1=Culvert (Inlet Controls 2.58 cfs @ 2.48 fps)

Summary for Pond DMH6: DMH-6

Inflow Area	a =	0.235 ac, 3	36.02% Imperv	vious, Inflow De	epth = 1.55"	for 2-Yr Storm event
Inflow	=	0.29 cfs @	12.14 hrs, Vo	olume=	0.030 af	
Outflow	=	0.29 cfs @	12.14 hrs, Vo	olume=	0.030 af, Att	en= 0%, Lag= 0.0 min
Primary	=	0.29 cfs @	12.14 hrs, Vo	olume=	0.030 af	-
Routed	to Pond	I DMH7 :				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 90.28' @ 12.14 hrs Flood Elev= 95.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.00'	15.0" Round Culvert L= 163.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.00' / 80.85' S= 0.0561 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=0.29 cfs @ 12.14 hrs HW=90.28' (Free Discharge) -1=Culvert (Inlet Controls 0.29 cfs @ 1.42 fps)

Summary for Pond DMH7:

Inflow Area = 0.458 ac, 67.13% Impervious, Inflow Depth = 2.14" for 2-Yr Storm event Inflow 0.81 cfs @ 12.15 hrs. Volume= 0.082 af = 0.81 cfs @ 12.15 hrs, Volume= Outflow = 0.082 af, Atten= 0%, Lag= 0.0 min 0.81 cfs @ 12.15 hrs, Volume= Primary = 0.082 af Routed to Pond DMH8 : DMH-8 Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 81.22' @ 12.15 hrs Flood Elev= 90.40' Device Routing Invert **Outlet Devices** #1 Primary 80.75' 18.0" Round Culvert L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 80.75' / 80.50' S= 0.0051 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=0.80 cfs @ 12.15 hrs HW=81.22' (Free Discharge) —1=Culvert (Barrel Controls 0.80 cfs @ 2.57 fps)

Summary for Pond DMH8: DMH-8

 Inflow Area =
 0.458 ac, 67.13% Impervious, Inflow Depth = 2.14" for 2-Yr Storm event

 Inflow =
 0.81 cfs @
 12.15 hrs, Volume=
 0.082 af

 Outflow =
 0.81 cfs @
 12.15 hrs, Volume=
 0.082 af, Atten= 0%, Lag= 0.0 min

 Primary =
 0.81 cfs @
 12.15 hrs, Volume=
 0.082 af

 Routed to Reach 1R :
 0.81 cfs @
 12.15 hrs, Volume=

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 80.87' @ 12.15 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.40'	18.0" Round Culvert L= 93.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 80.40' / 80.00' S= 0.0043 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=0.81 cfs @ 12.15 hrs HW=80.87' (Free Discharge) -1=Culvert (Barrel Controls 0.81 cfs @ 2.56 fps)

Summary for Pond PP-1: Pervious Pavers

 Inflow Area =
 0.433 ac, 93.72% Impervious, Inflow Depth =
 2.65" for 2-Yr Storm event

 Inflow =
 1.25 cfs @
 12.09 hrs, Volume=
 0.096 af

 Outflow =
 0.35 cfs @
 12.43 hrs, Volume=
 0.096 af, Atten= 72%, Lag= 20.6 min

 Primary =
 0.35 cfs @
 12.43 hrs, Volume=
 0.096 af

 Routed to Pond DMH3 : DMH-3
 DMH-3

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

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Peak Elev= 91.68' @ 12.43 hrs Surf.Area= 3,978 sf Storage= 1,353 cf Flood Elev= 95.00' Surf.Area= 3,978 sf Storage= 7,630 cf

Plug-Flow detention time= 76.5 min calculated for 0.096 af (100% of inflow) Center-of-Mass det. time= 77.0 min (852.0 - 775.0)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	90.	83'	7,630 cf	Custom Stage	Data (Prismatio	c)Listed below (Recalc)
		~ ~ ~ ~			a a	
Elevatio		Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
90.8	83	3,978	0.0	0	0	
91. ⁻	16	3,978	40.0	525	525	
91.5	50	3,978	40.0	541	1,066	
92.0	00	3,978	40.0	796	1,862	
93.0	00	3,978	40.0	1,591	3,453	
94.(00	3,978	40.0	1,591	5,044	
94.5	50	3,978	30.0	597	5,641	
95.0	00	3,978	100.0	1,989	7,630	
Device	Routing	Ir	vert Out	let Devices		
#1	Primary	90).83' 12.()" Round Culver	rt	
	2		L= ^	100.0' CPP, proj	ecting, no headv	vall. Ke= 0.900
				· · · · ·	Q.	S= 0.0183 '/' Cc= 0.900
				0.012, Flow Area		
#2	Device ²	1 90				Limited to weir flow at low heads
<i></i>	201100					

Primary OutFlow Max=0.35 cfs @ 12.43 hrs HW=91.68' (Free Discharge)

-1=Culvert (Passes 0.35 cfs of 1.76 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.35 cfs @ 3.98 fps)

Summary for Pond PP-2: Pervious Pavers

 Inflow Area =
 0.059 ac, 90.30% Impervious, Inflow Depth =
 2.65" for 2-Yr Storm event

 Inflow =
 0.17 cfs @
 12.09 hrs, Volume=
 0.013 af

 Outflow =
 0.17 cfs @
 12.09 hrs, Volume=
 0.013 af, Atten= 1%, Lag= 0.6 min

 Primary =
 0.17 cfs @
 12.09 hrs, Volume=
 0.013 af

 Routed to Pond DMH2 : DMH-2
 DMH-2

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.84' @ 12.09 hrs Surf.Area= 1,023 sf Storage= 6 cf Flood Elev= 97.50' Surf.Area= 1,023 sf Storage= 2,116 cf

Plug-Flow detention time= 0.6 min calculated for 0.013 af (100% of inflow) Center-of-Mass det. time= 0.6 min (775.5 - 775.0)

Volume	Invert	Avail.Storage	Storage Description
#1	92.83'	2,116 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

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Elevatior (feet)	-	Surf.Area (sq-ft)	Void %)		Cum.Store (cubic-feet)	
92.83	/	1,023	0.0	/ /		
93.33	3	1,023	40.0) 205	205	
94.00)	1,023	40.0) 274	479	
95.00)	1,023	40.0) 409	888	
95.50)	1,023	40.0) 205	1,093	
96.00)	1,023	40.0) 205	1,297	
97.00)	1,023	30.0) 307	1,604	
97.50)	1,023	100.0) 512	2,116	
Device	Routing	In	vert	Outlet Devices		
#1	Primary	90).83'	12.0" Round Culv	ert	
	-			L= 100.0' CPP, pro		
				Inlet / Outlet Invert=	= 90.83' / 89.00' \$	S= 0.0183 '/' Cc= 0.900
				n= 0.012, Flow Are		
#2	Device 1	90).83'	4.0" Vert. Orifice/G	Grate C= 0.600	Limited to weir flow at low heads

Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=92.84' (Free Discharge)

1=Culvert (Passes 0.57 cfs of 3.67 cfs potential flow) **2=Orifice/Grate** (Orifice Controls 0.57 cfs @ 6.54 fps)

Summary for Pond PP-3: Pervious Pavers

Inflow Are	a =	0.113 ac,10	0.00% Imperv	vious, Inflow D	Depth = 2.87	7" for 2-Yr Storm event
Inflow	=	0.34 cfs @	12.08 hrs, Vo	olume=	0.027 af	
Outflow	=	0.32 cfs @	12.11 hrs, Vo	olume=	0.027 af, <i>i</i>	Atten= 6%, Lag= 1.8 min
Primary	=	0.32 cfs @	12.11 hrs, Vo	olume=	0.027 af	
Routed	to Ponc	DMH2 : DM	H-2			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.36' @ 12.11 hrs Surf.Area= 3,446 sf Storage= 37 cf Flood Elev= 96.50' Surf.Area= 3,446 sf Storage= 6,437 cf

Plug-Flow detention time= 1.9 min calculated for 0.027 af (100% of inflow) Center-of-Mass det. time= 1.9 min (759.0 - 757.1)

Volume	Invert	Ava	il.Storage	Storage Descri	ption	
#1	92.33'		6,437 cf	Custom Stage	Data (Prismatio	:) Listed below (Recalc)
Elevation	Sur	f.Area	Voids	Inc.Store	Cum.Store	
(feet)		(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
92.33		3,446	0.0	0	0	
92.83		3,446	40.0	689	689	
93.00		3,446	40.0	234	924	
94.00		3,446	40.0	1,378	2,302	
94.50		3,446	40.0	689	2,991	
95.00		3,446	40.0	689	3,680	
96.00		3,446	30.0	1,034	4,714	
96.50		3,446	100.0	1,723	6,437	

Type III 24-hr 2-Yr Storm Rainfall=3.10" Printed 6/13/2022

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Device	Routing	Invert	Outlet Devices					
#1	Primary	90.83'	12.0" Round Culvert					
			L= 100.0' CPP, projecting, no headwall, Ke= 0.900					
			Inlet / Outlet Invert= 90.83' / 89.00' S= 0.0183 '/' Cc= 0.900					
			n= 0.012, Flow Area= 0.79 sf					
#2	Device 1	90.83'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads					

Primary OutFlow Max=0.49 cfs @ 12.11 hrs HW=92.36' (Free Discharge)

1=Culvert (Passes 0.49 cfs of 3.02 cfs potential flow)

1–2=Orifice/Grate (Orifice Controls 0.49 cfs @ 5.62 fps)

Summary for Pond UDSF-1: Filtration Basin

Inflow Area = 0.436 ac, 42.19% Impervious, Inflow Depth = 1.53" for 2-Yr Storm event 0.78 cfs @ 12.09 hrs, Volume= 0.056 af Inflow = Outflow = 0.46 cfs @ 12.21 hrs, Volume= 0.056 af, Atten= 41%, Lag= 7.2 min Primary = 0.46 cfs @ 12.21 hrs, Volume= 0.056 af Routed to Reach 1R : Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Reach 1R :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 81.68' @ 12.21 hrs Surf.Area= 691 sf Storage= 327 cf

Plug-Flow detention time= 14.4 min calculated for 0.056 af (100% of inflow) Center-of-Mass det. time= 14.3 min (848.7 - 834.4)

Volume	Invert	Ava	il.Storage	Storage Descrip	tion					
#1	80.33'		3,282 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)				
Elevation Surf.Are (feet) (sq-		ırf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)						
80.3		691	0.0	0	0					
80.6		691	40.0	94	94					
81.0	00	691	40.0	91	185					
81.5	50	691	30.0	104	289					
82.0	00	691	30.0	104	392					
82.5	50	691	30.0	104	496					
83.0	00	835	100.0	382	878					
84.0		1,172	100.0	1,004	1,881					
85.0	00	1,629	100.0	1,401	3,282					
Device	Routing	In	vert Ou	tlet Devices						
#1	Primary	80).33' 12 .	0" Round Culver	t					
	-		L=	25.0' CPP, square	e edge headwall,	Ke= 0.500				
						= 0.0132 '/' Cc= 0.900				
				0.012, Flow Area=						
#2	Device 1									
#3	Device 1	84		B.0" Horiz. Orifice/Grate C= 0.600						
	. .			mited to weir flow at low heads						
#4	Secondary	84		•		dth Broad-Crested Rectangular Weir01.201.401.601.802.00				

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2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83

Primary OutFlow Max=0.46 cfs @ 12.21 hrs HW=81.68' (Free Discharge) 1=Culvert (Passes 0.46 cfs of 3.49 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.46 cfs @ 5.25 fps) 3=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.33' (Free Discharge) 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link SP-1:

Inflow Area =	e 2.373 ac,	78.91% Impervious,	Inflow Depth = 2.1	10" for 2-Yr Storm event
Inflow =	5.22 cfs @) 12.08 hrs, Volum	e= 0.415 af	
Primary =	5.22 cfs @) 12.08 hrs, Volume	e= 0.415 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Link SP-2:

Inflow Area	a =	4.667 ac, 52.	.49% Impervious,	Inflow Depth =	1.86" 1	for 2-Yr Storm event
Inflow	=	4.93 cfs @ 1	2.22 hrs, Volume	e= 0.723	af	
Primary	=	4.93 cfs @ 1	2.22 hrs, Volume	e= 0.723 a	af, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Subcatchment 10S:

Runoff = 2.03 cfs @ 12.08 hrs, Volume= 0.165 af, Depth= 4.36" Routed to Pond CB-1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

	Area (sf)	CN	Description		
*	10,475	98	Offsite impe	ervious	
*	9,057	98	Existing Im	pervious	
	285	74	>75% Gras	s cover, Go	ood, HSG C
	19,817 285 19,532		Weighted A 1.44% Perv 98.56% Imp	vious Area	rea
- (mi	Гс Length n) (feet)	Slope (ft/ft		Capacity (cfs)	Description
6	.0				Direct Entry,

Summary for Subcatchment 20S:

Runoff = 0.17 cfs @ 12.08 hrs, Volume= 0.014 af, Depth= 4.36" Routed to Pond 2P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

	A	rea (sf)	CN	Description					
*		784	98	Existing Imp	pervious				
*		853	98	Offsite Impe	ervious				
		1,637	98	8 Weighted Average					
		1,637		100.00% Im	npervious A	Area			
	Тс	Length	Slope	e Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
	6.0					Direct Entry,			

Summary for Subcatchment 30S:

Runoff = 1.79 cfs @ 12.08 hrs, Volume= 0.141 af, Depth= 4.14" Routed to Pond 3P :

Type III 24-hr 10-Yr Storm Rainfall=4.60" Printed 6/13/2022

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	А	rea (sf)	CN	Description		
*		13,779	98	Offsite Impe	ervious	
*		2,496	98	Impervious		
*		746	74	Offsite Land	lscaped	
		754	74	>75% Grass	s cover, Go	ood, HSG C
		17,775	96	Weighted A	verage	
		1,500		8.44% Perv	ious Area	
		16,275		91.56% Imp	ervious Ar	rea
	Тс	Length	Slop	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	6.0					Direct Entry,

Summary for Subcatchment 40S:

Runoff	=	1.01 cfs @	12.09 hrs,	Volume=	0.074 af,	Depth= 3.49"
Routed	I to Pond	4P :				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

	Area (sf) CN	Description					
*	7,341	1 98	Offsite Imp	ervious				
*	806	6 98	Existing Im	pervious				
	1,646	6 74	>75% Gras	s cover, Go	ood, HSG C			
	1,338	8 65	Brush, Goo	d, HSG C				
	11,13 ⁻	1 90	90 Weighted Average					
	2,984	4	26.81% Pe	rvious Area				
	8,147	7	73.19% lm	pervious Ar	ea			
	Tc Leng (min) (fee		pe Velocity /ft) (ft/sec)	Capacity (cfs)	Description			
	6.0				Direct Entry,			

Summary for Subcatchment 50S:

Runoff = 0.77 cfs @ 12.08 hrs, Volume= 0.063 af, Depth= 4.36" Routed to Pond 5P :

	Area (sf)	CN	Description
*	5,773	98	Offsite Impervious
*	1,775	98	Existing Impervious
	7,548	98	Weighted Average
	7,548		100.00% Impervious Area

Prepare		ntic Re	source Co	Type III 24-hr	10-Yr Storm Rainfall=4.60" Printed 6/13/2022 Page 27		
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Descriptior	1	
6.0					Direct Ent	ry,	
			Sur	nmary fo	r Subcatcl	hment 60S:	
Runoff Route	= ed to Pond		fs @ 12.0	8 hrs, Volu	ıme=	0.050 af, Dep	th= 4.36"
			thod, UH=S n Rainfall=/		ited-CN, Tim	e Span= 0.00-7	2.00 hrs, dt= 0.03 hrs
A	rea (sf)	CN	Description				
*	3,268		Offsite Imp				
*	2,700		Existing Im				
	5,968 5,968		Weighted A 100.00% In	npervious A	rea		
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Descriptior	ı	
6.0					Direct Ent	ry,	
			Sur	nmary fo	r Subcatcl	hment 70S:	
Runoff Route	= ed to Pone		fs @ 12.0	9 hrs, Volu	ıme=	0.086 af, Dep	th= 3.49"
			thod, UH=S n Rainfall=4		ited-CN, Tim	e Span= 0.00-7	2.00 hrs, dt= 0.03 hrs
A	rea (sf)	CN	Description				
*	9,239		Offsite Imp				
	2,635				ood, HSG C		
	1,040		Brush, Goo				
	12,914 3,675		Weighted A	werage rvious Area	1		
	0,070						

	Length (feet)	 Velocity (ft/sec)	Capacity (cfs)	Description	
6.0				Direct Entry	

9,239

71.54% Impervious Area

Direct Entry,

Summary for Subcatchment 80S:

1.05 cfs @ 12.00 hrs, Volume= Runoff 0.063 af, Depth= 2.91" = Routed to Pond 7P :

Type III 24-hr 10-Yr Storm Rainfall=4.60" Printed 6/13/2022

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	Area (sf)	CN	Description
*	9,056	98	Offsite Impervious
	2,284	30	Brush, Good, HSG A
	11,340	84	Weighted Average
	2,284		20.14% Pervious Area
	9,056		79.86% Impervious Area

Summary for Subcatchment 90S:

Runoff	=	0.09 cfs @	12.15 hrs,	Volume=	0.014 af,	Depth= 0.49"
Routed	l to Pond	d 9P :				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

_	Are	ea (sf)	CN	Description					
*		4,183	98	Offsite Impe	ervious area	a			
	1	1,072	30	Brush, Goo	d, HSG A				
	1	5,255	49	Weighted Average					
	1	1,072		72.58% Pervious Area					
		4,183		27.42% Imp	pervious Are	rea			
		Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
	6.0					Direct Entry,			
						•			

Summary for Subcatchment 100:

Runoff = 2.96 cfs @ 12.28 hrs, Volume= 0.314 af, Depth= 2.38" Routed to Link SP-2 :

	Area (sf)	CN	Description			
	41,443	74	>75% Grass cover, Good, HSG C			
*	16,896	74	OFSITE AREA TO BE UPDATED			
*	10,765	98	Offsite Impervious			
	69,104	78	Weighted Average			
	58,339		84.42% Pervious Area			
	10,765		15.58% Impervious Area			

20-001.1 Post-DevelopmentType III 24-hr 10-Yr Storm Rainfall=4.60"Prepared by Atlantic Resource ConsultantsPrinted 6/13/2022HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLCPage 29Tc LengthSlopeVelocityCapacityDescription(min)(feet)(ft/ft)

_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	16.3	75	0.0930	0.08		Sheet Flow, A-B
						Woods: Dense underbrush n= 0.800 P2= 3.10"
	3.2	110	0.0540	0.58		Shallow Concentrated Flow, B-C
						Forest w/Heavy Litter Kv= 2.5 fps
	0.6	70	0.0100	1.95	1.95	Parabolic Channel, C-D
_						W=3.00' D=0.50' Area=1.0 sf Perim=3.2' n= 0.035
	20.1	255	Total			

Summary for Subcatchment 101S:

Runoff = 1.43 cfs @ 12.09 hrs, Volume= 0.102 af, Depth= 2.81" Routed to Pond UDSF-1 : Filtration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

Area	a (sf) CN	Description	l	
8	,017 98			
3	,766 70	Woods, Go	od, HSG C	
7	,218 74	>75% Gras	s cover, Go	ood, HSG C
19	,001 83	Weighted A	Verage	
10	,984		rvious Area	1
8	,017	42.19% Im	pervious Ar	ea
	•	ope Velocity t/ft) (ft/sec)	Capacity (cfs)	Description
6.0				Direct Entry,

Summary for Subcatchment 102S:

Runoff = 0.69 cfs @ 12.09 hrs, Volume= 0.050 af, Depth= 3.29" Routed to Pond DMH2 : DMH-2

A	rea (sf)	CN	Description						
	4,611	98	Roofs, HSG D						
	3,387	74 :	>75% Gras	s cover, Go	ood, HSG C				
	7,998	88	Weighted Average						
	3,387	4	12.35% Per	vious Area	а				
	4,611	:	57.65% Imp	ervious Ar	rea				
Tc (min)	Length	Slope	,	Capacity (cfs)	1				
(min)	(feet)	(ft/ft)	(It/sec)	(CIS)					
6.0					Direct Entry,				

Summary for Subcatchment 103S:

Runoff 1.90 cfs @ 12.08 hrs, Volume= 0.149 af, Depth= 4.14" = Routed to Pond PP-1 : Pervious Pavers

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

A	rea (sf)	CN [Description				
	17,688	98 F	Paved park	ing, HSG D			
	1,185	74 >	75% Gras	s cover, Go	ood, HSG C		
	18,873	96 V	Veighted A	verage			
	1,185	6	6.28% Pervious Area				
	17,688	ę	03.72% Imp	pervious Are	ea		
Tc	Length	Slope	Velocity	Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 104S:

Runoff 2.11 cfs @ 12.08 hrs, Volume= = Routed to Pond DF-1 : Drip Strip

0.172 af, Depth= 4.36"

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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

_	A	rea (sf)	CN	Description				
		18,803	98	Roofs, HSG	6 D			
*		1,786	98	Stone				
		20,589	98	Weighted A	verage			
		20,589		100.00% Impervious Area				
	Tc	Length	Slop	e Velocity	Capacity	Description		
_	(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)			
	6.0					Direct Entry,		

Summary for Subcatchment 105S:

Runoff 1.00 cfs @ 12.08 hrs, Volume= 0.081 af, Depth= 4.36" = Routed to Pond DF-2 : Drip Strip

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	A	rea (sf)	CN	Description				
		8,810	98	Roofs, HSG	G D			
*		896	98	Stone				
		9,706 9,706	98	Weighted Average 100.00% Impervious Area				
	Tc (min)	Length (feet)	Slop (ft/fl		Capacity (cfs)	Description		
_	6.0					Direct Entry,		

Summary for Subcatchment 106S:

Runoff = 3.57 cfs @ 12.09 hrs, Volume= 0.261 af, Depth= 3.39" Routed to Pond 1P : R-TANK

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

Area (sf)	CN	Description					
21,170	98	Paved park	ing, HSG D)			
3,223	98	Roofs, HSC	δĎ				
15,887	74	>75% Gras	s cover, Go	bod, HSG C			
40,280	89	Weighted A	Weighted Average				
15,887		39.44% Per	vious Area				
24,393		60.56% Imp	pervious Ar	ea			
Tc Length	Slop		Capacity	Description			
(min) (feet)	(ft/1	ft) (ft/sec)	(cfs)				
6.0				Direct Entry,			
				- ·			

Summary for Subcatchment 107S:

Runoff = 0.50 cfs @ 12.09 hrs, Volume= 0.037 af, Depth= 3.59" Routed to Pond DF-3 : Drip Strip

	Area (sf)	CN	Description
	3,214	98	Roofs, HSG D
*	480	98	Stone
	1,665	74	>75% Grass cover, Good, HSG C
	5,359	91	Weighted Average
	1,665		31.07% Pervious Area
	3,694		68.93% Impervious Area

						<i>p</i> = =	
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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.0					Direct Entry,		
			Sum	mary for	[.] Subcatchm	ent 10	3S:
Runoff Route	= ed to Ponc			9 hrs, Volu	ume= 0	.018 af,	Depth= 1.89"
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"						
<u> </u>	rea (sf)	CN D	escription				
	2,447 2,448			s cover, Go od, HSG C	ood, HSG C		
	4,895	72 W	/eighted A	verage			
	4,895			ervious Are	a		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
6.0	6.0 Direct Entry,						
Summary for Subcatchment 109S:							

Type III 24-hr 10-Yr Storm Rainfall=4.60"

Runoff = 0.26 cfs @ 12.08 hrs, Volume= 0.020 af, Depth= 4.14" Routed to Pond PP-2 : Pervious Pavers

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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 10-Yr Storm Rainfall=4.60"

A	rea (sf)	CN	Description				
	2,326	98	Paved parking, HSG D				
	250	74	>75% Grass cover, Good, HSG C				
	2,576	96	Weighted A	verage			
	250		9.70% Pervious Area				
	2,326		90.30% Impervious Area				
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 110S:

Runoff	=	0.50 cfs @	12.08 hrs,	Volume=	0.041 af	, Depth= 4.36"
Routed	l to Pond	d PP-3 : Pervi	ous Pavers			

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Area (sf) CN Description							
4,907 98 Paved parking, HSG D							
4,907 100.00% Impervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
6.0 Direct Entry,							
Summary for Reach 1R:							
Inflow Area = 0.894 ac, 54.97% Impervious, Inflow Depth = 3.16" for 10-Yr Storm event Inflow = 1.88 cfs @ 12.15 hrs, Volume= 0.236 af Outflow = 1.87 cfs @ 12.17 hrs, Volume= 0.236 af, Atten= 1%, Lag= 1.4 min Routed to Link SP-2 :							
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Max. Velocity= 1.44 fps, Min. Travel Time= 2.0 min Avg. Velocity = 0.40 fps, Avg. Travel Time= 7.3 min							
Peak Storage= 224 cf @ 12.17 hrs Average Depth at Peak Storage= 0.06' , Surface Width= 20.64' Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 60.16 cfs							
20.00' x 0.50' deep channel, n= 0.035 Earth, dense weeds Side Slope Z-value= 5.0 '/' Top Width= 25.00' Length= 173.0' Slope= 0.0462 '/' Inlet Invert= 80.00', Outlet Invert= 72.00'							
±							
Summary for Pond 1P: R-TANK							

 Inflow Area =
 0.925 ac, 60.56% Impervious, Inflow Depth =
 3.39" for 10-Yr Storm event

 Inflow =
 3.57 cfs @
 12.09 hrs, Volume=
 0.261 af

 Outflow =
 0.85 cfs @
 12.48 hrs, Volume=
 0.261 af, Atten= 76%, Lag= 23.6 min

 Primary =
 0.85 cfs @
 12.48 hrs, Volume=
 0.261 af

 Routed to Pond DMH3 : DMH-3
 0.48 hrs, Volume=
 0.261 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 89.59' @ 12.48 hrs Surf.Area= 3,593 sf Storage= 3,214 cf Flood Elev= 91.30' Surf.Area= 3,593 sf Storage= 6,075 cf

Plug-Flow detention time= 35.5 min calculated for 0.261 af (100% of inflow) Center-of-Mass det. time= 35.6 min (832.9 - 797.3)

Type III 24-hr 10-Yr Storm Rainfall=4.60" Printed 6/13/2022

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Volume	Invert	Avail.Storage	Storage Description
#1	85.30'	1,131 cf	Custom Stage Data (Prismatic)Listed below (Recalc)
#2A	88.22'	1,396 cf	31.56'W x 62.65'L x 4.07'H Field A
			8,049 cf Overall - 4,560 cf Embedded = 3,489 cf x 40.0% Voids
#3A	88.47'	4,332 cf	ACF R-Tank HD 2 x 525 Inside #2
			Inside= 15.7"W x 33.9"H => 3.52 sf x 2.35'L = 8.3 cf
			Outside= 15.7"W x 33.9"H => 3.70 sf x 2.35'L = 8.7 cf
			525 Chambers in 21 Rows
		6 859 cf	Total Available Storage

6,859 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.30	1,616	0.0	0	0
85.63	1,616	40.0	213	213
85.97	1,616	20.0	110	323
86.47	1,616	20.0	162	485
86.97	1,616	20.0	162	646
87.47	1,616	20.0	162	808
87.97	1,616	20.0	162	970
88.47	1,616	20.0	162	1,131

Device	Routing	Invert	Outlet Devices
#1	Primary	85.10'	15.0" Round Culvert
			L= 126.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 85.10' / 83.50' S= 0.0127 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.23 sf
#2	Device 1	85.30'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	90.60'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
			4.0' Crest Height
#4	Device 1	90.00'	12.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.85 cfs @ 12.48 hrs HW=89.59' (Free Discharge)

-**1=Culvert** (Passes 0.85 cfs of 9.17 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.85 cfs @ 9.78 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond 2P:

Inflow Area = 1.918 ac, 74.25% Impervious, Inflow Depth = 3.16" for 10-Yr Storm event Inflow 6.15 cfs @ 12.08 hrs, Volume= 0.505 af = Outflow 6.15 cfs @ 12.08 hrs, Volume= = 6.15 cfs @12.08 hrs, Volume=0.505 af,6.15 cfs @12.08 hrs, Volume=0.505 af 0.505 af, Atten= 0%, Lag= 0.0 min Primary = Routed to Pond CB-1:

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 95.39' @ 12.08 hrs Flood Elev= 95.86'

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Device Routing Invert Outlet Devices					
#1 Primary 90.65' 12.0" Round Culvert L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.65' / 90.00' S= 0.0103 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf					
Primary OutFlow Max=6.08 cfs @ 12.08 hrs HW=95.30' (Free Discharge) 1=Culvert (Inlet Controls 6.08 cfs @ 7.74 fps)					
Summary for Pond 3P:					
Inflow Area = 1.881 ac, 73.74% Impervious, Inflow Depth = 3.14" for 10-Yr Storm event Inflow = 5.98 cfs @ 12.08 hrs, Volume= 0.491 af Outflow = 5.98 cfs @ 12.08 hrs, Volume= 0.491 af, Atten= 0%, Lag= 0.0 min Primary = 5.98 cfs @ 12.08 hrs, Volume= 0.491 af Routed to Pond 2P : 5.98 cfs @ 12.08 hrs, Volume=					
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 95.26' @ 12.08 hrs Flood Elev= 95.40'					
Device Routing Invert Outlet Devices					
#1 Primary 90.75' 12.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.29' / 90.75' S= -0.0092 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf					
Primary OutFlow Max=5.92 cfs @ 12.08 hrs HW=95.18' (Free Discharge) ☐_1=Culvert (Inlet Controls 5.92 cfs @ 7.53 fps)					
Summary for Pond 4P:					
Inflow Area = 1.473 ac, 68.80% Impervious, Inflow Depth = 2.86" for 10-Yr Storm event Inflow = 4.21 cfs @ 12.07 hrs, Volume= 0.351 af Outflow = 4.21 cfs @ 12.07 hrs, Volume= 0.351 af, Atten= 0%, Lag= 0.0 min Primary = 4.21 cfs @ 12.07 hrs, Volume= 0.351 af Routed to Pond 3P : 12.07 hrs, Volume= 0.351 af					
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.34' @ 12.07 hrs Flood Elev= 96.14'					
Device Routing Invert Outlet Devices					
#1 Primary 90.85' 12.0" Round Culvert L= 77.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.85' / 90.29' S= 0.0073 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf					

Primary OutFlow Max=4.16 cfs @ 12.07 hrs HW=93.29' (Free Discharge) **1=Culvert** (Inlet Controls 4.16 cfs @ 5.30 fps)

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			Summary for Po	ond 5P:	
Inflow Ai Inflow Outflow Primary Route	= =	3.22 cfs @ 12 3.22 cfs @ 12 3.22 cfs @ 12	8% Impervious, Inflow .07 hrs, Volume= .07 hrs, Volume= .07 hrs, Volume=	Depth = 2.73" for 1 0.276 af 0.276 af, Atten= 0% 0.276 af	
Peak Ele		d method, Time s @ 12.07 hrs	Span= 0.00-72.00 hrs, c	lt= 0.03 hrs	
Device	Routing	Invert	Outlet Devices		
#1	Primary Primary	91.13' 95.65'	12.0" Round Culvert L= 70.0' CPP, project	rifice/Grate C= 0.600	
1=Cu	Ilvert (Bari) 12.07 hrs HW=92.82' cfs @ 4.06 fps) cfs)	(Free Discharge)	
			Summary for Po	ond 6P:	
Inflow An Inflow Outflow Primary Route	= =	1.46 cfs @ 12 1.46 cfs @ 12 1.46 cfs @ 12	00% Impervious, Inflow .01 hrs, Volume= .01 hrs, Volume= .01 hrs, Volume=		
Peak Ele		d method, Time 5 @ 12.01 hrs	Span= 0.00-72.00 hrs, c	lt= 0.03 hrs	
Device	Routing	Invert	Outlet Devices		
#1	Primary	91.83'		ing, no headwall, Ke= (.83' / 91.23' S= 0.0094 0.79 sf	
		Max=1.43 cfs @ t Controls 1.43 c) 12.01 hrs HW=92.57' fs @ 2.30 fps)	(Free Discharge)	

Summary for Pond 7P:

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Inflow Area = 0.260 ac, 79.86% Impervious, Inflow Definition Inflow = 1.05 cfs @ 12.00 hrs, Volume= Outflow = 1.05 cfs @ 12.00 hrs, Volume= Primary = 1.05 cfs @ 12.00 hrs, Volume= Routed to Pond 6P : 0.260 ac, 79.86% Impervious, Inflow Definition	0.063 af 0.063 af, Atten= 0%, Lag= 0.0 min					
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= Peak Elev= 93.08' @ 12.00 hrs Flood Elev= 97.40'	0.03 hrs					
DeviceRoutingInvertOutlet Devices#1Primary92.43'12.0" Round Culvert						
L= 105.0' CPP, projectin	ng, no headwall, Ke= 0.900 3' / 92.03' S= 0.0038 '/' Cc= 0.900 79 sf					
Primary OutFlow Max=1.04 cfs @ 12.00 hrs HW=93.08' (I -1=Culvert (Barrel Controls 1.04 cfs @ 2.75 fps)	Free Discharge)					
Summary for Pon	d 8P:					
Inflow Area = 0.647 ac, 47.65% Impervious, Inflow Depth = 1.87" for 10-Yr Storm event Inflow = 1.24 cfs @ 12.09 hrs, Volume= 0.101 af Outflow = 1.24 cfs @ 12.09 hrs, Volume= 0.101 af, Atten= 0%, Lag= 0.0 min Primary = 1.24 cfs @ 12.09 hrs, Volume= 0.101 af Routed to Pond 5P : 12.09 hrs, Volume= 0.101 af						
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.83' @ 12.09 hrs Flood Elev= 97.36'						
Device Routing Invert Outlet Devices						
	g, no headwall, Ke= 0.900 6' / 91.23' S= 0.0109 '/' Cc= 0.900 79 sf					
Primary OutFlow Max=1.24 cfs @ 12.09 hrs HW=92.83' (Free Discharge) ↓1=Culvert (Inlet Controls 1.24 cfs @ 2.20 fps)						

Summary for Pond 9P:

 Inflow Area =
 0.350 ac, 27.42% Impervious, Inflow Depth =
 0.49" for 10-Yr Storm event

 Inflow =
 0.09 cfs @
 12.15 hrs, Volume=
 0.014 af

 Outflow =
 0.09 cfs @
 12.15 hrs, Volume=
 0.014 af, Atten= 0%, Lag= 0.0 min

 Primary =
 0.09 cfs @
 12.15 hrs, Volume=
 0.014 af

 Routed to Pond 8P :
 0.014 af
 0.014 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.22' @ 12.15 hrs Flood Elev= 96.37' 20-001.1 Post-Development Prepared by Atlantic Resource Consultants

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Device Routing	Invert Outlet Devices				
#1 Primary 9		g, no headwall, Ke= 0.900 6' / 92.16' S= 0.0125 '/' Cc= 0.900 79 sf			
Primary OutFlow Max=0.0	09 cfs @ 12.15 hrs HW=93.22' (ls 0.09 cfs @ 1.09 fps)	Free Discharge)			
	Summary for Pond	CB-1:			
Inflow = 8.18 cfs Outflow = 8.18 cfs	ac, 78.91% Impervious, Inflow De s @ 12.08 hrs, Volume= s @ 12.08 hrs, Volume= s @ 12.08 hrs, Volume=	epth = 3.39" for 10-Yr Storm event 0.671 af 0.671 af, Atten= 0%, Lag= 0.0 min 0.671 af			
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.65' @ 12.08 hrs Flood Elev= 95.55'					
Device Routing	Invert Outlet Devices				
#1 Primary 8		g, no headwall, Ke= 0.900 5' / 89.50' S= 0.0129 '/' Cc= 0.900 23 sf			
Primary OutFlow Max=8.09 cfs @ 12.08 hrs HW=93.58' (Free Discharge) [▲] -1=Culvert (Inlet Controls 8.09 cfs @ 6.59 fps)					
Summary for Pond DF-1: Drip Strip					
Inflow = 2.11 cfs Outflow = 1.04 cfs	s @ 12.08 hrs, Volume= s @ 12.23 hrs, Volume= s @ 12.23 hrs, Volume=	epth = 4.36" for 10-Yr Storm event 0.172 af 0.168 af, Atten= 51%, Lag= 8.9 min 0.168 af			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 84.35' @ 12.23 hrs Surf.Area= 1,786 sf Storage= 1,348 cf

Plug-Flow detention time= 47.1 min calculated for 0.168 af (98% of inflow) Center-of-Mass det. time= 31.9 min (781.4 - 749.4)

Volume	Invert	Avail.Storage	Storage Description
#1	81.83'	8,485 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Device

#1

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Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.83	1,786	0.0	0	0
82.16	1,786	30.0	177	177
94.00	1,786	30.0	6,344	6,521
94.25	1,786	40.0	179	6,699
94.75	1,786	40.0	357	7,056
95.75	1,786	40.0	714	7,771
96.75	1,786	40.0	714	8,485

 Routing
 Invert
 Outlet Devices

 Primary
 82.16'
 6.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.16' / 81.00' S= 0.0232 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=1.04 cfs @ 12.23 hrs HW=84.34' (Free Discharge)

Summary for Pond DF-2: Drip Strip

Inflow Are	a =	0.223 ac,10	0.00% Impervious,	Inflow Depth =	4.36"	for 10-Y	r Storm event
Inflow	=	1.00 cfs @	12.08 hrs, Volume	e= 0.081	af		
Outflow	=	0.71 cfs @	12.16 hrs, Volume	e 0.079	af, Atte	n= 29%,	Lag= 4.7 min
Primary	=	0.71 cfs @	12.16 hrs, Volume	e= 0.079	af		
Routed	to Pond	1 DMH7 :					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 83.32' @ 12.16 hrs Surf.Area= 896 sf Storage= 400 cf

Plug-Flow detention time= 40.2 min calculated for 0.079 af (97% of inflow) Center-of-Mass det. time= 24.2 min (773.7 - 749.4)

Volume	Inve	ert Ava	il.Storage	Storage Descri	ption	
#1	81.8	33'	4,257 cf	Custom Stage	Data (Prismatic) Listed below (Recalc)
Elevatio (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
81.8	3	896	0.0	0	0	
82.1	6	896	30.0	89	89	
94.0	0	896	30.0	3,183	3,271	
94.2	5	896	40.0	90	3,361	
94.7	5	896	40.0	179	3,540	
95.7	5	896	40.0	358	3,898	
96.7	5	896	40.0	358	4,257	
Device	Routing			et Devices		
#1	Primary	82	L= 9 Inlet		ecting, no headwa 82.16' / 80.00' S	all, Ke= 0.900 S= 0.0227 '/' Cc= 0.900

Primary OutFlow Max=0.71 cfs @ 12.16 hrs HW=83.31' (Free Discharge) -1=Culvert (Inlet Controls 0.71 cfs @ 3.61 fps)

Summary for Pond DF-3: Drip Strip

Inflow Area =	0.123 ac, 68.93% Impervious, Inflo	w Depth = 3.59" for 10-Yr Storm event
Inflow =	0.50 cfs @ 12.09 hrs, Volume=	0.037 af
Outflow =	0.35 cfs @ 12.17 hrs, Volume=	0.037 af, Atten= 30%, Lag= 4.9 min
Primary =	0.35 cfs @ 12.17 hrs, Volume=	0.037 af
Routed to Por	nd DMH6 : DMH-6	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.79' @ 12.17 hrs Surf.Area= 1,786 sf Storage= 247 cf

Plug-Flow detention time= 30.6 min calculated for 0.037 af (100% of inflow) Center-of-Mass det. time= 30.7 min (820.3 - 789.6)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	93.	33'	2,859 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevatio (fee 93.3 93.6 94.0 95.0 95.5 96.5 97.5	et) 33 66 00 00 50 50	Surf.Area (sq-ft) 1,786 1,786 1,786 1,786 1,786 1,786 1,786 1,786	Voids (%) 0.0 30.0 40.0 40.0 40.0 40.0 40.0	Inc.Store (cubic-feet) 0 177 182 714 357 714 714 714	Cum.Store (cubic-feet) 0 177 359 1,073 1,431 2,145 2,859	
Device #1	Routing Primary		3.33' 6.0'' L= 5	et Devices Round Culvert 50.0' CPP, project () Outlet Invert= 9	cting, no headwal	ll, Ke= 0.900 = 0.0266 '/' Cc= 0.900
				0.012, Flow Area		

Primary OutFlow Max=0.34 cfs @ 12.17 hrs HW=93.79' (Free Discharge) ←1=Culvert (Inlet Controls 0.34 cfs @ 1.82 fps)

Summary for Pond DMH2: DMH-2

 Inflow Area =
 0.355 ac, 76.51% Impervious, Inflow Depth =
 3.77" for 10-Yr Storm event

 Inflow =
 1.41 cfs @
 12.10 hrs, Volume=
 0.112 af

 Outflow =
 1.41 cfs @
 12.10 hrs, Volume=
 0.112 af, Atten= 0%, Lag= 0.0 min

 Primary =
 1.41 cfs @
 12.10 hrs, Volume=
 0.112 af, Atten= 0%, Lag= 0.0 min

 Routed to Pond DMH3 : DMH-3
 0.112 af
 0.112 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Peak Elev= 86.61' @ 12.10 hrs Flood Elev= 94.25'

DeviceRoutingInvertOutlet Devices#1Primary86.00'**18.0" Round Culvert**
L= 150.0' CPP, projecting, no headwall, Ke= 0.900
Inlet / Outlet Invert= 86.00' / 83.50' S= 0.0167 '/' Cc= 0.900
n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=1.39 cfs @ 12.10 hrs HW=86.60' (Free Discharge) -1=Culvert (Inlet Controls 1.39 cfs @ 2.09 fps)

Summary for Pond DMH3: DMH-3

Inflow Area = 1.713 ac, 72.25% Impervious, Inflow Depth = 3.66" for 10-Yr Storm event Inflow = 2.56 cfs @ 12.10 hrs, Volume= 0.522 af Outflow = 2.56 cfs @ 12.10 hrs, Volume= 0.522 af, Atten= 0%, Lag= 0.0 min 2.56 cfs @ 12.10 hrs, Volume= Primary = 0.522 af Routed to Pond DMH4 : DMH-4

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 82.38' @ 12.10 hrs Flood Elev= 96.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	81.51'	18.0" Round Culvert L= 82.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 81.51' / 81.10' S= 0.0050 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=2.53 cfs @ 12.10 hrs HW=82.37' (Free Discharge) -1=Culvert (Barrel Controls 2.53 cfs @ 3.47 fps)

Summary for Pond DMH4: DMH-4

 Inflow Area =
 2.186 ac, 78.25% Impervious, Inflow Depth =
 3.79" for 10-Yr Storm event

 Inflow =
 3.49 cfs @
 12.12 hrs, Volume=
 0.690 af

 Outflow =
 3.49 cfs @
 12.12 hrs, Volume=
 0.690 af, Atten= 0%, Lag= 0.0 min

 Primary =
 3.49 cfs @
 12.12 hrs, Volume=
 0.690 af

 Routed to Pond DMH5 : DMH-5
 DMH-5

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 82.02' @ 12.12 hrs Flood Elev= 96.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	81.00'	18.0" Round Culvert L= 213.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 81.00' / 79.90' S= 0.0052 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=3.48 cfs @ 12.12 hrs HW=82.02' (Free Discharge) -1=Culvert (Inlet Controls 3.48 cfs @ 2.72 fps)

Summary for Pond DMH5: DMH-5

 Inflow Area =
 2.186 ac, 78.25% Impervious, Inflow Depth =
 3.79" for 10-Yr Storm event

 Inflow =
 3.49 cfs @
 12.12 hrs, Volume=
 0.690 af

 Outflow =
 3.49 cfs @
 12.12 hrs, Volume=
 0.690 af, Atten= 0%, Lag= 0.0 min

 Primary =
 3.49 cfs @
 12.12 hrs, Volume=
 0.690 af

 Routed to Link SP-2 :
 0.690 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 80.82' @ 12.12 hrs Flood Elev= 96.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	79.80'	18.0" Round Culvert L= 55.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 79.80' / 78.00' S= 0.0327 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=3.48 cfs @ 12.12 hrs HW=80.82' (Free Discharge) -1=Culvert (Inlet Controls 3.48 cfs @ 2.72 fps)

Summary for Pond DMH6: DMH-6

Inflow Are	a =	0.235 ac, 3	36.02% Impervious	, Inflow Depth =	2.78"	for 10-Yr Storm event
Inflow	=	0.56 cfs @	12.12 hrs, Volum	e= 0.055	af	
Outflow	=	0.56 cfs @	12.12 hrs, Volum	e= 0.055	af, Atter	n= 0%, Lag= 0.0 min
Primary	=	0.56 cfs @	12.12 hrs, Volum	e= 0.055	af	-
Routed	l to Ponc	DMH7 :				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 90.39' @ 12.12 hrs Flood Elev= 95.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.00'	15.0" Round Culvert L= 163.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.00' / 80.85' S= 0.0561 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=0.56 cfs @ 12.12 hrs HW=90.39' (Free Discharge) -1=Culvert (Inlet Controls 0.56 cfs @ 1.69 fps)

Summary for Pond DMH7:

Inflow Area = 0.458 ac, 67.13% Impervious, Inflow Depth = 3.50" for 10-Yr Storm event Inflow 1.26 cfs @ 12.14 hrs. Volume= 0.134 af = 1.26 cfs @ 12.14 hrs, Volume= Outflow = 0.134 af, Atten= 0%, Lag= 0.0 min 1.26 cfs @ 12.14 hrs, Volume= Primary = 0.134 af Routed to Pond DMH8 : DMH-8 Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 81.35' @ 12.14 hrs Flood Elev= 90.40' Device Routing Invert Outlet Devices #1 Primary 80.75' 18.0" Round Culvert L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 80.75' / 80.50' S= 0.0051 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=1.25 cfs @ 12.14 hrs HW=81.34' (Free Discharge) —1=Culvert (Barrel Controls 1.25 cfs @ 2.84 fps)

Summary for Pond DMH8: DMH-8

 Inflow Area =
 0.458 ac, 67.13% Impervious, Inflow Depth =
 3.50" for 10-Yr Storm event

 Inflow =
 1.26 cfs @
 12.14 hrs, Volume=
 0.134 af

 Outflow =
 1.26 cfs @
 12.14 hrs, Volume=
 0.134 af, Atten= 0%, Lag= 0.0 min

 Primary =
 1.26 cfs @
 12.14 hrs, Volume=
 0.134 af

 Routed to Reach 1R :
 12.14 hrs, Volume=
 0.134 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 80.99' @ 12.14 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.40'	18.0" Round Culvert L= 93.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 80.40' / 80.00' S= 0.0043 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=1.25 cfs @ 12.14 hrs HW=80.99' (Free Discharge) -1=Culvert (Barrel Controls 1.25 cfs @ 2.85 fps)

Summary for Pond PP-1: Pervious Pavers

 Inflow Area =
 0.433 ac, 93.72% Impervious, Inflow Depth = 4.14" for 10-Yr Storm event

 Inflow =
 1.90 cfs @
 12.08 hrs, Volume=
 0.149 af

 Outflow =
 0.46 cfs @
 12.47 hrs, Volume=
 0.149 af, Atten= 76%, Lag= 22.9 min

 Primary =
 0.46 cfs @
 12.47 hrs, Volume=
 0.149 af

 Routed to Pond DMH3 : DMH-3
 0.149 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

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Peak Elev= 92.18' @ 12.47 hrs Surf.Area= 3,978 sf Storage= 2,144 cf Flood Elev= 95.00' Surf.Area= 3,978 sf Storage= 7,630 cf

Plug-Flow detention time= 74.8 min calculated for 0.149 af (100% of inflow) Center-of-Mass det. time= 74.5 min (838.9 - 764.4)

Volume	Inve	ert Ava	il.Storage	Storage Descrip	otion	
#1	90.8	33'	7,630 cf	Custom Stage	Data (Prismatio	c) Listed below (Recalc)
- ,		0 ()				
Elevatio		Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
90.8	33	3,978	0.0	0	0	
91.1	6	3,978	40.0	525	525	
91.5	50	3,978	40.0	541	1,066	
92.0	00	3,978	40.0	796	1,862	
93.0	00	3,978	40.0	1,591	3,453	
94.0	00	3,978	40.0	1,591	5,044	
94.5	50	3,978	30.0	597	5,641	
95.0	00	3,978	100.0	1,989	7,630	
Device	Routing	In	vert Out	let Devices		
#1	Primary	90).83' 12.0	" Round Culver	rt	
	-		L= 1	100.0' CPP, proj	ectina. no headv	vall. Ke= 0.900
						S= 0.0183 '/' Cc= 0.900
			n= (0.012, Flow Area	= 0.79 sf	
#2	Device 1	90		,		Limited to weir flow at low heads

Primary OutFlow Max=0.46 cfs @ 12.47 hrs HW=92.18' (Free Discharge)

-1=Culvert (Passes 0.46 cfs of 2.75 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.46 cfs @ 5.23 fps)

Summary for Pond PP-2: Pervious Pavers

 Inflow Area =
 0.059 ac, 90.30% Impervious, Inflow Depth =
 4.14" for 10-Yr Storm event

 Inflow =
 0.26 cfs @
 12.08 hrs, Volume=
 0.020 af

 Outflow =
 0.26 cfs @
 12.09 hrs, Volume=
 0.020 af

 Primary =
 0.26 cfs @
 12.09 hrs, Volume=
 0.020 af

 Routed to Pond DMH2 : DMH-2
 DMH-2

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.85' @ 12.09 hrs Surf.Area= 1,023 sf Storage= 9 cf Flood Elev= 97.50' Surf.Area= 1,023 sf Storage= 2,116 cf

Plug-Flow detention time= 0.6 min calculated for 0.020 af (100% of inflow) Center-of-Mass det. time= 0.6 min (765.0 - 764.4)

Volume	Invert	Avail.Storage	Storage Description
#1	92.83'	2,116 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

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Elevatio (fee		Surf.Area (sq-ft)	Voids (%)		Cum.Store (cubic-feet)	
92.8	33	1,023	0.0) 0	0	
93.3	33	1,023	40.0) 205	205	
94.0	00	1,023	40.0) 274	479	
95.0	00	1,023	40.0) 409	888	
95.5	50	1,023	40.0) 205	1,093	
96.0	00	1,023	40.0) 205	1,297	
97.0	00	1,023	30.0) 307	1,604	
97.5	50	1,023	100.0) 512	2,116	
Device	Routing	In	vert	Outlet Devices		
#1	Primary	90		12.0" Round Culv L= 100.0' CPP, pro	•••	vall. Ke= 0.900
					90.83'/89.00' \$	S= 0.0183 '/' Cc= 0.900
#2	Device 1	90		,		Limited to weir flow at low heads

Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=92.85' (Free Discharge)

1=Culvert (Passes 0.57 cfs of 3.68 cfs potential flow) **2=Orifice/Grate** (Orifice Controls 0.57 cfs @ 6.56 fps)

Summary for Pond PP-3: Pervious Pavers

Inflow Area	a =	0.113 ac,10	0.00% Impervious,	Inflow Depth =	4.36"	for 10-Yr Storr	n event
Inflow	=	0.50 cfs @	12.08 hrs, Volume	e= 0.041	af		
Outflow	=	0.47 cfs @	12.11 hrs, Volume	e= 0.041	af, Atte	en= 6%, Lag= 1	.8 min
Primary	=	0.47 cfs @	12.11 hrs, Volume	e= 0.041	af		
Routed	to Pond	DMH2 : DM	H-2				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.37' @ 12.11 hrs Surf.Area= 3,446 sf Storage= 55 cf Flood Elev= 96.50' Surf.Area= 3,446 sf Storage= 6,437 cf

Plug-Flow detention time= 1.9 min calculated for 0.041 af (100% of inflow) Center-of-Mass det. time= 1.9 min (751.4 - 749.4)

Volume	Invert	Ava	il.Storage	Storage Descri	ption	
#1	92.33'		6,437 cf	Custom Stage	Data (Prismatio	:) Listed below (Recalc)
Elevation	Surf	Area	Voids	Inc.Store	Cum.Store	
(feet)		(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
92.33	;	3,446	0.0	0	0	
92.83		3,446	40.0	689	689	
93.00	:	3,446	40.0	234	924	
94.00	:	3,446	40.0	1,378	2,302	
94.50	;	3,446	40.0	689	2,991	
95.00	;	3,446	40.0	689	3,680	
96.00	:	3,446	30.0	1,034	4,714	
96.50	:	3,446	100.0	1,723	6,437	

Type III 24-hr 10-Yr Storm Rainfall=4.60" Printed 6/13/2022

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Device	Routing	Invert	Outlet Devices
#1	Primary	90.83'	12.0" Round Culvert
	-		L= 100.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 90.83' / 89.00' S= 0.0183 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf
#2	Device 1	90.83'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.49 cfs @ 12.11 hrs HW=92.37' (Free Discharge)

-1=Culvert (Passes 0.49 cfs of 3.04 cfs potential flow)

1-2=Orifice/Grate (Orifice Controls 0.49 cfs @ 5.64 fps)

Summary for Pond UDSF-1: Filtration Basin

0.436 ac, 42.19% Impervious, Inflow Depth = 2.81" for 10-Yr Storm event Inflow Area = 1.43 cfs @ 12.09 hrs, Volume= 0.102 af Inflow = Outflow = 0.64 cfs @ 12.28 hrs, Volume= 0.102 af, Atten= 55%, Lag= 11.7 min Primary 0.64 cfs @ 12.28 hrs, Volume= 0.102 af = Routed to Reach 1R : Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Reach 1R :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 82.84' @ 12.28 hrs Surf.Area= 788 sf Storage= 746 cf

Plug-Flow detention time= 14.5 min calculated for 0.102 af (100% of inflow) Center-of-Mass det. time= 14.5 min (831.4 - 816.9)

Volume	Invert	Ava	il.Storage	Storage Descript	tion	
#1	80.33'		3,282 cf	Custom Stage Data (Prismatic)Listed below (Recalc)		
Elevatio	n Su	ırf.Area	Voids	Inc.Store	Cum.Store	
(fee		(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
80.3		<u>691</u>	0.0	0	0	
80.6		691	40.0	94	94	
81.0	00	691	40.0	91	185	
81.5	50	691	30.0	104	289	
82.0	00	691	30.0	104	392	
82.5		691	30.0	104	496	
83.0		835	100.0	382	878	
84.0		1,172	100.0	1,004	1,881	
85.0	00	1,629	100.0	1,401	3,282	
Device	Routing	In	vert Out	let Devices		
#1	Primary	80	.33' 12.	0" Round Culvert		
			L= :	25.0' CPP, square	e edge headwall,	Ke= 0.500
			Inle	t / Outlet Invert= 80	0.33 [°] /80.00'S=	= 0.0132 '/' Cc= 0.900
			n=	0.012, Flow Area=	0.79 sf	
#2	Device 1	80				mited to weir flow at low heads
#3	Device 1 84.50' 18.0" Horiz. Orifice/Grate C= 0.600					
				ited to weir flow at		
#4	Secondary	84				Ith Broad-Crested Rectangular Weir01.201.401.601.802.00

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Type III 24-hr 10-Yr Storm Rainfall=4.60"Printed 6/13/2022utions LLCPage 47

2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83

Primary OutFlow Max=0.64 cfs @ 12.28 hrs HW=82.84' (Free Discharge) 1=Culvert (Passes 0.64 cfs of 5.36 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.64 cfs @ 7.37 fps) 3=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.33' (Free Discharge) 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link SP-1:

Inflow Are	a =	2.373 ac, 78.91% Impervious, Inflow Depth = 3.39" for 10-Yr Storm event
Inflow	=	8.18 cfs @ 12.08 hrs, Volume= 0.671 af
Primary	=	8.18 cfs @ 12.08 hrs, Volume= 0.671 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Link SP-2:

Inflow Area	a =	4.667 ac, 52.49% Impervious, Inflow Depth = 3.19" for 10-Yr Storm event
Inflow	=	7.74 cfs @ 12.23 hrs, Volume= 1.240 af
Primary	=	7.74 cfs @ 12.23 hrs, Volume= 1.240 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Subcatchment 10S:

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Runoff 2.57 cfs @ 12.08 hrs, Volume= 0.211 af, Depth= 5.56" = Routed to Pond CB-1:

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

	Area (sf)	CN	Description		
*	10,475	98	Offsite impe	ervious	
*	9,057	98	Existing Im	pervious	
	285	74	>75% Gras	s cover, Go	ood, HSG C
	19,817 285 19,532		Weighted A 1.44% Perv 98.56% Imp	ious Area	rea
(m	Tc Length in) (feet)	Slope (ft/ft)		Capacity (cfs)	Description
6	6.0				Direct Entry,

Summary for Subcatchment 20S:

0.21 cfs @ 12.08 hrs, Volume= Runoff 0.017 af, Depth= 5.56" = Routed to Pond 2P:

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

	A	rea (sf)	CN	Description					
*		784	98	Existing Imp	pervious				
*		853	98	Offsite Impervious					
		1,637	98	98 Weighted Average					
		1,637		100.00% Im	npervious A	Area			
	Тс	Length	Slop		Capacity	Description			
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
	6.0					Direct Entry,			

Summary for Subcatchment 30S:

2.28 cfs @ 12.08 hrs, Volume= 0.181 af, Depth= 5.33" Runoff = Routed to Pond 3P :

Type III 24-hr 25-Yr Storm Rainfall=5.80" Printed 6/13/2022

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	А	rea (sf)	CN	Description							
*		13,779	98	Offsite Impe	ervious						
*		2,496	98	Impervious							
*		746	74	Offsite Land	lscaped						
		754	74								
		17,775	,775 96 Weighted Average								
		1,500		8.44% Pervious Area							
		16,275		91.56% Imp	ervious Ar	rea					
	Тс	Length	Slop	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft	t) (ft/sec)	(cfs)						
	6.0					Direct Entry,					

Summary for Subcatchment 40S:

Runoff	=	1.32 cfs @	12.09 hrs,	Volume=	0.099 af,	Depth= 4.65"
Routed	l to Pond	4P :				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

	Area (sf)	CN	Description		
*	7,341	98	Offsite Imper	rvious	
*	806	98	Existing Impe	ervious	
	1,646	74	>75% Grass	cover, Go	ood, HSG C
_	1,338	65	Brush, Good	, HSG C	
	11,131	90	Weighted Av		
	2,984		26.81% Perv	vious Area	а
	8,147		73.19% Impe	ervious Are	rea
	Tc Length (min) (feet)	Slop (ft/		Capacity (cfs)	•
	6.0				Direct Entry,

Summary for Subcatchment 50S:

Runoff = 0.98 cfs @ 12.08 hrs, Volume= 0.080 af, Depth= 5.56" Routed to Pond 5P :

	Area (sf)	CN	Description
*	5,773	98	Offsite Impervious
*	1,775	98	Existing Impervious
	7,548	98	Weighted Average
	7,548		100.00% Impervious Area

Prepare	20-001.1 Post-Development Type III 24-hr25-Yr Storm Rainfall=5.80"Prepared by Atlantic Resource ConsultantsPrinted 6/13/2022HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLCPage 50								
Tc (min)	Length (feet)	Slop (ft/f		ocity sec)	Capacity (cfs)	Description			
6.0						Direct Entr	ry,		
				Sun	nmary fo	r Subcatch	nment 60S:		
Runoff Route	= ed to Pond		cfs @	12.08	8 hrs, Volu	ime=	0.064 af, Dep	th= 5.56"	
	y SCS TR 24-hr 25-`					ited-CN, Tim	e Span= 0.00-7	2.00 hrs, dt= 0.03 hrs	
A	rea (sf)	CN	Descri	ption					
*	3,268	98			ervious				
*	2,700	98			pervious				
	5,968 5,968	98			verage pervious A	vrea			
Tc (min)	Length (feet)	Slop (ft/f		ocity sec)	Capacity (cfs)	Description	I		
6.0		•				Direct Ent	ry,		
				Sun	nmary fo	r Subcatch	nment 70S:		
Runoff Route	= ed to Pond		cfs @	12.0	9 hrs, Volu	ime=	0.115 af, Dep	th= 4.65"	
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"								
А	rea (sf)	CN	Descri	ption					
*	9,239	98			ervious				
	2,635	74	>75%	Gras	s cover, Go	ood, HSG C			
	1,040	65			d, HSG C				
	12,914	90	Weigh	ted A	verage				

	3,675 28.46% Pervious Area 9,239 71.54% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		

6.0

Direct Entry,

Summary for Subcatchment 80S:

Runoff = 1.43 cfs @ 12.00 hrs, Volume= 0.087 af, Depth= 4.01" Routed to Pond 7P :

Type III 24-hr25-Yr Storm Rainfall=5.80"Printed 6/13/2022lutions LLCPage 51

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	Area (sf)	CN	Description
*	9,056	98	Offsite Impervious
	2,284	30	Brush, Good, HSG A
	11,340	84	Weighted Average
	2,284		20.14% Pervious Area
	9,056		79.86% Impervious Area

Summary for Subcatchment 90S:

Runoff	=	0.29 cfs @	12.11 hrs,	Volume=	0.029 af,	Depth=	0.98"
Routed	l to Pond	19P :				-	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

	Area (sf)	CN	Description				
*	4,183	98	Offsite Impe	ervious area	a		
	11,072	30	Brush, Goo	d, HSG A			
	15,255	49	Weighted Average				
	11,072		72.58% Pe	rvious Area	3		
	4,183		27.42% Imp	pervious Ar	rea		
Т	c Length	Slope		Capacity	Description		
(mir	n) (feet)	(ft/ft)) (ft/sec)	(cfs)			
6.	0				Direct Entry,		

Summary for Subcatchment 100:

Runoff = 4.26 cfs @ 12.28 hrs, Volume= 0.450 af, Depth= 3.40" Routed to Link SP-2 :

	Area (sf)	CN	Description		
	41,443	74	>75% Grass cover, Good, HSG C		
*	16,896	74	DFSITE AREA TO BE UPDATED		
*	10,765	98	Offsite Impervious		
	69,104	78	Weighted Average		
	58,339		84.42% Pervious Area		
	10,765		15.58% Impervious Area		

20-001.1 Post-DevelopmentType III 24-hr25-Yr Storm Rainfall=5.80"Prepared by Atlantic Resource ConsultantsPrinted 6/13/2022HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLCPage 52Tc LengthSlopeVelocityCapacityDescription

	10	Lengui	Siope	velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	16.3	75	0.0930	0.08		Sheet Flow, A-B
						Woods: Dense underbrush n= 0.800 P2= 3.10"
	3.2	110	0.0540	0.58		Shallow Concentrated Flow, B-C
						Forest w/Heavy Litter Kv= 2.5 fps
	0.6	70	0.0100	1.95	1.95	Parabolic Channel, C-D
_						W=3.00' D=0.50' Area=1.0 sf Perim=3.2' n= 0.035
	20.1	255	Total			

Summary for Subcatchment 101S:

Runoff = 1.97 cfs @ 12.09 hrs, Volume= 0.142 af, Depth= 3.91" Routed to Pond UDSF-1 : Filtration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

Area	a (sf) CN	Description	Description						
8	,017 98								
3	,766 70	Woods, Go	od, HSG C						
7	,218 74	>75% Gras	s cover, Go	ood, HSG C					
19	,001 83	Weighted A	Verage						
10	,984		rvious Area	1					
8	,017	42.19% Im	pervious Ar	ea					
	•	ope Velocity t/ft) (ft/sec)	Capacity (cfs)	Description					
6.0				Direct Entry,					

Summary for Subcatchment 102S:

Runoff = 0.92 cfs @ 12.09 hrs, Volume= 0.068 af, Depth= 4.43" Routed to Pond DMH2 : DMH-2

Α	rea (sf)	CN	Description				
	4,611	98	Roofs, HSG	D			
	3,387	74 :	>75% Gras	s cover, Go	ood, HSG C		
	7,998	88	Weighted Average				
	3,387	4	12.35% Per	vious Area	3		
	4,611	:	57.65% Imp	ervious Are	rea		
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 103S:

Runoff = 2.42 cfs @ 12.08 hrs, Volume= 0.192 af, Depth= 5.33" Routed to Pond PP-1 : Pervious Pavers

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

A	rea (sf)	CN I	Description					
	17,688	98	Paved park	ing, HSG D)			
	1,185	74 :	>75% Gras	s cover, Go	bod, HSG C			
	18,873	96	Neighted A	verage				
	1,185	(6.28% Perv	rious Area				
	17,688	ę	93.72% Imp	pervious Are	ea			
-		~		o				
Tc	Length	Slope	,	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	/ft) (ft/sec) (cfs)					
6.0					Direct Entry,			
					-			

Summary for Subcatchment 104S:

Runoff = 2.67 cfs @ 12.08 hrs, Volume= Routed to Pond DF-1 : Drip Strip 0.219 af, Depth= 5.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

_	A	rea (sf)	CN	Description		
		18,803	98	Roofs, HSG	6 D	
*		1,786	98	Stone		
		20,589	98	Weighted A	verage	
		20,589		100.00% Im	pervious A	Area
	Tc	Length	Slop	e Velocity	Capacity	Description
_	(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)	
	6.0					Direct Entry,

Summary for Subcatchment 105S:

Runoff = 1.26 cfs @ 12.08 hrs, Volume= 0.103 af, Depth= 5.56" Routed to Pond DF-2 : Drip Strip

Type III 24-hr 25-Yr Storm Rainfall=5.80" Printed 6/13/2022

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	А	rea (sf)	CN	Description					
		8,810	98	Roofs, HSG D					
*		896	98	Stone					
		9,706 9,706	98	Weighted Average 100.00% Impervious Area					
	Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description			
	6.0					Direct Entry,			

Summary for Subcatchment 106S:

Runoff = 4.71 cfs @ 12.09 hrs, Volume= 0.350 af, Depth= 4.54" Routed to Pond 1P : R-TANK

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

Area (sf)	CN	Description					
21,170	98	Paved parki	ng, HSG D				
3,223	98	Roofs, HSG	Ď				
15,887	74	>75% Grass	s cover, Go	ood, HSG C			
40,280	89	Weighted Av	verage				
15,887		39.44% Per					
24,393		60.56% Imp	ervious Are	ea			
Tc Length (min) (feet)	Slop (ft/		Capacity (cfs)	Description			
6.0	•	<i>i i i</i>		Direct Entry,			
				-			

Summary for Subcatchment 107S:

Runoff = 0.65 cfs @ 12.09 hrs, Volume= 0.049 af, Depth= 4.76" Routed to Pond DF-3 : Drip Strip

	Area (sf)	CN	Description			
	3,214	98	Roofs, HSG D			
*	480	98	Stone			
	1,665	74	>75% Grass cover, Good, HSG C			
	5,359 1,665	91	Weighted Average 31.07% Pervious Area			
	3,694		68.93% Impervious Area			

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) Software Sol	utions LLC	Page 55		
Tc (min)									
6.0					Direct Entr	у,			
Summary for Subcatchment 108S:									
Runoff Route	= ed to Ponc			9 hrs, Volu	ime=	0.027 af,	Depth= 2.83"		
			nod, UH=S Rainfall=5		ted-CN, Time	e Span= 0.	00-72.00 hrs, dt= 0.03 hrs		
A	rea (sf)	CN D	escription						
	2,447				od, HSG C				
	2,448	70 V	Voods, Go	od, HSG C					
	4,895 4,895		Veighted A 00.00% Pe	verage ervious Are	а				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entr	у,			
	Summary for Subcatchment 109S:								

Type III 24-hr 25-Yr Storm Rainfall=5.80"

Runoff = 0.33 cfs @ 12.08 hrs, Volume= 0.026 af, Depth= 5.33" Routed to Pond PP-2 : Pervious Pavers

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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 25-Yr Storm Rainfall=5.80"

A	rea (sf)	CN	Description						
	2,326	98	Paved parking, HSG D						
	250	74	>75% Ġras	s cover, Go	ood, HSG C				
	2,576	96	Weighted Average						
	250		9.70% Perv	ious Area					
	2,326		90.30% Imp	pervious Ar	rea				
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment 110S:

Runoff	=	0.64 cfs @	12.08 hrs,	Volume=	0.052 af,	Depth= 5.56"
Routed	l to Pond	d PP-3 : Pervi	ous Pavers			

Type III 24-hr 25-Yr Storm Rainfall=5.80" Printed 6/13/2022

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Area (sf) CN Description					
4,907 98 Paved parking, HSG D					
4,907 100.00% Impervious Area					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
6.0 Direct Entry,					
Summary for Reach 1R:					
Inflow Area = 0.894 ac, 54.97% Impervious, Inflow Depth = 4.27" for 25-Yr Storm event Inflow = 2.26 cfs @ 12.14 hrs, Volume= 0.319 af Outflow = 2.25 cfs @ 12.16 hrs, Volume= 0.319 af, Atten= 0%, Lag= 1.2 min Routed to Link SP-2 :					
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Max. Velocity= 1.55 fps, Min. Travel Time= 1.9 min Avg. Velocity = 0.43 fps, Avg. Travel Time= 6.7 min					
Peak Storage= 251 cf @ 12.16 hrs Average Depth at Peak Storage= 0.07' , Surface Width= 20.71' Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 60.16 cfs					
20.00' x 0.50' deep channel, n= 0.035 Earth, dense weeds Side Slope Z-value= 5.0 '/' Top Width= 25.00' Length= 173.0' Slope= 0.0462 '/' Inlet Invert= 80.00', Outlet Invert= 72.00'					
*					
Summary for Pond 1P: R-TANK					

Inflow Area	a =	0.925 ac, 6	0.56% Imp	ervious,	Inflow D	Depth =	4.54"	for 25-Y	r Storm event
Inflow	=	4.71 cfs @	12.09 hrs,	Volume	=	0.350	af		
Outflow	=	1.35 cfs @	12.43 hrs,	Volume	=	0.350	af, Atte	en= 71%,	Lag= 20.4 min
Primary	=	1.35 cfs @	12.43 hrs,	Volume	=	0.350	af		•
Routed	to Pond	DMH3 : DMI	H-3						

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 90.32' @ 12.43 hrs Surf.Area= 3,593 sf Storage= 4,441 cf Flood Elev= 91.30' Surf.Area= 3,593 sf Storage= 6,075 cf

Plug-Flow detention time= 40.6 min calculated for 0.350 af (100% of inflow) Center-of-Mass det. time= 40.3 min (829.6 - 789.3)

Type III 24-hr 25-Yr Storm Rainfall=5.80" Printed 6/13/2022

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Volume	Invert	Avail.Storage	Storage Description
#1	85.30'	1,131 cf	Custom Stage Data (Prismatic)Listed below (Recalc)
#2A	88.22'	1,396 cf	31.56'W x 62.65'L x 4.07'H Field A
			8,049 cf Overall - 4,560 cf Embedded = 3,489 cf x 40.0% Voids
#3A	88.47'	4,332 cf	ACF R-Tank HD 2 x 525 Inside #2
			Inside= 15.7"W x 33.9"H => 3.52 sf x 2.35'L = 8.3 cf
			Outside= 15.7"W x 33.9"H => 3.70 sf x 2.35'L = 8.7 cf
			525 Chambers in 21 Rows
		6 859 cf	Total Available Storage

6,859 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.30	1,616	0.0	0	0
85.63	1,616	40.0	213	213
85.97	1,616	20.0	110	323
86.47	1,616	20.0	162	485
86.97	1,616	20.0	162	646
87.47	1,616	20.0	162	808
87.97	1,616	20.0	162	970
88.47	1,616	20.0	162	1,131

Device	Routing	Invert	Outlet Devices
#1	Primary	85.10'	15.0" Round Culvert
			L= 126.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 85.10' / 83.50' S= 0.0127 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.23 sf
#2	Device 1	85.30'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	90.60'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
			4.0' Crest Height
#4	Device 1	90.00'	12.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=1.35 cfs @ 12.43 hrs HW=90.32' (Free Discharge)

-**1=Culvert** (Passes 1.35 cfs of 10.00 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.93 cfs @ 10.61 fps)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.42 cfs @ 1.93 fps)

Summary for Pond 2P:

Inflow Area = 1.918 ac, 74.25% Impervious, Inflow Depth = 4.20" for 25-Yr Storm event Inflow 8.14 cfs @ 12.08 hrs, Volume= 0.672 af = Outflow 8.14 cfs @ 12.08 hrs, Volume= =
 8.14 cfs @
 12.08 hrs, Volume=
 0.672 af,

 8.14 cfs @
 12.08 hrs, Volume=
 0.672 af
 0.672 af, Atten= 0%, Lag= 0.0 min Primary = Routed to Pond CB-1:

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 98.57' @ 12.08 hrs Flood Elev= 95.86'

20-001.1 Post-Development Prepared by Atlantic Resource Consultants

Type III 24-hr 25-Yr Storm Rainfall=5.80" Printed 6/13/2022 HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLC Page 58

Device Routing Invert Outlet Devices
#1 Primary 90.65' 12.0" Round Culvert L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.65' / 90.00' S= 0.0103 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf
Primary OutFlow Max=8.05 cfs @ 12.08 hrs HW=98.41' (Free Discharge) 1=Culvert (Inlet Controls 8.05 cfs @ 10.24 fps)
Summary for Pond 3P:
Inflow Area = 1.881 ac, 73.74% Impervious, Inflow Depth = 4.18" for 25-Yr Storm event Inflow = 7.93 cfs @ 12.08 hrs, Volume= 0.655 af Outflow = 7.93 cfs @ 12.08 hrs, Volume= 0.655 af, Atten= 0%, Lag= 0.0 min Primary = 7.93 cfs @ 12.08 hrs, Volume= 0.655 af Routed to Pond 2P : 12.08 hrs, Volume= 0.655 af
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 98.29' @ 12.08 hrs Flood Elev= 95.40'
Device Routing Invert Outlet Devices
#1 Primary 90.75' 12.0'' Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.29' / 90.75' S= -0.0092 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf
Primary OutFlow Max=7.84 cfs @ 12.08 hrs HW=98.14' (Free Discharge) ☐1=Culvert (Inlet Controls 7.84 cfs @ 9.98 fps)
Summary for Pond 4P:
Inflow Area = 1.473 ac, 68.80% Impervious, Inflow Depth = 3.86" for 25-Yr Storm event Inflow = 5.67 cfs @ 12.07 hrs, Volume= 0.473 af Outflow = 5.67 cfs @ 12.07 hrs, Volume= 0.473 af, Atten= 0%, Lag= 0.0 min Primary = 5.67 cfs @ 12.07 hrs, Volume= 0.473 af Routed to Pond 3P : 5.67 cfs @ 12.07 hrs, Volume=
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 94.96' @ 12.07 hrs Flood Elev= 96.14'
Device Routing Invert Outlet Devices
#1 Primary 90.85' 12.0" Round Culvert L= 77.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.85' / 90.29' S= 0.0073 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=5.61 cfs @ 12.07 hrs HW=94.88' (Free Discharge) **1=Culvert** (Inlet Controls 5.61 cfs @ 7.14 fps)

20-001.1 Post-Development Prepared by Atlantic Resource Consultants HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCA									
Summary for Pond 5P:									
Inflow Area = 1.217 ac, 67.88% Imperv Inflow = 4.37 cfs @ 12.07 hrs, Vo Outflow = 4.37 cfs @ 12.07 hrs, Vo Primary = 4.37 cfs @ 12.07 hrs, Vo Routed to Pond 4P :	lume= 0.374 af, Atten= 0%, Lag= 0.0 min								
Routing by Stor-Ind method, Time Span= 0.00 Peak Elev= 93.77' @ 12.07 hrs Flood Elev= 95.65'	-72.00 hrs, dt= 0.03 hrs								
Device Routing Invert Outlet Dev	ices								
#1 Primary 91.13' 12.0" Rou L= 70.0' (Inlet / Outl	nd Culvert CPP, projecting, no headwall, Ke= 0.900 et Invert= 91.13' / 90.85' S= 0.0040 '/' Cc= 0.900 Flow Area= 0.79 sf								
#2 Primary 95.65' 24.0" x 24	0" Horiz. Orifice/Grate C= 0.600 weir flow at low heads								
Primary OutFlow Max=4.33 cfs @ 12.07 hrs 1=Culvert (Inlet Controls 4.33 cfs @ 5.52 f 2=Orifice/Grate (Controls 0.00 cfs)									
Sumn	ary for Pond 6P:								
Inflow Area = 0.397 ac, 86.80% Impervious Inflow = 1.95 cfs @ 12.01 hrs, Voc Outflow = 1.95 cfs @ 12.01 hrs, Voc Primary = 1.95 cfs @ 12.01 hrs, Voc Routed to Pond 5P : 12.01 hrs, Voc	lume= 0.150 af, Atten= 0%, Lag= 0.0 min								
Routing by Stor-Ind method, Time Span= 0.00 Peak Elev= 92.75' @ 12.01 hrs Flood Elev= 96.08'	-72.00 hrs, dt= 0.03 hrs								
Device Routing Invert Outlet Dev	ices								
L= 64.0' (Inlet / Outl	nd Culvert CPP, projecting, no headwall, Ke= 0.900 et Invert= 91.83' / 91.23' S= 0.0094 '/' Cc= 0.900 Flow Area= 0.79 sf								
Primary OutFlow Max=1.91 cfs @ 12.01 hrs 1=Culvert (Inlet Controls 1.91 cfs @ 2.55 f									

Summary for Pond 7P:

20-001.1 Post-DevelopmentType III 24-hr25-Yr Storm Rainfall=5.80"Prepared by Atlantic Resource ConsultantsPrinted 6/13/2022HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLCPage 60
Inflow Area = 0.260 ac, 79.86% Impervious, Inflow Depth = 4.01" for 25-Yr Storm event Inflow = 1.43 cfs @ 12.00 hrs, Volume= 0.087 af Outflow = 1.43 cfs @ 12.00 hrs, Volume= 0.087 af, Atten= 0%, Lag= 0.0 min Primary = 1.43 cfs @ 12.00 hrs, Volume= 0.087 af Routed to Pond 6P : 12.00 hrs, Volume= 0.087 af
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.22' @ 12.00 hrs Flood Elev= 97.40'
DeviceRoutingInvertOutlet Devices#1Primary92.43' 12.0" Round Culvert L= 105.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.43' / 92.03' S= 0.0038 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf
Primary OutFlow Max=1.42 cfs @ 12.00 hrs HW=93.21' (Free Discharge) 1=Culvert (Barrel Controls 1.42 cfs @ 2.96 fps)
Summary for Pond 8P:
Inflow Area = 0.647 ac, 47.65% Impervious, Inflow Depth = 2.66" for 25-Yr Storm event Inflow = 1.81 cfs @ 12.09 hrs, Volume= 0.143 af Outflow = 1.81 cfs @ 12.09 hrs, Volume= 0.143 af, Atten= 0%, Lag= 0.0 min Primary = 1.81 cfs @ 12.09 hrs, Volume= 0.143 af Routed to Pond 5P : 12.09 hrs, Volume= 0.143 af
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.03' @ 12.09 hrs Flood Elev= 97.36'
DeviceRoutingInvertOutlet Devices#1Primary92.16' 12.0'' Round Culvert L= 85.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.16' / 91.23' S= 0.0109 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=1.81 cfs @ 12.09 hrs HW=93.03' (Free Discharge) -1=Culvert (Inlet Controls 1.81 cfs @ 2.50 fps)

Summary for Pond 9P:

 Inflow Area =
 0.350 ac, 27.42% Impervious, Inflow Depth =
 0.98" for 25-Yr Storm event

 Inflow =
 0.29 cfs @
 12.11 hrs, Volume=
 0.029 af

 Outflow =
 0.29 cfs @
 12.11 hrs, Volume=
 0.029 af, Atten= 0%, Lag= 0.0 min

 Primary =
 0.29 cfs @
 12.11 hrs, Volume=
 0.029 af

 Routed to Pond 8P :
 0.029 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.36' @ 12.11 hrs Flood Elev= 96.37' 20-001.1 Post-Development Prepared by Atlantic Resource Consultants

Primary

Type III 24-hr 25-Yr Storm Rainfall=5.80" Printed 6/13/2022 HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLC Page 61

DeviceRoutingInvertOutlet Devices#1Primary93.06' 12.0'' Round Culvert L= 72.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 93.06' / 92.16'Ke= 0.900 S= 0.0125 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf						
Primary OutFlow Max=0.29 cfs @ 12.11 hrs HW=93.36' (Free Discharge) 1=Culvert (Inlet Controls 0.29 cfs @ 1.47 fps)						
Summary for Pond CB-1:						
Inflow Area = 2.373 ac, 78.91% Impervious, Inflow Depth = 4.46" for 25-Yr Storm event Inflow = 10.70 cfs @ 12.08 hrs, Volume= 0.883 af Outflow = 10.70 cfs @ 12.08 hrs, Volume= 0.883 af, Atten= 0%, Lag= 0.0 min Primary = 10.70 cfs @ 12.08 hrs, Volume= 0.883 af Routed to Link SP-1 : Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 95.83' @ 12.08 hrs Flood Elev= 95.55'						
Device Routing Invert Outlet Devices						
#1 Primary 89.95' 15.0" Round Culvert L= 35.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 89.95' / 89.50' S= 0.0129 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf						
Primary OutFlow Max=10.58 cfs @ 12.08 hrs HW=95.72' (Free Discharge) ←1=Culvert (Inlet Controls 10.58 cfs @ 8.62 fps)						
Summary for Pond DF-1: Drip Strip						
Inflow Area = 0.473 ac,100.00% Impervious, Inflow Depth = 5.56" for 25-Yr Storm event Inflow = 2.67 cfs @ 12.08 hrs, Volume= 0.219 af Outflow = 1.22 cfs @ 12.25 hrs, Volume= 0.215 af, Atten= 54%, Lag= 10.0 min						

1.22 cfs @ 12.25 hrs, Volume= 1.22 cfs @ 12.25 hrs, Volume= 0.215 af, Atten= 54%, Lag= 10.0 min = 0.215 af Routed to Pond DMH4 : DMH-4

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 85.10' @ 12.25 hrs Surf.Area= 1,786 sf Storage= 1,751 cf

Plug-Flow detention time= 41.9 min calculated for 0.215 af (98% of inflow) Center-of-Mass det. time= 29.7 min (775.4 - 745.7)

Volume	Invert	Avail.Storage	Storage Description
#1	81.83'	8,485 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Device

#1

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Elevation (feet) 81.83 82.16 94.00 94.25 94.75 95.75	Surf.Area (sq-ft) 1,786 1,786 1,786 1,786 1,786 1,786 1,786	Voids (%) 0.0 30.0 30.0 40.0 40.0 40.0	Inc.Store (cubic-feet) 0 177 6,344 179 357 714	Cum.Store (cubic-feet) 0 177 6,521 6,699 7,056 7,771
94.75	1,786	40.0	337	7,058
95.75	1,786	40.0	714	7,771
96.75	1,786	40.0	714	8,485

 Routing
 Invert
 Outlet Devices

 Primary
 82.16'
 6.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 82.16' / 81.00' S= 0.0232 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=1.22 cfs @ 12.25 hrs HW=85.10' (Free Discharge) **1=Culvert** (Inlet Controls 1.22 cfs @ 6.23 fps)

Summary for Pond DF-2: Drip Strip

Inflow Area	a =	0.223 ac,10	0.00% Impervious,	Inflow Depth =	5.56"	for 25-Yr	Storm event
Inflow	=	1.26 cfs @	12.08 hrs, Volume	e= 0.103	af		
Outflow	=	0.85 cfs @	12.17 hrs, Volume	e 0.101	af, Atte	n= 32%,	Lag= 5.2 min
Primary	=	0.85 cfs @	12.17 hrs, Volume	e= 0.101	af		
Routed	to Pond	I DMH7 :					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 83.71' @ 12.17 hrs Surf.Area= 896 sf Storage= 506 cf

Plug-Flow detention time= 34.5 min calculated for 0.101 af (98% of inflow) Center-of-Mass det. time= 21.6 min (767.3 - 745.7)

Volume	Inver	t Ava	il.Storage	Storage Descri	ption	
#1	81.83	5"	4,257 cf	Custom Stage	Data (Prismatio)Listed below (Recalc)
Elevation (feet)	S	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
81.83		896	0.0	0	0	
82.16		896	30.0	89	89	
94.00		896	30.0	3,183	3,271	
94.25		896	40.0	90	3,361	
94.75		896	40.0	179	3,540	
95.75		896	40.0	358	3,898	
96.75		896	40.0	358	4,257	
<u>Device R</u>	outing	In	vert Out	et Devices		
#1 P	rimary	82	L= 9 Inlet		cting, no headwa 82.16' / 80.00' S	all, Ke= 0.900 S= 0.0227 '/' Cc= 0.900

Primary OutFlow Max=0.85 cfs @ 12.17 hrs HW=83.71' (Free Discharge) -1=Culvert (Inlet Controls 0.85 cfs @ 4.33 fps)

Summary for Pond DF-3: Drip Strip

Inflow Area =	= (0.123 ac, 6	8.93% Impervious,	Inflow Depth =	4.76"	for 25-Yr Storm event
Inflow =	: C).65 cfs @	12.09 hrs, Volume	e= 0.049	af	
Outflow =	: C).43 cfs @	12.18 hrs, Volume	e= 0.049	af, Atte	en= 34%, Lag= 5.4 min
Primary =	: C).43 cfs @	12.18 hrs, Volume	e= 0.049	af	-
Routed to	Pond D	DMH6 : DMH	H- 6			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.91' @ 12.18 hrs Surf.Area= 1,786 sf Storage= 312 cf

Plug-Flow detention time= 28.1 min calculated for 0.049 af (100% of inflow) Center-of-Mass det. time= 27.7 min (809.8 - 782.1)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	93.	33'	2,859 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevatio 93.3 93.6 94.0 95.0 95.9 96.9 97.9	et) 33 66 00 00 50 50	Surf.Area (sq-ft) 1,786 1,786 1,786 1,786 1,786 1,786 1,786 1,786	Voids (%) 0.0 30.0 30.0 40.0 40.0 40.0 40.0 40.0	Inc.Store (cubic-feet) 0 177 182 714 357 714 714 714	Cum.Store (cubic-feet) 0 177 359 1,073 1,431 2,145 2,859	
Device #1	Routing Primary		3.33' 6.0''	et Devices Round Culvert 50.0' CPP, projec		I, Ke= 0.900
			Inlet)3.33'/92.00' S=	= 0.0266 '/' Cc= 0.900

Primary OutFlow Max=0.43 cfs @ 12.18 hrs HW=93.91' (Free Discharge) -1=Culvert (Inlet Controls 0.43 cfs @ 2.19 fps)

Summary for Pond DMH2: DMH-2

 Inflow Area =
 0.355 ac, 76.51% Impervious, Inflow Depth =
 4.94" for 25-Yr Storm event

 Inflow =
 1.74 cfs @
 12.09 hrs, Volume=
 0.146 af

 Outflow =
 1.74 cfs @
 12.09 hrs, Volume=
 0.146 af

 Primary =
 1.74 cfs @
 12.09 hrs, Volume=
 0.146 af

 Routed to Pond DMH3 : DMH-3
 0.146 af
 0.146 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Peak Elev= 86.68' @ 12.09 hrs Flood Elev= 94.25'

DeviceRoutingInvertOutlet Devices#1Primary86.00'**18.0" Round Culvert**
L= 150.0' CPP, projecting, no headwall, Ke= 0.900
Inlet / Outlet Invert= 86.00' / 83.50' S= 0.0167 '/' Cc= 0.900
n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=1.73 cfs @ 12.09 hrs HW=86.68' (Free Discharge) -1=Culvert (Inlet Controls 1.73 cfs @ 2.22 fps)

Summary for Pond DMH3: DMH-3

 Inflow Area =
 1.713 ac, 72.25% Impervious, Inflow Depth =
 4.82" for 25-Yr Storm event

 Inflow =
 2.99 cfs @
 12.10 hrs, Volume=
 0.689 af

 Outflow =
 2.99 cfs @
 12.10 hrs, Volume=
 0.689 af, Atten= 0%, Lag= 0.0 min

 Primary =
 2.99 cfs @
 12.10 hrs, Volume=
 0.689 af, Atten= 0%, Lag= 0.0 min

 Primary =
 2.99 cfs @
 12.10 hrs, Volume=
 0.689 af

 Routed to Pond DMH4 : DMH-4
 0.689 af
 0.689 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 82.46' @ 12.10 hrs Flood Elev= 96.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	81.51'	18.0" Round Culvert L= 82.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 81.51' / 81.10' S= 0.0050 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=2.97 cfs @ 12.10 hrs HW=82.46' (Free Discharge) ←1=Culvert (Barrel Controls 2.97 cfs @ 3.59 fps)

Summary for Pond DMH4: DMH-4

Inflow Area = 2.186 ac, 78.25% Impervious, Inflow Depth = 4.96" for 25-Yr Storm event Inflow = 4.07 cfs @ 12.12 hrs, Volume= 0.904 af Outflow = 4.07 cfs @ 12.12 hrs, Volume= 0.904 af, Atten= 0%, Lag= 0.0 min Primary = 4.07 cfs @ 12.12 hrs, Volume= 0.904 af Routed to Pond DMH5 : DMH-5

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 82.13' @ 12.12 hrs Flood Elev= 96.50'

Device	Routing	Invert	Outlet Devices
<u>=====</u> #1	Primary		18.0" Round Culvert L= 213.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 81.00' / 79.90' S= 0.0052 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=4.06 cfs @ 12.12 hrs HW=82.13' (Free Discharge) -1=Culvert (Inlet Controls 4.06 cfs @ 2.85 fps)

Summary for Pond DMH5: DMH-5

 Inflow Area =
 2.186 ac, 78.25% Impervious, Inflow Depth = 4.96" for 25-Yr Storm event

 Inflow =
 4.07 cfs @
 12.12 hrs, Volume=
 0.904 af

 Outflow =
 4.07 cfs @
 12.12 hrs, Volume=
 0.904 af

 Primary =
 4.07 cfs @
 12.12 hrs, Volume=
 0.904 af

 Routed to Link SP-2 :
 0.904 af
 0.904 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 80.93' @ 12.12 hrs Flood Elev= 96.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	79.80'	18.0" Round Culvert L= 55.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 79.80' / 78.00' S= 0.0327 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=4.06 cfs @ 12.12 hrs HW=80.93' (Free Discharge) -1=Culvert (Inlet Controls 4.06 cfs @ 2.85 fps)

Summary for Pond DMH6: DMH-6

Inflow Area	=	0.235 ac, 3	36.02% Impervious	, Inflow Depth =	3.84"	for 25-Yr Storm eve	ent	
Inflow	=	0.76 cfs @	12.11 hrs, Volum	e= 0.075	af			
Outflow	=	0.76 cfs @	12.11 hrs, Volum	e= 0.075	af, Atte	n= 0%, Lag= 0.0 mi	in	
Primary	=	0.76 cfs @	12.11 hrs, Volum	e= 0.075	af	-		
Routed to Pond DMH7 :								

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 90.46' @ 12.11 hrs Flood Elev= 95.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.00'	15.0" Round Culvert L= 163.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.00' / 80.85' S= 0.0561 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=0.75 cfs @ 12.11 hrs HW=90.46' (Free Discharge)

Summary for Pond DMH7:

Inflow Area = 0.458 ac, 67.13% Impervious, Inflow Depth = 4.62" for 25-Yr Storm event Inflow 1.58 cfs @ 12.13 hrs. Volume= 0.177 af = 1.58 cfs @ 12.13 hrs, Volume= Outflow = 0.177 af, Atten= 0%, Lag= 0.0 min 1.58 cfs @ 12.13 hrs, Volume= Primary = 0.177 af Routed to Pond DMH8 : DMH-8 Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 81.43' @ 12.13 hrs Flood Elev= 90.40' Device Routing Invert **Outlet Devices** #1 Primary 80.75' 18.0" Round Culvert L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 80.75' / 80.50' S= 0.0051 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=1.57 cfs @ 12.13 hrs HW=81.43' (Free Discharge) **1=Culvert** (Barrel Controls 1.57 cfs @ 2.99 fps)

Summary for Pond DMH8: DMH-8

0.458 ac, 67.13% Impervious, Inflow Depth = 4.62" for 25-Yr Storm event Inflow Area = Inflow 1.58 cfs @ 12.13 hrs, Volume= 0.177 af = 1.58 cfs @ 12.13 hrs, Volume= Outflow = 0.177 af, Atten= 0%, Lag= 0.0 min Primarv = 1.58 cfs @ 12.13 hrs, Volume= 0.177 af Routed to Reach 1R :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 81.07' @ 12.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.40'	18.0" Round Culvert L= 93.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 80.40' / 80.00' S= 0.0043 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=1.57 cfs @ 12.13 hrs HW=81.07' (Free Discharge) ←1=Culvert (Barrel Controls 1.57 cfs @ 3.01 fps)

Summary for Pond PP-1: Pervious Pavers

Inflow Area = 0.433 ac, 93.72% Impervious, Inflow Depth = 5.33" for 25-Yr Storm event Inflow = 2.42 cfs @ 12.08 hrs, Volume= 0.192 af 0.53 cfs @ 12.49 hrs, Volume= Outflow = 0.192 af, Atten= 78%, Lag= 24.2 min 0.53 cfs @ 12.49 hrs, Volume= = Primarv 0.192 af Routed to Pond DMH3 : DMH-3

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

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Peak Elev= 92.60' @ 12.49 hrs Surf.Area= 3,978 sf Storage= 2,812 cf Flood Elev= 95.00' Surf.Area= 3,978 sf Storage= 7,630 cf

Plug-Flow detention time= 74.8 min calculated for 0.192 af (100% of inflow) Center-of-Mass det. time= 75.2 min (834.2 - 759.0)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	90.	83'	7,630 cf	Custom Stage	Data (Prismatio	c)Listed below (Recalc)
- 1		0	\/			
Elevatio		Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
90.8	33	3,978	0.0	0	0	
91. ⁻	16	3,978	40.0	525	525	
91.5	50	3,978	40.0	541	1,066	
92.0	00	3,978	40.0	796	1,862	
93.0	00	3,978	40.0	1,591	3,453	
94.(00	3,978	40.0	1,591	5,044	
94.5	50	3,978	30.0	597	5,641	
95.0	00	3,978	100.0	1,989	7,630	
Device	Routing	In	vert Out	let Devices		
#1	Primary	90).83' 12. 0)" Round Culver	rt	
	2		L= '	100.0' CPP, proj	ectina, no headv	vall. Ke= 0.900
					Q.	S= 0.0183 '/' Cc= 0.900
				0.012, Flow Area		
#2	Device 2	1 90		,		Limited to weir flow at low heads
#2	Device	1 90		vert. Office/Gr	ale 0-0.000	Limited to well now at low neads

Primary OutFlow Max=0.53 cfs @ 12.49 hrs HW=92.60' (Free Discharge)

-1=Culvert (Passes 0.53 cfs of 3.36 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.53 cfs @ 6.09 fps)

Summary for Pond PP-2: Pervious Pavers

 Inflow Area =
 0.059 ac, 90.30% Impervious, Inflow Depth =
 5.33" for 25-Yr Storm event

 Inflow =
 0.33 cfs @
 12.08 hrs, Volume=
 0.026 af

 Outflow =
 0.33 cfs @
 12.09 hrs, Volume=
 0.026 af, Atten= 1%, Lag= 0.6 min

 Primary =
 0.33 cfs @
 12.09 hrs, Volume=
 0.026 af

 Routed to Pond DMH2 : DMH-2
 DMH-2
 0.026 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.86' @ 12.09 hrs Surf.Area= 1,023 sf Storage= 11 cf Flood Elev= 97.50' Surf.Area= 1,023 sf Storage= 2,116 cf

Plug-Flow detention time= 0.6 min calculated for 0.026 af (100% of inflow) Center-of-Mass det. time= 0.6 min (759.5 - 759.0)

Volume	Invert	Avail.Storage	Storage Description
#1	92.83'	2,116 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

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Elevatio (fee		Surf.Area (sq-ft)	Voids %)		Cum.Store (cubic-feet)		
92.8	33	1,023	0.0) 0	0		
93.3	33	1,023	40.0) 205	205		
94.0	00	1,023	40.0) 274	479		
95.0	00	1,023	40.0) 409	888		
95.5	50	1,023	40.0) 205	1,093		
96.0	00	1,023	40.0) 205	1,297		
97.0	00	1,023	30.0) 307	1,604		
97.5	50	1,023	100.0) 512	2,116		
Device	Routing	In	vert	Outlet Devices			
#1	Primary	90).83'	12.0" Round Culv	ert		
	-			L= 100.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.83' / 89.00' S= 0.0183 '/' Cc= 0.900			
#2	Device 1	90		n= 0.012, Flow Area= 0.79 sf 4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads			

Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=92.86' (Free Discharge)

1=Culvert (Passes 0.57 cfs of 3.69 cfs potential flow) **2=Orifice/Grate** (Orifice Controls 0.57 cfs @ 6.57 fps)

Summary for Pond PP-3: Pervious Pavers

Inflow Area	a =	0.113 ac,10	0.00% Impervious,	Inflow Depth =	5.56"	for 25-Yr	Storm event	
Inflow	=	0.64 cfs @	12.08 hrs, Volume	e= 0.052	af			
Outflow	=	0.50 cfs @	12.15 hrs, Volume	e= 0.052	af, Atte	en= 22%,	Lag= 3.8 min	
Primary	=	0.50 cfs @	12.15 hrs, Volume	e= 0.052	af			
Routed to Pond DMH2 : DMH-2								

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.40' @ 12.15 hrs Surf.Area= 3,446 sf Storage= 90 cf Flood Elev= 96.50' Surf.Area= 3,446 sf Storage= 6,437 cf

Plug-Flow detention time= 2.0 min calculated for 0.052 af (100% of inflow) Center-of-Mass det. time= 2.0 min (747.7 - 745.7)

Volume	Invert	Avai	I.Storage	Storage Descr	iption	
#1	92.33'		6,437 cf	Custom Stage	e Data (Prismatio	c) Listed below (Recalc)
Elevation	Surf.	Area	Voids	Inc.Store	Cum.Store	
(feet)	(9	sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
92.33	3	3,446	0.0	0	0	
92.83	3	3,446	40.0	689	689	
93.00	3	3,446	40.0	234	924	
94.00	3	3,446	40.0	1,378	2,302	
94.50	3	3,446	40.0	689	2,991	
95.00	3	3,446	40.0	689	3,680	
96.00	3	3,446	30.0	1,034	4,714	
96.50	3	3,446	100.0	1,723	6,437	
94.50 95.00 96.00	3 3 3	8,446 8,446 8,446	40.0 40.0 30.0	689 689 1,034	2,991 3,680 4,714	

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Device	Routing	Invert	Outlet Devices
#1	Primary	90.83'	12.0" Round Culvert
	-		L= 100.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 90.83' / 89.00' S= 0.0183 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf
#2	Device 1	90.83'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.50 cfs @ 12.15 hrs HW=92.40' (Free Discharge)

-1=Culvert (Passes 0.50 cfs of 3.08 cfs potential flow)

1–2=Orifice/Grate (Orifice Controls 0.50 cfs @ 5.69 fps)

Summary for Pond UDSF-1: Filtration Basin

Inflow Area = 0.436 ac, 42.19% Impervious, Inflow Depth = 3.91" for 25-Yr Storm event 1.97 cfs @ 12.09 hrs, Volume= 0.142 af Inflow = 0.71 cfs @ 12.36 hrs, Volume= Outflow = 0.142 af, Atten= 64%, Lag= 16.2 min Primary = 0.71 cfs @ 12.36 hrs, Volume= 0.142 af Routed to Reach 1R : Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Reach 1R :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 83.39' @ 12.36 hrs Surf.Area= 967 sf Storage= 1,230 cf

Plug-Flow detention time= 16.7 min calculated for 0.142 af (100% of inflow) Center-of-Mass det. time= 16.8 min (824.3 - 807.6)

Volume	Invert	Ava	il.Storage	Storage Descrip	tion					
#1	80.33'		3,282 cf	Custom Stage I	Data (Prismatic)	Listed below (Recalc)				
Elevatior (feet		rf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)					
80.33	1	691	0.0	0	0					
80.67	7	691	40.0	94	94					
81.00)	691	40.0	91	185					
81.50)	691	30.0	104	289					
82.00)	691	30.0	104	392					
82.50)	691	30.0	104	496					
83.00)	835	100.0	382	878					
84.00		1,172	100.0	1,004	1,881					
85.00)	1,629	100.0	1,401	3,282					
Device	Routing	In	vert Ou	tlet Devices						
#1	Primary	80	.33' 12.	0" Round Culver	t					
	-		L=	25.0' CPP, square	e edge headwall,	Ke= 0.500				
			Inle	et / Outlet Invert= 8	0.33 [°] / 80.00' S:	= 0.0132 '/' Cc= 0.900				
n			n=	n= 0.012, Flow Area= 0.79 sf						
					imited to weir flow at low heads					
#3	Device 1	84	84.50' 18.0" Horiz. Orifice/Grate C= 0.600							
				nited to weir flow at						
#4	Secondary	84		10.0' long + 3.0 '/' SideZ x 6.0' breadth Broad-Crested Rectangular W Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00						

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2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83

Primary OutFlow Max=0.71 cfs @ 12.36 hrs HW=83.39' (Free Discharge) -**1=Culvert** (Passes 0.71 cfs of 6.05 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.71 cfs @ 8.19 fps) -3=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.33' (Free Discharge) 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link SP-1:

Inflow Are	a =	2.373 ac, 78.91% Impervious, Inflow Depth = 4.46" for 25-Yr Storm event
Inflow	=	10.70 cfs @ 12.08 hrs, Volume= 0.883 af
Primary	=	10.70 cfs @ 12.08 hrs, Volume= 0.883 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Link SP-2:

Inflow Are	a =	4.667 ac, 52.49% Impervious, Inflow Depth = 4.30" for 25-Yr Storm event	t
Inflow	=	10.08 cfs @ 12.25 hrs, Volume= 1.672 af	
Primary	=	10.08 cfs @ 12.25 hrs, Volume= 1.672 af, Atten= 0%, Lag= 0.0 min	

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Subcatchment 10S:

Runoff = 3.60 cfs @ 12.08 hrs, Volume= 0.298 af, Depth= 7.86" Routed to Pond CB-1 :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

	A	rea (sf)	CN I	Description		
*		10,475	98 (Offsite impe	ervious	
*		9,057	98 I	Existing Imp	pervious	
		285	74 >	>75% Gras	s cover, Go	ood, HSG C
		19,817 285		Veighted A I.44% Perv		
		19,532			bervious Area	rea
		.0,002				
	Тс	Length	Slope	Velocity	Capacity	1
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Summary for Subcatchment 20S:

Runoff = 0.30 cfs @ 12.08 hrs, Volume= 0.025 af, Depth= 7.86" Routed to Pond 2P :

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

_	A	rea (sf)	CN	Description		
*		784	98	Existing Imp	pervious	
*		853	98	Offsite Impe	ervious	
		1,637	98	Weighted A	verage	
		1,637		100.00% Im	npervious A	rea
	Тс	Length	Slope	e Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	6.0					Direct Entry,

Summary for Subcatchment 30S:

Runoff = 3.20 cfs @ 12.08 hrs, Volume= 0.259 af, Depth= 7.62" Routed to Pond 3P :

Type III 24-hr 100-Yr Storm Rainfall=8.10" Printed 6/13/2022

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	А	rea (sf)	CN	Description		
*		13,779	98	Offsite Impe	ervious	
*		2,496	98	Impervious		
*		746	74	Offsite Land	lscaped	
		754	74	>75% Gras	s cover, Go	ood, HSG C
		17,775	96	Weighted A	verage	
		1,500		8.44% Perv		
		16,275		91.56% Imp	ervious Ar	rea
	Тс	Length	Slope	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	6.0					Direct Entry,

•

Summary for Subcatchment 40S:

Runoff	=	1.92 cfs @	12.09 hrs,	Volume=	0.147 af,	Depth= 6.90"
Routed	to Pond	I 4P :				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

	A	rea (sf)	CN	Description			
*		7,341	98	Offsite Impe	ervious		
*		806	98	Existing Im	pervious		
		1,646	74	>75% Gras	s cover, Go	ood, HSG C	
		1,338	65	Brush, Goo	d, HSG C		
		11,131	90	Weighted A	verage		
		2,984		26.81% Pei	vious Area		
		8,147		73.19% lmp	pervious Ar	ea	
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
	6.0					Direct Entry,	

Summary for Subcatchment 50S:

Runoff = 1.37 cfs @ 12.08 hrs, Volume= 0.113 af, Depth= 7.86" Routed to Pond 5P :

	Area (sf)	CN	Description
*	5,773	98	Offsite Impervious
*	1,775	98	Existing Impervious
	7,548	98	Weighted Average
	7,548		100.00% Impervious Area

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Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	<u>-g- · ·</u>							
6.0 Direct Entry,	6.0 Direct Entry,							
Summary for Subcatchment 60S:								
Runoff = 1.08 cfs @ 12.08 hrs, Volume= 0.090 af, Depth= 7.86" Routed to Pond 6P :								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"								
Area (sf) CN Description								
* 3,268 98 Offsite Impervious								
<u>* 2,700 98 Existing Impervious</u> 5,968 98 Weighted Average								
5,968 98 Weighted Average 5,968 100.00% Impervious Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)								
6.0 Direct Entry,								
Summary for Subcatchment 70S:								
Runoff = 2.23 cfs @ 12.09 hrs, Volume= 0.171 af, Depth= 6.90" Routed to Pond 8P :								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"								
Area (sf) CN Description								
* 9,239 98 Offsite Impervious								
2,635 74 >75% Grass cover, Good, HSG C								
1,040 65 Brush, Good, HSG C 12.914 90 Weighted Average								

1,040	00	DIUSII, GOOU, NGG C
12,914	90	Weighted Average
3,675		28.46% Pervious Area
9,239		71.54% Impervious Area
To Length	Slor	Ne Velocity Canacity Description

 <u>(IIIII)</u> 6.0	(leel)	(11/11)	(II/Sec)	(015)	Direct Entry,	_
IC (min)			Velocity (ft/sec)	Capacity (cfs)	Description	

Direct Entry,

Summary for Subcatchment 80S:

2.17 cfs @ 12.00 hrs, Volume= Runoff 0.134 af, Depth= 6.19" = Routed to Pond 7P :

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	Area (sf)	CN	Description
*	9,056	98	Offsite Impervious
	2,284	30	Brush, Good, HSG A
	11,340	84	Weighted Average
	2,284		20.14% Pervious Area
	9,056		79.86% Impervious Area

Summary for Subcatchment 90S:

Runoff	=	0.82 cfs @	12.10 hrs,	Volume=	(0.064 af, Depth=	= 2.21"
Routed	to Pond	9P:					

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

_	Ai	rea (sf)	CN	Description		
*		4,183	98	Offsite Impe	ervious area	a
		11,072	30	Brush, Goo	d, HSG A	
		15,255	49	Weighted A	verage	
		11,072		72.58% Pei	vious Area	3
		4,183		27.42% Imp	pervious Are	rea
	_		~		• •	-
	ŢĊ	Length	Slope		Capacity	Description
_	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	6.0					Direct Entry,
						• '

Summary for Subcatchment 100:

Runoff = 6.81 cfs @ 12.27 hrs, Volume= 0.725 af, Depth= 5.48" Routed to Link SP-2 :

	Area (sf)	CN	Description			
	41,443	74	>75% Grass cover, Good, HSG C			
*	16,896	74	OFSITE AREA TO BE UPDATED			
*	10,765	98	Offsite Impervious			
	69,104	78	Weighted Average			
	58,339		84.42% Pervious Area			
	10,765		15.58% Impervious Area			

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 Type III 24-hr 100-Yr Storm Rainfall=8.10"

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	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	16.3	75	0.0930	0.08		Sheet Flow, A-B
						Woods: Dense underbrush n= 0.800 P2= 3.10"
	3.2	110	0.0540	0.58		Shallow Concentrated Flow, B-C
						Forest w/Heavy Litter Kv= 2.5 fps
	0.6	70	0.0100	1.95	1.95	Parabolic Channel, C-D
_						W=3.00' D=0.50' Area=1.0 sf Perim=3.2' n= 0.035
	20.1	255	Total			

Summary for Subcatchment 101S:

Runoff = 3.00 cfs @ 12.09 hrs, Volume= 0.221 af, Depth= 6.07" Routed to Pond UDSF-1 : Filtration Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

Area	a (sf) CN	Description	Description				
8	,017 98						
3	,766 70	Woods, Go	od, HSG C				
7	,218 74	>75% Gras	s cover, Go	ood, HSG C			
19	,001 83	Weighted A	Verage				
10	,984		57.81% Pervious Area				
8	,017	42.19% Im	42.19% Impervious Area				
	•	ope Velocity t/ft) (ft/sec)	Capacity (cfs)	Description			
6.0				Direct Entry,			

Summary for Subcatchment 102S:

Runoff = 1.35 cfs @ 12.09 hrs, Volume= 0.102 af, Depth= 6.67" Routed to Pond DMH2 : DMH-2

Α	rea (sf)	CN	Description				
	4,611	98	Roofs, HSG	b D			
	3,387	74	>75% Gras	s cover, Go	ood, HSG C		
	7,998	88	Weighted Average				
	3,387		42.35% Pervious Area				
	4,611		57.65% Impervious Area				
Тс	Length	Slope	,	Capacity	1		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 103S:

Runoff = 3.40 cfs @ 12.08 hrs, Volume= 0.275 af, Depth= 7.62" Routed to Pond PP-1 : Pervious Pavers

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

A	rea (sf)	CN [Description					
	17,688	98 F	Paved park	ing, HSG D				
	1,185	74 >	•75% Gras	s cover, Go	bod, HSG C			
	18,873	96 V	Veighted A	verage				
	1,185	6	6.28% Pervious Area					
	17,688	ę	93.72% Imp	pervious Are	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			
					-			

Summary for Subcatchment 104S:

Runoff = 3.74 cfs @ 12.08 hrs, Volume= Routed to Pond DF-1 : Drip Strip 0.310 af, Depth= 7.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

	Area (sf)	CN	Description					
	18,803	98	Roofs, HSG	G D				
*	1,786	98	Stone					
	20,589	98	Weighted A	Weighted Average				
	20,589		100.00% In	npervious A	Area			
To	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			
					•			

Summary for Subcatchment 105S:

Runoff = 1.76 cfs @ 12.08 hrs, Volume= 0.146 af, Depth= 7.86" Routed to Pond DF-2 : Drip Strip

Type III 24-hr 100-Yr Storm Rainfall=8.10" Printed 6/13/2022

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	А	rea (sf)	CN	Description					
		8,810	98	Roofs, HSG	G D				
*		896	98	Stone					
		9,706	98	Weighted Average					
		9,706		100.00% Im	vrea				
	Тс	Length	Slop		Capacity	Description			
_	(min)	(feet)	(ft/f) (ft/sec)	(cfs)				
	6.0					Direct Entry,			

Summary for Subcatchment 106S:

Runoff = 6.88 cfs @ 12.09 hrs, Volume= 0.523 af, Depth= 6.78" Routed to Pond 1P : R-TANK

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

Are	ea (sf)	CN	Description				
2	1,170	98	Paved park	ing, HSG D			
	3,223	98	Roofs, HSC	δĎ			
1	5,887	74	>75% Gras	s cover, Go	bod, HSG C		
4	0,280	89	Weighted A	verage			
1	5,887		39.44% Pei				
2	4,393		60.56% Imp	pervious Are	ea		
	Length	Slope		Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 107S:

Runoff = 0.93 cfs @ 12.08 hrs, Volume= 0.072 af, Depth= 7.02" Routed to Pond DF-3 : Drip Strip

	Area (sf)	CN	Description
	3,214	98	Roofs, HSG D
*	480	98	Stone
	1,665	74	>75% Grass cover, Good, HSG C
	5,359 1,665	91	Weighted Average 31.07% Pervious Area
	3,694		68.93% Impervious Area

Prepare	d by Atla	Developr Intic Resol	urce Coi		<i>Type</i> Software Solution		<i>-hr 100-Yr Storm Rainfall=8.10"</i> Printed 6/13/2022
HYDIOCA	D® 10.20-	20 5/11 000 1			Soltware Solution	IS LLC	Page 78
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
6.0					Direct Entry,		
			Sum	mary for	Subcatchme	nt 10	8S:
Runoff Route	= ed to Pone	0.63 cfs (d DMH6 : D		9 hrs, Volu	me= 0.0	45 af,	Depth= 4.78"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"						.00-72.00 hrs, dt= 0.03 hrs	
A	rea (sf)		scription				
	2,447				od, HSG C		
	2,448	70 Wo	ods, Goo	od, HSG C			
	4,895		eighted Av				
	4,895	100	0.00% Pe	ervious Are	a		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
6.0					Direct Entry,		
			Sum	mary for	Subcatchme	nt 10	9S:

Runoff = 0.46 cfs @ 12.08 hrs, Volume= 0.038 af, Depth= 7.62" Routed to Pond PP-2 : Pervious Pavers

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Type III 24-hr 100-Yr Storm Rainfall=8.10"

A	rea (sf)	CN	Description			
	2,326	98	Paved park	ing, HSG D	D	
	250	74	>75% Ġras	s cover, Go	ood, HSG C	
	2,576	96	Weighted A	verage		
	250	1	9.70% Pervious Area			
	2,326		90.30% Imp	pervious Ar	rea	
Тс	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment 110S:

Runoff	=	0.89 cfs @	12.08 hrs,	Volume=	0.	074 af, I	Depth= 7.86"
Routed	l to Pond	d PP-3 : Pervi	ous Pavers				

Type III 24-hr 100-Yr Storm Rainfall=8.10" 20-001.1 Post-Development Prepared by Atlantic Resource Consultants Printed 6/13/2022 HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLC Area (sf) CN Description

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Area (sf) CN Description					
4,907 98 Paved parking, HSG D					
4,907 100.00% Impervious Area					
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)					
6.0 Direct Entry,					
Summary for Reach 1R:					
Inflow Area = 0.894 ac, 54.97% Impervious, Inflow Depth = 6.46" for 100-Yr Storm event Inflow = 2.92 cfs @ 12.13 hrs, Volume= 0.481 af Outflow = 2.91 cfs @ 12.16 hrs, Volume= 0.481 af, Atten= 0%, Lag= 1.4 min Routed to Link SP-2 :					
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Max. Velocity= 1.71 fps, Min. Travel Time= 1.7 min Avg. Velocity = 0.48 fps, Avg. Travel Time= 6.0 min					
Peak Storage= 293 cf @ 12.16 hrs Average Depth at Peak Storage= 0.08' , Surface Width= 20.83' Bank-Full Depth= 0.50' Flow Area= 11.3 sf, Capacity= 60.16 cfs					
20.00' x 0.50' deep channel, n= 0.035 Earth, dense weeds Side Slope Z-value= 5.0 '/' Top Width= 25.00' Length= 173.0' Slope= 0.0462 '/' Inlet Invert= 80.00', Outlet Invert= 72.00'					
* *					
Summary for Pond 1P: R-TANK					

Inflow Area	a =	0.925 ac, 6	0.56% Imper	rvious, Inflow	Depth = 6.78"	for 100-Yr Storm event
Inflow	=	6.88 cfs @	12.09 hrs, V	√olume=	0.523 af	
Outflow	=	4.42 cfs @	12.19 hrs, V	Volume=	0.523 af, Att	en= 36%, Lag= 6.1 min
Primary	=	4.42 cfs @	12.19 hrs, ∖	√olume=	0.523 af	
Routed	to Pond	DMH3 : DMI	H-3			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 90.82' @ 12.19 hrs Surf.Area= 3,593 sf Storage= 5,273 cf Flood Elev= 91.30' Surf.Area= 3,593 sf Storage= 6,075 cf

Plug-Flow detention time= 35.6 min calculated for 0.523 af (100% of inflow) Center-of-Mass det. time= 35.7 min (814.4 - 778.7)

Type III 24-hr 100-Yr Storm Rainfall=8.10" Printed 6/13/2022

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Volume	Invert	Avail.Storage	Storage Description
#1	85.30'	1,131 cf	Custom Stage Data (Prismatic)Listed below (Recalc)
#2A	88.22'	1,396 cf	31.56'W x 62.65'L x 4.07'H Field A
			8,049 cf Overall - 4,560 cf Embedded = 3,489 cf x 40.0% Voids
#3A	88.47'	4,332 cf	ACF R-Tank HD 2 x 525 Inside #2
			Inside= 15.7"W x 33.9"H => 3.52 sf x 2.35'L = 8.3 cf
			Outside= 15.7"W x 33.9"H => 3.70 sf x 2.35'L = 8.7 cf
			525 Chambers in 21 Rows
		6 859 cf	Total Available Storage

6,859 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.30	1,616	0.0	0	0
85.63	1,616	40.0	213	213
85.97	1,616	20.0	110	323
86.47	1,616	20.0	162	485
86.97	1,616	20.0	162	646
87.47	1,616	20.0	162	808
87.97	1,616	20.0	162	970
88.47	1,616	20.0	162	1,131

Device	Routing	Invert	Outlet Devices
#1	Primary	85.10'	15.0" Round Culvert
			L= 126.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 85.10' / 83.50' S= 0.0127 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.23 sf
#2	Device 1	85.30'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	90.60'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
			4.0' Crest Height
#4	Device 1	90.00'	12.0" Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=4.35 cfs @ 12.19 hrs HW=90.81' (Free Discharge)

-**1=Culvert** (Passes 4.35 cfs of 10.52 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.97 cfs @ 11.13 fps)

-3=Sharp-Crested Rectangular Weir (Weir Controls 1.28 cfs @ 1.52 fps)

-4=Orifice/Grate (Orifice Controls 2.10 cfs @ 3.07 fps)

Summary for Pond 2P:

Inflow Area = 1.918 ac, 74.25% Impervious, Inflow Depth = 6.28" for 100-Yr Storm event Inflow 12.05 cfs @ 12.08 hrs, Volume= 1.003 af = Outflow 12.05 cfs @ 12.08 hrs, Volume= =
 12.05 cfs @
 12.08 hrs, Volume=
 1.003 af,

 12.05 cfs @
 12.08 hrs, Volume=
 1.003 af
 1.003 af, Atten= 0%, Lag= 0.0 min Primary = Routed to Pond CB-1:

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 107.44' @ 12.08 hrs Flood Elev= 95.86'

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Type III 24-hr 100-Yr Storm Rainfall=8.10" Printed 6/13/2022 HydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLC Page 81

Device Routing Invert Outlet Devices
#1 Primary 90.65' 12.0" Round Culvert L= 63.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.65' / 90.00' S= 0.0103 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf
Primary OutFlow Max=11.92 cfs @ 12.08 hrs HW=107.08' (Free Discharge) -1=Culvert (Inlet Controls 11.92 cfs @ 15.17 fps)
Summary for Pond 3P:
Inflow Area = 1.881 ac, 73.74% Impervious, Inflow Depth = 6.24" for 100-Yr Storm event Inflow = 11.76 cfs @ 12.08 hrs, Volume= 0.979 af Outflow = 11.76 cfs @ 12.08 hrs, Volume= 0.979 af, Atten= 0%, Lag= 0.0 min Primary = 11.76 cfs @ 12.08 hrs, Volume= 0.979 af Routed to Pond 2P :
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 106.75' @ 12.08 hrs Flood Elev= 95.40'
Device Routing Invert Outlet Devices
#1 Primary 90.75' 12.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.29' / 90.75' S= -0.0092 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf
Primary OutFlow Max=11.62 cfs @ 12.08 hrs HW=106.41' (Free Discharge) -1=Culvert (Inlet Controls 11.62 cfs @ 14.80 fps)
Summary for Pond 4P:
Inflow Area = 1.473 ac, 68.80% Impervious, Inflow Depth = 5.86" for 100-Yr Storm event Inflow = 8.59 cfs @ 12.07 hrs, Volume= 0.719 af Outflow = 8.59 cfs @ 12.07 hrs, Volume= 0.719 af, Atten= 0%, Lag= 0.0 min Primary = 8.59 cfs @ 12.07 hrs, Volume= 0.719 af Routed to Pond 3P : 8.59 cfs @ 12.07 hrs, Volume=
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 99.62' @ 12.07 hrs Flood Elev= 96.14'
DeviceRoutingInvertOutlet Devices#1Primary90.85' 12.0'' Round Culvert L= 77.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.85' / 90.29' S= 0.0073 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=8.49 cfs @ 12.07 hrs HW=99.43' (Free Discharge) **1=Culvert** (Inlet Controls 8.49 cfs @ 10.81 fps)

20-001.1 Post-Development TypPrepared by Atlantic Resource ConsultantsHydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solution	oe III 24-hr 100-Yr Storm Rainfall=8.10" Printed 6/13/2022 ons LLC Page 82							
Summary for Pond 5P:								
Outflow = $6.70 \text{ cfs} (a) = 12.07 \text{ hrs}$, Volume= 0.	h = 5.64" for 100-Yr Storm event 572 af 572 af, Atten= 0%, Lag= 0.0 min 572 af							
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0. Peak Elev= 95.73' @ 12.07 hrs Flood Elev= 95.65'	03 hrs							
Device Routing Invert Outlet Devices								
#1 Primary 91.13' 12.0" Round Culvert L= 70.0' CPP, projecting, r	'90.85' S= 0.0040 '/' Cc= 0.900 sf //Grate C= 0.600							
Primary OutFlow Max=6.62 cfs @ 12.07 hrs HW=95.73' (Fre -1=Culvert (Inlet Controls 6.04 cfs @ 7.70 fps) -2=Orifice/Grate (Weir Controls 0.58 cfs @ 0.92 fps)	ee Discharge)							
Summary for Pond	6P:							
Outflow = 2.88 cfs (a) 12.01 hrs, Volume= 0.	224 af							
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0. Peak Elev= 93.26' @ 12.01 hrs Flood Elev= 96.08'	03 hrs							
DeviceRoutingInvertOutlet Devices#1Primary91.83' 12.0" Round Culvert L= 64.0' CPP, projecting, r Inlet / Outlet Invert= 91.83' / n= 0.012, Flow Area= 0.79	91.23' S= 0.0094 '/' Cc= 0.900							
Primary OutFlow Max=2.82 cfs @ 12.01 hrs HW=93.22' (Fre 1=Culvert (Inlet Controls 2.82 cfs @ 3.59 fps)	ee Discharge)							
Summary for Pond	7P:							

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Inflow Area = 0.260 ac, 79.86% Impervious, Inflow Depth = 6.19" for 100-Yr Storm event Inflow = 2.17 cfs @ 12.00 hrs, Volume= 0.134 af Outflow = 2.17 cfs @ 12.00 hrs, Volume= 0.134 af, Atten= 0%, Lag= 0.0 min Primary = 2.17 cfs @ 12.00 hrs, Volume= 0.134 af Routed to Pond 6P : 0.134 af
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.49' @ 12.00 hrs Flood Elev= 97.40'
Device Routing Invert Outlet Devices
#1 Primary 92.43' 12.0" Round Culvert L= 105.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.43' / 92.03' S= 0.0038 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf Primary OutFlow Max=2.16 cfs @ 12.00 hrs HW=93.48' (Free Discharge)
1=Culvert (Barrel Controls 2.16 cfs @ 3.24 fps)
Summary for Pond 8P:
Inflow Area = 0.647 ac, 47.65% Impervious, Inflow Depth = 4.36" for 100-Yr Storm event Inflow = 3.04 cfs @ 12.09 hrs, Volume= 0.235 af Outflow = 3.04 cfs @ 12.09 hrs, Volume= 0.235 af, Atten= 0%, Lag= 0.0 min Primary = 3.04 cfs @ 12.09 hrs, Volume= 0.235 af Routed to Pond 5P : 0.235 af
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.70' @ 12.09 hrs Flood Elev= 97.36'

Device	Routing	Invert	Outlet Devices
#1	Primary	92.16'	12.0" Round Culvert L= 85.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 92.16' / 91.23' S= 0.0109 '/' Cc= 0.900 n= 0.012, Flow Area= 0.79 sf

Primary OutFlow Max=3.03 cfs @ 12.09 hrs HW=93.69' (Free Discharge) -1=Culvert (Inlet Controls 3.03 cfs @ 3.86 fps)

Summary for Pond 9P:

 Inflow Area =
 0.350 ac, 27.42% Impervious, Inflow Depth =
 2.21" for 100-Yr Storm event

 Inflow =
 0.82 cfs @
 12.10 hrs, Volume=
 0.064 af

 Outflow =
 0.82 cfs @
 12.10 hrs, Volume=
 0.064 af, Atten= 0%, Lag= 0.0 min

 Primary =
 0.82 cfs @
 12.10 hrs, Volume=
 0.064 af

 Routed to Pond 8P :
 0.064 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 93.59' @ 12.10 hrs Flood Elev= 96.37'

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 Type III 24-hr
 100-Yr Storm Rainfall=8.10"

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HydroCA	ydroCAD® 10.20-2d s/n 08018 © 2021 HydroCAD Software Solutions LLC Page					
Device	Routing	Invert	Outlet Devices			
#1	Primary	93.06'	12.0" Round Culvert L= 72.0' CPP, projecting, no headwall, Ke= 0.9 Inlet / Outlet Invert= 93.06' / 92.16' S= 0.0125 '/ n= 0.012, Flow Area= 0.79 sf			
	Primary OutFlow Max=0.81 cfs @ 12.10 hrs HW=93.58' (Free Discharge)					
			Summary for Pond CB-1:			
Routing Peak Ele	= = = ed to Link by Stor-In	15.64 cfs @ 12 15.64 cfs @ 12 15.64 cfs @ 12 SP-1 : d method, Time 1' @ 12.08 hrs	91% Impervious, Inflow Depth = 6.58" for 100 2.08 hrs, Volume= 1.301 af 2.08 hrs, Volume= 1.301 af, Atten= 0%, 2.08 hrs, Volume= 1.301 af Span= 0.00-72.00 hrs, dt= 0.03 hrs			
Device	Routing	Invert	Outlet Devices			
#1	Primary	89.95'	15.0" Round Culvert L= 35.0' CPP, projecting, no headwall, Ke= 0.9 Inlet / Outlet Invert= 89.95' / 89.50' S= 0.0129 '/ n= 0.012, Flow Area= 1.23 sf			
Primary OutFlow Max=15.46 cfs @ 12.08 hrs HW=101.56' (Free Discharge) -1=Culvert (Inlet Controls 15.46 cfs @ 12.60 fps)						
		Sı	Immary for Pond DF-1: Drip Strip			
Inflow A	rea = =		00% Impervious, Inflow Depth = 7.86" for 100 2 08 hrs Volume= 0.310 af	-Yr Storm event		

 Inflow Area =
 0.473 ac,100.00% Impervious, Inflow Depth =
 7.86" for 100-Yr Storm event

 Inflow =
 3.74 cfs @
 12.08 hrs, Volume=
 0.310 af

 Outflow =
 1.54 cfs @
 12.29 hrs, Volume=
 0.306 af, Atten= 59%, Lag= 12.1 min

 Primary =
 1.54 cfs @
 12.29 hrs, Volume=
 0.306 af

 Routed to Pond DMH4 : DMH-4
 0.473 ac,100.00% Impervious, Inflow Depth =
 7.86" for 100-Yr Storm event

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 86.68' @ 12.29 hrs Surf.Area= 1,786 sf Storage= 2,599 cf

Plug-Flow detention time= 36.1 min calculated for 0.305 af (99% of inflow) Center-of-Mass det. time= 27.4 min (768.5 - 741.1)

Volume	Invert	Avail.Storage	Storage Description
#1	81.83'	8,485 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Device

#1

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Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.83	1,786	0.0	0	0
82.16	1,786	30.0	177	177
94.00	1,786	30.0	6,344	6,521
94.25	1,786	40.0	179	6,699
94.75	1,786	40.0	357	7,056
95.75	1,786	40.0	714	7,771
96.75	1,786	40.0	714	8,485

RoutingInvertOutlet DevicesPrimary82.16'6.0" Round Culvert
L= 50.0' CPP, projecting, no headwall, Ke= 0.900
Inlet / Outlet Invert= 82.16' / 81.00' S= 0.0232 '/' Cc= 0.900
n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=1.54 cfs @ 12.29 hrs HW=86.68' (Free Discharge) -1=Culvert (Inlet Controls 1.54 cfs @ 7.85 fps)

Summary for Pond DF-2: Drip Strip

Inflow Are	a =	0.223 ac,10	0.00% Impervious, Inflo	w Depth = 7.86"	for 100-Yr Storm event
Inflow	=	1.76 cfs @	12.08 hrs, Volume=	0.146 af	
Outflow	=	1.10 cfs @	12.18 hrs, Volume=	0.144 af, Atte	en= 38%, Lag= 6.0 min
Primary	=	1.10 cfs @	12.18 hrs, Volume=	0.144 af	
Routed	l to Ponc	DMH7 :			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 84.59' @ 12.18 hrs Surf.Area= 896 sf Storage= 741 cf

Plug-Flow detention time= 28.0 min calculated for 0.144 af (99% of inflow) Center-of-Mass det. time= 18.6 min (759.7 - 741.1)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	81.8	33'	4,257 cf	Custom Stage	Data (Prismatic) Listed below (Recalc)
Elevatio (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
81.8	3	896	0.0	0	0	
82.1	6	896	30.0	89	89	
94.0	0	896	30.0	3,183	3,271	
94.2	5	896	40.0	90	3,361	
94.7	5	896	40.0	179	3,540	
95.7	5	896	40.0	358	3,898	
96.7	5	896	40.0	358	4,257	
Device	Routing	In	vert Outl	et Devices		
#1	Primary	82	L= 9 Inlet	Round Culvert 5.0' CPP, project (Outlet Invert= 8 0.012, Flow Area	cting, no headwa 32.16' / 80.00' S	all, Ke= 0.900 S= 0.0227 '/' Cc= 0.900

Primary OutFlow Max=1.10 cfs @ 12.18 hrs HW=84.58' (Free Discharge) -1=Culvert (Inlet Controls 1.10 cfs @ 5.60 fps)

Summary for Pond DF-3: Drip Strip

Inflow Are	a =	0.123 ac, 68.93% Impervious, Inflow Depth = 7.02" for 100-Yr Storm event	t
Inflow	=	0.93 cfs @ 12.08 hrs, Volume= 0.072 af	
Outflow	=	0.56 cfs @ 12.19 hrs, Volume= 0.072 af, Atten= 41%, Lag= 6.5 min	
Primary	=	0.56 cfs @ 12.19 hrs, Volume= 0.072 af	
Routed	I to Pone	DMH6 : DMH-6	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 94.13' @ 12.19 hrs Surf.Area= 1,786 sf Storage= 454 cf

Plug-Flow detention time= 24.4 min calculated for 0.072 af (100% of inflow) Center-of-Mass det. time= 24.5 min (796.8 - 772.2)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	93.3	33'	2,859 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevatio (fee 93.3 93.6 94.0 95.0 95.5 96.5 97.5	et) 33 66 00 00 50 50	Surf.Area (sq-ft) 1,786 1,786 1,786 1,786 1,786 1,786 1,786 1,786	Voids (%) 30.0 30.0 40.0 40.0 40.0 40.0 40.0	Inc.Store (cubic-feet) 0 177 182 714 357 714 357 714 714	Cum.Store (cubic-feet) 0 177 359 1,073 1,431 2,145 2,859	
Device #1	Routing Primary		3.33' 6.0'' L= 5 Inlet	et Devices Round Culvert 50.0' CPP, project 7 Outlet Invert= 9 50.012, Flow Areas	cting, no headwa 93.33' / 92.00' S	II, Ke= 0.900 = 0.0266 '/' Cc= 0.900

Primary OutFlow Max=0.55 cfs @ 12.19 hrs HW=94.13' (Free Discharge) ←1=Culvert (Inlet Controls 0.55 cfs @ 2.82 fps)

Summary for Pond DMH2: DMH-2

 Inflow Area =
 0.355 ac, 76.51% Impervious, Inflow Depth =
 7.20" for 100-Yr Storm event

 Inflow =
 2.31 cfs @
 12.09 hrs, Volume=
 0.213 af

 Outflow =
 2.31 cfs @
 12.09 hrs, Volume=
 0.213 af, Atten= 0%, Lag= 0.0 min

 Primary =
 2.31 cfs @
 12.09 hrs, Volume=
 0.213 af, Atten= 0%, Lag= 0.0 min

 Primary =
 2.31 cfs @
 12.09 hrs, Volume=
 0.213 af

 Routed to Pond DMH3 : DMH-3
 0.213 af
 0.213 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Peak Elev= 86.80' @ 12.09 hrs Flood Elev= 94.25'

Device Routing Invert Outlet Devices #1 86.00' 18.0" Round Culvert Primary L= 150.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 86.00' / 83.50' S= 0.0167 '/' Cc= 0.900 n= 0.012. Flow Area= 1.77 sf

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Primary OutFlow Max=2.31 cfs @ 12.09 hrs HW=86.80' (Free Discharge) **1=Culvert** (Inlet Controls 2.31 cfs @ 2.40 fps)

Summary for Pond DMH3: DMH-3

Inflow Area = 1.713 ac, 72.25% Impervious, Inflow Depth = 7.08" for 100-Yr Storm event Inflow = 6.68 cfs @ 12.18 hrs, Volume= 1.011 af Outflow = 6.68 cfs @ 12.18 hrs, Volume= 1.011 af, Atten= 0%, Lag= 0.0 min 6.68 cfs @ 12.18 hrs, Volume= Primary = 1.011 af Routed to Pond DMH4 : DMH-4

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 83.25' @ 12.18 hrs Flood Elev= 96.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	81.51'	18.0" Round Culvert L= 82.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 81.51' / 81.10' S= 0.0050 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=6.67 cfs @ 12.18 hrs HW=83.24' (Free Discharge) **1=Culvert** (Inlet Controls 6.67 cfs @ 3.77 fps)

Summary for Pond DMH4: DMH-4

Inflow Area = 2.186 ac. 78.25% Impervious. Inflow Depth = 7.23" for 100-Yr Storm event 8.17 cfs @ 12.18 hrs, Volume= Inflow = 1.317 af 8.17 cfs @ 12.18 hrs, Volume= 1.317 af, Atten= 0%, Lag= 0.0 min Outflow = Primary = 8.17 cfs @ 12.18 hrs, Volume= 1.317 af Routed to Pond DMH5 : DMH-5

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 83.23' @ 12.18 hrs Flood Elev= 96.50'

Device	Routing	Invert	Outlet Devices
<u>=====</u> #1	Primary		18.0" Round Culvert L= 213.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 81.00' / 79.90' S= 0.0052 '/' Cc= 0.900
			n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=8.17 cfs @ 12.18 hrs HW=83.23' (Free Discharge) -1=Culvert (Inlet Controls 8.17 cfs @ 4.62 fps)

Summary for Pond DMH5: DMH-5

 Inflow Area =
 2.186 ac, 78.25% Impervious, Inflow Depth =
 7.23" for 100-Yr Storm event

 Inflow =
 8.17 cfs @
 12.18 hrs, Volume=
 1.317 af

 Outflow =
 8.17 cfs @
 12.18 hrs, Volume=
 1.317 af, Atten= 0%, Lag= 0.0 min

 Primary =
 8.17 cfs @
 12.18 hrs, Volume=
 1.317 af, Atten= 0%, Lag= 0.0 min

 Routed to Link SP-2 :
 12.18 hrs, Volume=
 1.317 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 82.03' @ 12.18 hrs Flood Elev= 96.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	79.80'	18.0" Round Culvert L= 55.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 79.80' / 78.00' S= 0.0327 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=8.17 cfs @ 12.18 hrs HW=82.03' (Free Discharge) -1=Culvert (Inlet Controls 8.17 cfs @ 4.62 fps)

Summary for Pond DMH6: DMH-6

Inflow Area	a =	0.235 ac, 3	36.02% Imper	vious, Inflow D	epth = 5.95"	for 100-Yr Storm event
Inflow	=	1.13 cfs @	12.10 hrs, V	/olume=	0.117 af	
Outflow	=	1.13 cfs @	12.10 hrs, V	/olume=	0.117 af, Att	en= 0%, Lag= 0.0 min
Primary	=	1.13 cfs @	12.10 hrs, V	/olume=	0.117 af	-
Routed	to Pond	DMH7 :				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 90.58' @ 12.10 hrs Flood Elev= 95.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	90.00'	15.0" Round Culvert L= 163.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 90.00' / 80.85' S= 0.0561 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=1.12 cfs @ 12.10 hrs HW=90.57' (Free Discharge)

Summary for Pond DMH7:

Inflow Area = 0.458 ac, 67.13% Impervious, Inflow Depth = 6.83" for 100-Yr Storm event Inflow 2.16 cfs @ 12.13 hrs. Volume= 0.261 af = 2.16 cfs @ 12.13 hrs, Volume= Outflow = 0.261 af, Atten= 0%, Lag= 0.0 min 2.16 cfs @ 12.13 hrs. Volume= Primary = 0.261 af Routed to Pond DMH8 : DMH-8 Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 81.56' @ 12.13 hrs Flood Elev= 90.40' Device Routing Invert **Outlet Devices** #1 Primary 80.75' 18.0" Round Culvert L= 49.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 80.75' / 80.50' S= 0.0051 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=2.15 cfs @ 12.13 hrs HW=81.56' (Free Discharge) **1=Culvert** (Barrel Controls 2.15 cfs @ 3.21 fps)

Summary for Pond DMH8: DMH-8

0.458 ac, 67.13% Impervious, Inflow Depth = 6.83" for 100-Yr Storm event Inflow Area = Inflow 2.16 cfs @ 12.13 hrs, Volume= 0.261 af = 2.16 cfs @ 12.13 hrs, Volume= Outflow = 0.261 af, Atten= 0%, Lag= 0.0 min Primarv = 2.16 cfs @ 12.13 hrs, Volume= 0.261 af Routed to Reach 1R :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 81.20' @ 12.13 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	80.40'	18.0" Round Culvert L= 93.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 80.40' / 80.00' S= 0.0043 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

Primary OutFlow Max=2.15 cfs @ 12.13 hrs HW=81.20' (Free Discharge) ←1=Culvert (Barrel Controls 2.15 cfs @ 3.24 fps)

Summary for Pond PP-1: Pervious Pavers

Inflow Area = 0.433 ac, 93.72% Impervious, Inflow Depth = 7.62" for 100-Yr Storm event Inflow = 3.40 cfs @ 12.08 hrs, Volume= 0.275 af 0.66 cfs @ 12.52 hrs, Volume= Outflow = 0.275 af, Atten= 81%, Lag= 25.9 min = 0.66 cfs @ 12.52 hrs, Volume= Primarv 0.275 af Routed to Pond DMH3 : DMH-3

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

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Peak Elev= 93.44' @ 12.52 hrs Surf.Area= 3,978 sf Storage= 4,161 cf Flood Elev= 95.00' Surf.Area= 3,978 sf Storage= 7,630 cf

Plug-Flow detention time= 79.9 min calculated for 0.275 af (100% of inflow) Center-of-Mass det. time= 79.6 min (831.7 - 752.1)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	90.8	33'	7,630 cf	Custom Stage	Data (Prismatio	c)Listed below (Recalc)
_		~				
Elevatio		Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
90.8	33	3,978	0.0	0	0	
91.1	16	3,978	40.0	525	525	
91.5	50	3,978	40.0	541	1,066	
92.0	00	3,978	40.0	796	1,862	
93.0	00	3,978	40.0	1,591	3,453	
94.0	00	3,978	40.0	1,591	5,044	
94.5	50	3,978	30.0	597	5,641	
95.0	00	3,978	100.0	1,989	7,630	
Device	Routing	In	vert Out	et Devices		
#1	Primary	90	.83' 12.0	" Round Culver	rt	
			L= 1	00.0' CPP, proj	ecting, no headv	vall, Ke= 0.900
						S= 0.0183 '/' Cc= 0.900
			n= (0.012, Flow Area	= 0.79 sf	
#2	Device 1	90		,		Limited to weir flow at low heads

Primary OutFlow Max=0.66 cfs @ 12.52 hrs HW=93.44' (Free Discharge)

-1=Culvert (Passes 0.66 cfs of 4.34 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.66 cfs @ 7.53 fps)

Summary for Pond PP-2: Pervious Pavers

Inflow Area = 0.059 ac, 90.30% Impervious, Inflow Depth = 7.62" for 100-Yr Storm event Inflow 0.46 cfs @ 12.08 hrs, Volume= 0.038 af = 0.46 cfs @ 12.09 hrs, Volume= 0.46 cfs @ 12.09 hrs, Volume= Outflow = 0.038 af, Atten= 1%, Lag= 0.6 min Primary = 0.038 af Routed to Pond DMH2 : DMH-2

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.87' @ 12.09 hrs Surf.Area= 1,023 sf Storage= 15 cf Flood Elev= 97.50' Surf.Area= 1,023 sf Storage= 2,116 cf

Plug-Flow detention time= 0.6 min calculated for 0.038 af (100% of inflow) Center-of-Mass det. time= 0.6 min (752.7 - 752.1)

Volume	Invert	Avail.Storage	Storage Description
#1	92.83'	2,116 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

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Elevatio (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
92.8	33	1,023	0.0	0		
93.3	33	1,023	40.0	205	205	
94.0	00	1,023	40.0	274	479	
95.0	00	1,023	40.0	409	888	
95.5	50	1,023	40.0	205	1,093	
96.0	00	1,023	40.0	205	1,297	
97.0	00	1,023	30.0	307	1,604	
97.5	50	1,023	100.0	512	2,116	
Device	Routing	In	vert (Outlet Devices		
#1	Primary	90		12.0" Round Culv		
				_= 100.0' CPP, pro		
						S= 0.0183 '/' Cc= 0.900
	_			n= 0.012, Flow Are		
#2	Device 1	90	.83' 4	1.0" Vert. Orifice/G	Frate C= 0.600	Limited to weir flow at low heads

Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=92.87' (Free Discharge)

1=Culvert (Passes 0.57 cfs of 3.70 cfs potential flow) **2=Orifice/Grate** (Orifice Controls 0.57 cfs @ 6.59 fps)

Summary for Pond PP-3: Pervious Pavers

Inflow Are	a =	0.113 ac,10	0.00% Impervious, In	nflow Depth = 7.86"	for 100-Yr Storm event
Inflow	=	0.89 cfs @	12.08 hrs, Volume=	0.074 af	
Outflow	=	0.51 cfs @	12.20 hrs, Volume=	0.074 af, At	ten= 42%, Lag= 6.7 min
Primary	=	0.51 cfs @	12.20 hrs, Volume=	0.074 af	
Routed	to Pond	d DMH2 : DMH	H-2		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 92.49' @ 12.20 hrs Surf.Area= 3,446 sf Storage= 219 cf Flood Elev= 96.50' Surf.Area= 3,446 sf Storage= 6,437 cf

Plug-Flow detention time= 2.8 min calculated for 0.074 af (100% of inflow) Center-of-Mass det. time= 2.8 min (743.8 - 741.1)

Volume	Invert	Ava	il.Storage	Storage Descri	iption	
#1	92.33'		6,437 cf	Custom Stage	e Data (Prismatio	:) Listed below (Recalc)
Elevation	Surf.		Voids	Inc.Store	Cum.Store	
(feet)	(ទ	sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
92.33	3	,446	0.0	0	0	
92.83		,446	40.0	689	689	
93.00		,446	40.0	234	924	
94.00	3	,446	40.0	1,378	2,302	
94.50	3	,446	40.0	689	2,991	
95.00	3	,446	40.0	689	3,680	
96.00	3	,446	30.0	1,034	4,714	
96.50	3	,446	100.0	1,723	6,437	

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Device	Routing	Invert	Outlet Devices
#1	Primary	90.83'	12.0" Round Culvert
	-		L= 100.0' CPP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 90.83' / 89.00' S= 0.0183 '/' Cc= 0.900
			n= 0.012, Flow Area= 0.79 sf
#2	Device 1	90.83'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.51 cfs @ 12.20 hrs HW=92.49' (Free Discharge)

-1=Culvert (Passes 0.51 cfs of 3.21 cfs potential flow)

1–2=Orifice/Grate (Orifice Controls 0.51 cfs @ 5.88 fps)

Summary for Pond UDSF-1: Filtration Basin

0.436 ac, 42.19% Impervious, Inflow Depth = 6.07" for 100-Yr Storm event Inflow Area = 3.00 cfs @ 12.09 hrs, Volume= 0.221 af Inflow = Outflow = 0.83 cfs @ 12.44 hrs, Volume= 0.221 af, Atten= 72%, Lag= 21.2 min Primary = 0.83 cfs @ 12.44 hrs, Volume= 0.221 af Routed to Reach 1R : Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af Routed to Reach 1R :

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs Peak Elev= 84.37' @ 12.44 hrs Surf.Area= 1,342 sf Storage= 2,348 cf

Plug-Flow detention time= 22.8 min calculated for 0.221 af (100% of inflow) Center-of-Mass det. time= 22.8 min (818.0 - 795.2)

Volume	Invert	Ava	il.Storage	Storage Descrip	tion		
#1	80.33'		3,282 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)	
Elevatio (fee		ırf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
80.3		691	0.0	0	0		
80.6		691	40.0	94	94		
81.0	00	691	40.0	91	185		
81.5	50	691	30.0	104	289		
82.0	00	691	30.0	104	392		
82.5	50	691	30.0	104	496		
83.0	00	835	100.0	382	878		
84.0		1,172	100.0	1,004	1,881		
85.0	00	1,629	100.0	1,401	3,282		
Device	Routing	In	vert Ou	tlet Devices			
#1	Primary	80).33' 12 .	0" Round Culver	t		
	-		L=	25.0' CPP, square	e edge headwall,	Ke= 0.500	
						= 0.0132 '/' Cc= 0.900	
				0.012, Flow Area=			
#2	Device 1					imited to weir flow at low heads	
#3	Device 1	84		0" Horiz. Orifice/C			
	. .		Limited to weir flow at low heads				
#4	Secondary	84		•		dth Broad-Crested Rectangular Weir01.201.401.601.802.00	

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2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.37 2.51 2.70 2.68 2.68 2.67 2.65 2.65 2.65 2.65 2.66 2.66 2.67 2.69 2.72 2.76 2.83

Primary OutFlow Max=0.83 cfs @ 12.44 hrs HW=84.37' (Free Discharge) -**1=Culvert** (Passes 0.83 cfs of 7.12 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.83 cfs @ 9.48 fps) -3=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.33' (Free Discharge) 4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link SP-1:

Inflow Are	a =	2.373 ac, 78.91% Impervious, Inflow Depth = 6.58" for 100-Yr Storm event
Inflow	=	15.64 cfs @ 12.08 hrs, Volume= 1.301 af
Primary	=	15.64 cfs @ 12.08 hrs, Volume= 1.301 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

Summary for Link SP-2:

Inflow Area =	4.667 ac, 52.49% Impervious, Inflow	v Depth = 6.49"	for 100-Yr Storm event
Inflow =	17.10 cfs @ 12.20 hrs, Volume=	2.523 af	
Primary =	17.10 cfs @ 12.20 hrs, Volume=	2.523 af, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-72.00 hrs, dt= 0.03 hrs

ATTACHMENT C - STORMWATER MAINTENANCE MANUAL



RAILROAD SQUARE REDEVELOPMENT YARMOUTH, MAINE STORMWATER MAINTENANCE PLAN

Yarmouth MS-4 Permit (Small Municipal Separate Storm Sewer System) and Post Construction Stormwater Management Ordinance Chapt. 330.

Railroad square is located within the Town of Yarmouth Brick Hollow Watershed which is regulated under the Department of Environmental Protection (DEP) General Permit for MS4's throughout Maine. The MS4 program requires specific site stormwater system inspection, maintenance and reporting for the post-construction building and site. This Stormwater Plan identifies the stormwater devices installed as part of the Railroad Square project and outlines inspection, maintenance, and reporting requirements to be provided to the Town of Yarmouth Engineer or responsible party for administering the Yarmouth MS4 program. Additional requirements may apply as regulated under the Town Chapter 330 Post Construction Stormwater Management Ordinance.

Introduction

The following plan outlines the anticipated inspection and maintenance procedures for the stormwater management devices (BMPs) for the project site. Also, this plan outlines several housekeeping requirements that shall be followed during and after construction. These procedures should be followed in order to ensure the intended function of the designed measures and to prevent unreasonable adverse impacts to the surrounding environment.

Maintenance Responsibilities

During the phasing in of construction activities, the maintenance of all stormwater measures will be the direct responsibility of the Contractor. After acceptance by the Owner, in whole or on a phased basis, the maintenance of all stormwater management facilities, the establishment of any contract services required to implement the program, and the keeping of records and maintenance logbook will be the responsibility of the subdivision homeowner's association, Railroad Square at Yarmouth Village (HOA). The current contact for Railroad Square Associates LLC (Owner) and Railroad Square at Yarmouth Village (HOA) is:

Matthew Teare, <u>Teare.mattd@gmail.com</u> Railroad Square Associates, LLC. 48 Railroad Square Yarmouth, Maine 04096



The procedures outlined in this inspection and maintenance plan are provided as an overview of the anticipated practices to be used on this site. In some instances, additional measures may be required due to unexpected conditions. For additional detail on any of the erosion and sedimentation control measures or stormwater management devices to be utilized on this project, refer to the most recently revised edition of the "Maine Erosion and Sedimentation Control BMP" manual and/or the "Stormwater Management for Maine: Best Management Practices" manual as published by the Maine Department of Environmental Protection (MDEP).

- 1. **Documentation:** A log summarizing the inspections and any corrective action taken must be maintained. The log must include the name(s) and qualifications of the person making the inspections, the date(s) of the inspections, and major observations about the operation and maintenance of controls. Major observations must include BMPs that need maintenance, BMPs that failed to operate as designed or proved inadequate for a particular location, and locations where additional BMPs are needed. For each BMP requiring maintenance, BMP needing replacement, and location needing additional BMPs, note in the log the corrective action taken and when it was taken. The log must be made accessible to the appropriate regulatory agency upon request. A sample "Stormwater Inspection and Maintenance Form" has been included as Attachment 1 of this Inspection, Maintenance, and Housekeeping Plan.
- 2. **Recertification:** A qualified post-construction stormwater inspector hired by the person having control over post-construction BMPs shall provide on or by June 30 of each year a completed and signed certification to the enforcement authority in a form provided by the municipality, certifying that the post-construction BMPs have been inspected and that they are adequately maintained and functioning as intended by the approved post-construction stormwater management plan, or that they require maintenance or repair, describing any required maintenance and any deficiencies found during inspection of the post construction BMPs and if the post-construction BMPs require maintenance or repair of deficiencies in order to function as intended by the approved post-construction stormwater management plan, that person shall provide a record of the required maintenance or deficiency and corrective action (s) taken.

MDEP Recertification:

Additionally, a certification of the following must also be submitted to the Maine Department of Environmental Protection (MDEP) within three months of the expiration of each five-year interval from the date of issuance of MDEP permits.

- A. Identification and repair of erosion problems. All areas of the project site have been inspected for areas of erosion, and appropriate steps have been taken to permanently stabilize these areas.
- B. Inspection and repair of stormwater control system. All aspects of the stormwater control system have been inspected for damage, wear, and malfunction, and appropriate steps have been taken to repair or replace the system, or portions of the system.



- C. The stormwater maintenance plan for the site is being implemented as approved by the Department, and the maintenance log is being maintained.
- D. All proprietary systems have been maintained according to the manufacturer's recommendations. Where required by the Department, the permittee shall execute a 5-year maintenance contract with a qualified professional for the coming 5-year interval. The maintenance contract must include provisions for routine inspections, cleaning and general maintenance.
- E. The Department may waive some or all of these recertification requirements on a case-by-case basis for permittees subject to the Department's Multi-Sector General Permit ("MSGP") and/or Maine Pollutant Discharge Elimination System ("MEPDES") programs where it is demonstrated that these programs are providing stormwater control that is at least as effective as required pursuant to this Chapter.
- 3. **Duration of Maintenance:** Perform maintenance as described and required for any associated permits unless and until the system is formally accepted by a municipality or quasi-municipal district or is placed under the jurisdiction of a legally created association that will be responsible for the maintenance of the system. If a municipality or quasi-municipal district chooses to accept a stormwater management system, or a component of a stormwater system, it must provide a letter to the MDEP stating that it assumes responsibility for the system. The letter must specify the components of the system for which the municipality or district will assume responsibility, and that the municipality or district agrees to maintain those components of the system in compliance with MDEP standards. Upon such assumption of responsibility, and approval by the MDEP, the municipality, quasi-municipal district, or association becomes a co-permittee for this purpose only and must comply with all terms and conditions of the permit.

Post Construction

- 1. **Inspection:** After construction, it is the responsibility of the owner/HOA or assigned heirs to comply with the inspection and maintenance procedures outlined in this section. All measures must be maintained in effective operating condition. A person with knowledge of erosion and stormwater control, including the standards and conditions in all applicable permits, shall conduct the inspections.
- 2. **Specific Inspection and Maintenance Tasks:** The following is a list of permanent erosion control and stormwater management measures and the inspection and maintenance tasks to be performed after construction.

Ditches, Swales, and Riprap Aprons

Open swales and ditches shall be inspected on a monthly basis or after a major rainfall event to assure that debris and/or sediments do not reduce the effectiveness of the system. Debris shall be removed at that time. Any sign of erosion or blockage shall be immediately repaired to assure a vigorous growth of vegetation for the stability of the structure and proper function. Maintenance shall include, but not be limited to, mowing, trimming and removal vegetation in



the ditches as required to prevent vegetation from blocking or diverting storm flows, replacement of riprap channel lining to prevent scour of the channel invert, removing vegetation and debris from the culverts.

Vegetated ditches should be mowed at least three times during the growing season. Larger brush or trees must not be allowed to become established in the channel. Any areas where the vegetation fails will be subject to erosion and should be reseeded and mulched immediately.

Riprap ditches and aprons where stone is displaced should be replaced and chinked to assure stability. With time, additional riprap may be added. Vegetation growing through riprap and accumulated sediments and debris should be removed on a bi-annual basis.

Drainage Pipes, Catch Basins, Drain Manholes and Culverts

Culverts and piped drainage systems including all structures (catch basins, manholes, etc.) shall be inspected on an annual basis to remove any obstructions to flow; remove accumulated sediments and debris at the inlet, at the outlet, and within the conduit; and to repair any erosion damage at the pipe inlet and outlet. Sediment should be removed when its level exceeds 20% of the pipe diameter. This may be accomplished by hydraulic flushing or any mechanical means; however, care should be taken to contain the sediment at the pipe outlet, and not flush the sediments into the detention/infiltration pond areas as this will reduce the ponds capacity and ability to infiltrate runoff, and will hasten the time when the pond must be cleaned/rehabilitated.

Driveways, Walkways and Paved Areas

Accumulations of winter sand along paved surfaces shall be cleared at least once a year, preferably in the spring, to minimize transportation of sediment during rainfall events. Accumulations on pavement may be removed by pavement sweeping.

Underdrained Soil Filter –Lots 4-5

Inspections of the underdrained filter shall be conducted on a semi-annual basis and following significant rainfall events. Delayed or poor maintenance practices can result in loss of treatment capacity. Records should be kept of all maintenance operations to help plan future work and identify problem areas.

The basin embankments should be maintained to preserve their integrity including, but not limited to, vegetation maintenance (mowing, control of woody vegetation), rodent control, erosion control and repair, and outlet control structure maintenance and repair. The embankment should be inspected annually for erosion or destabilization of side slopes, embankment settling and other signs of overtop structural failure.

Basin plantings, and vegetation should be maintained on a quarterly basis. Regular maintenance activities should include cutting back shrub plantings where necessary to prevent excessive woody growth, removal of dead vegetation and re-planting to maintain good cover and root spread. Shrub or grass clippings should be removed to minimize the amount of organic material accumulation in the basin.

Sediment and debris should be removed from the sediment forebay at least annually, where applicable.



Bioretention cells and underdrained filters shall not be used for snow storage area. Snow storage should be sited so that snow melt flows to a pretreatment BMP before reaching the infiltration area.

Vehicular equipment used to maintain or rehabilitate the basins should work from the cell perimeter and not enter the basin floor area, as this would compact the soil surface and reduce infiltration.

The surface of the basins may clog with fine sediments over time. Maintenance of good plant or grass cover should minimize this; however, if ponded runoff does not infiltrate within 48 hours, rototilling the top of the soil bed may be required to reestablish the soils infiltration capacity.

Roof Drip line Filters – Lot Buildings

Inspections of the filtering drip strips shall be undertaken on a quarterly basis and following significant rainfall events. The surface stone should be inspected for evidence of displacement, or erosion. Any accumulated debris, leaves, or loose vegetative matter should be removed from the surface to prevent clogging of the void space. The areas adjacent to the filtering drip strips should be inspected for erosion or bare soil that could migrate into the stone or filter media. Evidence of standing water, or poorly draining media should be noted as this may indicate the need for replacement of the materials.

Pervious Surfaces - Civic Plaza A, Thoroughfare 2, Lot 3,

Pervious surfaces and pavement, whether asphalt, concrete or paving stones, have the potential to become impervious if not properly maintained. The following need to be planned for and be met:

Frequent inspections are performed during the first few months following construction. Then, the system is inspected routinely on an annual basis. Inspections should be made after significant storm events to check for surface ponding that could indicate failure due to clogging. Non-routine maintenance may require reconstruction of the surface treatment, and possibly the filter and reservoir layers, to relieve major clogging.

Prevent sedimentation due to the erosion of areas upgradient the pervious pavement structures.

Prevent vehicles with muddy wheels from accessing onto areas intended for pervious pavement.

All pervious pavers/pavement shall be swept twice a year with a regenerative air vacuum sweeper. Mechanical and traditional vacuum sweepers are not acceptable.

Limit salt use for deicing, and do not use sand.

Remove leaves and organic debris in the fall.

Measures should be taken to ensure that an area designed to be porous does not receive a future overlay of conventional non-porous paving.

Subsurface Sand Filter – Thoroughfare 4

A legal agreement between the owner/HOA and an approved maintenance operator (ACF Environmental or approved equal) should identify the responsible inspector, all inspection and maintenance tasks, and all financial obligations.

Cleaning of the pretreatment device should be performed as identified by the entity holding the maintenance contractual agreement. A routine but specific inspection schedule needs to be identified for



every site based on site variables such as anticipated pollutant load, percent imperviousness, land use (i.e. road, industrial, commercial, residential), etc.

The filter should be draining within 48 hours following a one-inch storm or greater. If the system drains too fast, an orifice may need to be added on the underdrain outlet or may need to be modified if already present.

The pretreatment structure must be cleaned when necessary.

Reporting:

The responsible party shall maintain records/logs of all inspections in an electronic format and provide an annual report to the Town of Yarmouth in accordance with their specific MS4 reporting requirements. Refer to the Town Engineer and the MS4 Stormwater Management Plan located at the following link for specific annual reporting requirements. Sample inspection logs are attached as *Exhibit A*.

https://yarmouth.me.us/vertical/sites/%7B27541806-6670-456D-9204-5443DC558F94%7D/uploads/Yarm_2022_MS4StormwaterPlan2021_03_(1).pdf



Railroad Square Yarmouth Maine

Stormwater Inspection and Maintenance Log

Site Name

Railroad Square Location Yarmouth, ME

Date of Inspection

BMP	Inspection tasks	Completed	Notes	Maintenance Required	Maintenance Complete
Ditches, swales	Inspect for debris and channel blockages				
and open	Check vegetation for overgrowth				
channels	Inspect for evidence of erosion				
Catch Basins	Check sediment level in sumps				
	Inspect grates, frames and structures				
Pipe Inlet	Inspect riprap aprons				
and Outlet	Look for evidence of erosion				
Bioretention	Check plantings/grass cover				
Underdrained Filter					
	Inspect underdrain outlets				
	Evidence of high water level				
	Verify structure is draining				
	Inspect inlet grate and outlet structure				
	Look for evidence of sedimentation				
	Check stability of side slopes				
Subsurface Sand	Check system drain time, should be within 48 hours				
Filter	see maintenace agreement for other system requirements	(0			
Filtering Drip Strips	Filtering Drip Strips Inspect surface for displacement/erosion				
	Check for poor drainage				
	Check for debris/ soil ingress				
Paved areas,	Check for sand and salt accumulation				
Walways	Sand should be removed atleast once a year				
	Check integrity of surfaces and edges				
Culverts	Inspect structural integrity				
	Look for joint displacement				
	Inspect inlet and outlet structures				
	Check for sediment accumulation				
Pervious Surfaces	Limit salt use for deicing, and do not use sand.				
	Remove leaves and organic debris in the fall.				
	All pervious pavers/pavement shall be swept twice a year				
	Sweeping by regenerative air vacum sweeper				