February 2021

Preliminary Engineering Report Water System Performance Improvements 1

City of Truth or Consequences, New Mexico



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Preliminary Engineering Report Water System Performance Improvements 1 City of Truth or Consequences, New Mexico

Project No. 20-600-211-00

I, Mark A. Nasi, certify that I am a Licensed Professional Engineer, NMPE# 13076, and that this report was prepared by me or under my direction.



Mark A. Nasi, PE





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1 PROJECT PLANNING

1.1 Overview

The City of Truth or Consequences (T or C) is in need of replacements to the water distribution system due to deterioration. Approximately 57% of the existing system is over 50-years old and has high system pressures with transient events that have led to extensive line breakages, which make operation and maintenance continuous and costly. Although the City has multiple wells, they are unable to move water between zones when a break or booster station failure occurs, creating a lack of system redundancy. The Preliminary Engineering Report (PER) investigates the viability of fifteen water system alternatives to meet the demands of current and future water needs, within the area defined in Figure 7 by increasing the water supply redundancy, controlling the water pressure problems, and addressing the aging water distribution system.

The City's water system has six supply wells all located at the southwest end of the system. The system also has two booster stations, one designated as the "Cook St." booster station, and the other designated as the "Morgan St." booster station. Water coming out from the south part of the city is stored and boosted from the "Cook St." Facility to the "Morgan St" Facility, a second boost from "Morgan St." Facility to the Upper tanks on Cemetery road is needed to feed the entire water system. Based on the existing water system design, there is a current lack of redundancy of water supply for the northern area; given the case of either the Cook St. booster station or Morgan St. booster station fails. No treatment of the water is done beyond chlorine disinfection, as it is not necessary for these wells. The City's distribution system is in poor condition including water meters and fire hydrants that are need of immediate replacement. The city also operates a small water system at the airport that is not chlorinated, pressure tanks are not functioning, and historical buildings are in need of drastic repair, along with the well head not being properly protected.

This report was prepared in accordance with the requirements of USDA Rural Utilities Service Bulletin 1780-2, "Preliminary Engineering Reports for the Water and Waste Disposal Program" (4/4/13). The report addresses the City of T or C water distribution system.

1.2 Location

The City of Truth or Consequences (T or C), shown in **Figure 1** is in Sierra County in the southwestern part of New Mexico (NM), about 75 miles northwest of Las Cruces. T or C is the center of governmental and commercial activity in Sierra County. T or C is located at Latitude



33°8'1" N and Longitude 107°15'10" W. The City is at an elevation of 4,242 feet above mean sea level. The most populated nearby NM cities include Las Cruces (75 miles to the southeast) and Socorro (75 miles to the northeast).

The Village of Williamsburg neighbors to the Westside of T or C, and the City of T or C's water system serves the Village of Williamsburg. The southern, developed portion of T or C contains the entirety of the existing water system. Most of the northern portion of the T or C system included within the City Limits was recently acquired through annexation in 2003 and 2008. Additionally, the existing Municipal Airport Water System located near Truth or Consequences, was added under jurisdiction of the T or C Water Utility in 2018.

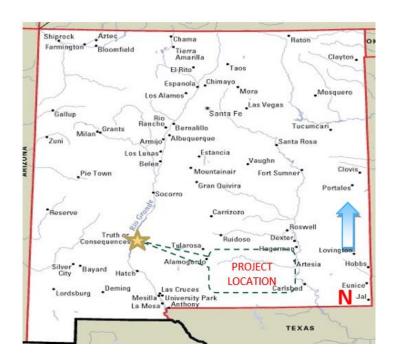


Figure 1: Vicinity Map

2 EXISTING CONDITIONS SUMMARY

2.1 Environmental Resources Present

An Environmental Report has not been prepared for this project. An Environmental Report will be completed at a future date contingent upon the specific funding agency requirements for the final project scope and selected final project alternative. This section of the <u>Preliminary Engineering Report (PER)</u> presents the required assessments of the "environmental resources present" in the study area. Important land resources surrounding and within the City include National Forest



Land, Farmland, Stream crossings, and Cultural Resources. As the water system is already existing, no impact on any of the aforementioned environmental resources is present. Important water resources within the City's existing service area include floodplain associated with the Rio Grande. Below is a brief summary of the environmental resources present.

2.1.1 Farmland

According to the United States Department of Agriculture (USDA) Natural Resources Conservation Service (NRCS) Soil Resource Report (**Appendix 1**), approximately 69% of the soil composition within the City and surrounding area is classified as "Not Prime Farmland" and 26% is classified as "Farmland of Statewide Importance". Although only 26% of soils in the area are considered favorable for farming purposes, the proposed improvements are focused within already developed areas replacing existing infrastructure. Therefore, no impacts to farmland are anticipated by the proposed improvements. See **Exhibit 101** in **Appendix 1** for the USGS land cover map.

2.1.2 Forest Land

The City of Truth or Consequences is located more than 10 miles away from any forest lands, with the Cibola National Forrest being the closest roughly 20 miles north-west of the project. On the opposite side about a mile east, sits the Elephant Butte Reservoir State Park, directly south, adjacent to Williamsburg, is the Caballo Lake State Park. These lands are not directly impacted by the recommended improvements to the project area. Any other monuments or forests are greater than 20 miles away in any direction. See **Exhibit 103** in **Appendix 1** for the US forest service map.

2.1.3 Historic and Cultural Resources

Few historic sites were identified within or near the project area as listed in the New Mexico Historic Preservation Division (NMHPD) as shown on **Exhibit 104.** Direct impacts to historic built environment resources are not anticipated if low vibratory equipment is used near eligible or listed properties. Visual impacts are not anticipated due to the nature of the project being subsurface. Several archeological sites have been identified outside and adjacent to the project area, LA 1119, LA 49016, LA49030, LA50548, LA517, Chambers Canyon Site (LA 49028), Horse Island Site (LA48996), Kettle Top Butte Site (LA48995), Longbottom Canyon Site (LA49033), Monticello Point Archeological District, Palomas Narrows North (LA38755), and Palomas Narrows South (LA49007). These archeological sites are not available in the state database and further research is recommended, which may include a site update. The proposed improvements recommended



by this PER will take place within previously disturbed areas and existing public rights-of-way and will have no effect on these properties.

2.1.4 Range Land

According to USGS, there are public lands in the T or C area used for ranching, grazing, mining, hunting, and fishing. The land use for this project in Truth or Consequences is residential, therefore there will be no negative impacts to any rangeland from recommended improvements to the project area. See **Exhibits** (101-102) in **Appendix 1** for the USGS land cover map.

2.1.5 Wetlands and Flood Plains

According to the Federal Emergency Management Agency's (FEMA) National Flood Hazard Layer (NFHL), significant areas of Truth or Consequences are within Zone A and AE (**Appendix 1- Exhibits 105-106**). Zone A is designated as an area with a 1% annual chance of flooding and a 26% chance of flooding over the life of a 30-year mortgage. This is due to the Rio Grande flowing through the southern and eastern end of the City. Erosion and sediment control plans will be set strictly in place to prevent construction activities from affecting the nearby Rio Grande.

2.1.6 Endangered Species

The ecological findings derived by the Biota Information System of New Mexico (BISON-M), provide a list of possible endangered and threatened species present in Sierra County. This list consists of 25 species, (See **Appendix 1**). According the U.S. Fish and Wildlife IPaC Resource list, the area of disturbance for any proposed construction for the City of Truth or Consequences does not contain any critical habitats. As the water system is already existing in developed areas, no impact is anticipated on any of these species or areas listed. Both lists are included in **Appendix 1**.

2.2 Impact on Surrounding Areas

2.2.1 Air Quality

Construction phase services would have a minor, temporary impact on local air quality. This would be mostly attributable to fugitive particulate matter (PM_{2.5}, PM₁₀) emissions resulting from the following activities:

- Construction and excavation activities such as grading and trenching.
- Dust track-out onto existing paved roads from construction areas.
- Processing and/or stockpiling of materials on-site prior to their use in the construction process.



During permanent operations, no anticipated source of significant air emissions would result. Due to the nature of the project, and since there are no new permanent stationary points of air emissions associated with the planned project activities, adverse air quality is not anticipated for the proposed improvements described herein. Emissions from project construction are anticipated to be minimal and would not jeopardize ambient air quality standards for any of the criteria pollutants. In addition, due to the topography and distance from the project site to the City Proper, the prevailing wind directions and the minimal air emissions anticipated, there are no air impacts anticipated to adjacent property land receptors. Mitigation of construction phase particulate emissions is proposed below.

- Standard management practices for dust abatement is recommended to include water spray and/or moisture addition within proposed grading and/or trenching areas, periodic watering of stockpiles, moisture addition for aggregate processing equipment, and control of vehicle track out of dust and/or mud from non-paved onto paved areas.
- Alternatively, periodic sweeping and/or washing of areas subject to track out can be implemented. In addition, transport trucks carrying import or export soils and/or construction debris materials should be covered with a tarp.

2.2.2 Noise

Noise levels during construction will be intermittent and the result of construction equipment. To mitigate effects of noise levels, construction will take place during normal daytime hours. Once construction is completed, no additional noise levels are anticipated. If required, appropriate sound attenuation will be recommended to mitigate noise levels. Noise levels from proposed alternatives are expected to remain at current levels.

2.3 Population Trends

Table 1 below provides the 1970 thru 2010 US Census data for the City of Truth or Consequences as well as Sierra County. 2016 and 2018 data for both County and City are taken from annual population estimates also provided by the US Census due to census 2020 is in current development. **Figure 2** is a graphic representation of **Table 1** with projected populations from 2011 thru 2018.



Year of Sierra T or C¹ Census County 1 1970 6,221 9,912 1980 5,219 8,454 1990 4,656 7,189 2000 7,289 13,270 2010 6,942 12,030 2016 6,444 11,191

6,278

10,968

Table 1: Population Data

¹ Source: U.S .Census.

2018

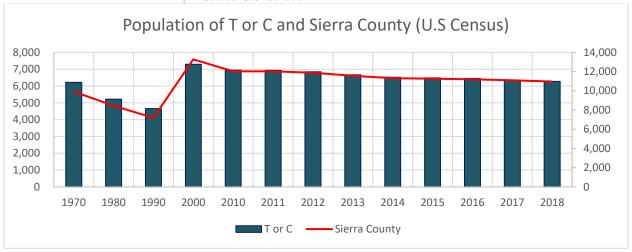


Figure 2 Population Data T or C and Sierra County

According to the US Census, the population of T or C grew from 6,221 people in the year 1970 to 7,289 people in the year 2000. Over this 30-year period, T or C's annual percent growth rate was approximately 0.33%. Growth experienced during this period can be attributed primarily to a large number of retirees that moved to the area. The 2010 Census originally reported the population of T or C as 7,289 but was revised in September 2010 to report the July 1, 2009 population as 6,942. For both T or C and Williamsburg, The decline in the recorded population during the decade can most likely be attributed to several factors: 1) Overall negative economic state of the nation in the second half of the decade, 2) downturn in the local economy caused by drought conditions in recent years and a related decrease in recreational tourist opportunities, and 3) the demographics of the two communities, where almost 30% of the population over 65 years of age.

The potential overall growth of Sierra County and its impact on the Project Planning Area also needs to be considered. As of July 1, 2010, the communities of T or C, Williamsburg, and Elephant Butte, which is located less than a mile northeast of T or C, made up 70% of the Sierra County



population. It is reasonable to assume that the future growth rate of the communities in the Project Planning Area will be similar to the overall projected growth rate of Sierra County. The projected populations and growth rates of Sierra County for the years of 2010 to 2040 as determined by the Bureau of Business and Economic Research (BBER), are contained below in **Table 2.**

Table 2: Projected Population of Sierra County (BBER)

Year	*2010	*2015	2020	2030	2040
Population	11,988	12,020	12,048	12,218	12,737

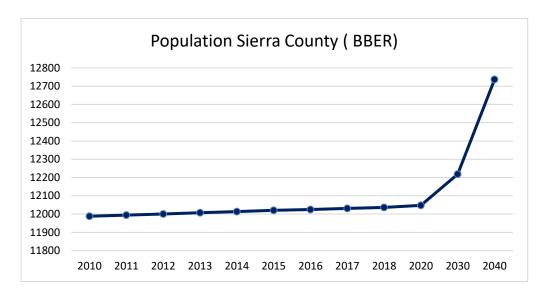


Figure 3: Projected Population of Sierra County (BBER)

Table 3: Projected Population of Sierra County and New Mexico (BBER)

Geographic Area	2005- 2010	2010- 2015	2015- 2020	2020- 2025	2025- 2030	2030- 2040
New Mexico	1.87	1.72	1.5	1.28	1.13	1.04
Sierra County	0.17	0.27	0.23	1.00	0.39	4.07

As **Table 1**, **Table 2**, and **Figure 3** indicate, the more recent projections on population growth from 2010 for the communities in the Project Planning Area are less aggressive than the projections from just a few years earlier. As shown in **Table 3**, the highest projected annual growth rate for Sierra County, over the next 25 years, is 4.07 %, occurring from 2030-2040. T or C itself experienced 1.61% average annual growth in the 1990s, an annual growth rate more than 2.5 times greater than what is currently projected for Sierra County, and is similar to what is currently



projected For the State of New Mexico as a whole. Evidently, there is a wide range of population projections that have been made over the last ten years for Sierra County and the T or C area. Taking into account the available population data, three growth scenarios for the Project Planning Area through the year 2040 have been determined for comparison. Each of the growth scenarios represents growth of communities in the Project Planning Area at an average annual growth rate. The growth scenarios are as follows:

- 1. Linear Growth Scenario: Growth at an average annual rate of 0.033%. This scenario represents the growth in the Project Planning Area that would be expected to occur if future growth follows the pattern of what is projected for Sierra County for the period of 1970-2000 by the US Census.
- **2. Low Growth Scenario** Growth at an average annual rate of 0.23%. This scenario represents the growth in the Project Planning Area that would be expected to occur if future growth follows the pattern of what is projected for Sierra County for the period of <u>2015-2020</u> by BBER.
- **3. High Growth Scenario** Growth at an average annual rate of 1%. This scenario represents the growth in the Project Planning Area that would be expected to occur if future growth follows the pattern of what is projected for Sierra County for the period of <u>2020-2025</u> by BBER.

The population data for these growth scenarios and are shown in **Table 4** and in **Figure 4**. All of the population growth scenarios start with a total population for the Project Planning Area of 6,372 in 2010, which is based on the populations of T or C and Williamsburg reported July 1, 2010 by the US Census (see **Table 1**).

Table 4: Growth Projection Scenarios for Project Planning Area

Year	Linear Growth Scenario (0.033% Annual Growth)	Low Growth Scenario (0.23% Annual Growth)	High Growth Scenario (1.0% Annual Growth)
2010	6,953	7,022	7,289
2015	6,965	7,102	7,636
2020	6,976	7,181	7,983
2025	6,988	7,261	8,330
2030	6,999	7,341	8,678
2035	7,011	7,421	9,025
2040	7,022	7,501	9.372



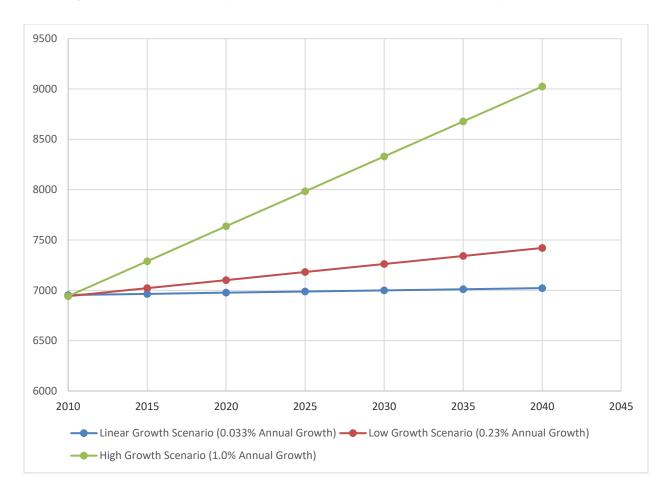


Figure 4: Growth Projection Scenarios for Project Planning Area

Comparison of the growth scenarios shows a very large difference between the High Growth and the Linear Growth scenarios. There is a risk that use of either one of these scenarios to predict future growth could result in significant over-sizing or under-sizing of new infrastructure, which must be avoided. Although recent population data suggests a decline in the future, BBER is projecting positive growth.

According to T or C officials and the current T or C Comprehensive Plan (2004), there are 4 areas where growth is expected in T or C and Williamsburg. The locations of these areas are in general agreement with a map of expected, future land use presented in the T or C Comprehensive Plan. None of the expected development is currently on-going or "on the books" to be done in the near future. All of these growth areas will generate water demand from the existing system. The identified growth areas are shown on **Figure 5** and are as follows:





Figure 5: Identified Growth Area



- a) In the area west of N. Date Street, in the vicinity of I-25, behind the Wal-Mart complex, there are preliminary plans for construction of a new hotel with approximately 115 rooms.
- b) In the vicinity of Turtleback Avenue (east of West Riverside Dr. and south of East Riverside Rd. across the Rio Grande), preliminary plans are in place for a new housing development. Preliminary site utility design/layout has been implemented, but no construction has occurred or is imminent.
- c) Development of a Business/Industrial Park along Broadway in the area south of Cook St.
 Is discussed in the 2004 T or C Comprehensive Plan.
- d) In the area to the south of Williamsburg near I-25 and highway 187 along the Rio Grande, the 2004 T or C Comprehensive Plan discusses future development of this area.

Based on our understanding from T or C governance utilities officials, and the planned growth area discussed above, use of a 1% average annual growth rate seems most appropriate to predict future growth of the communities in the Project Planning Area. Application of the 1% average annual growth rate will result in a practical design for new water system improvements that also incorporate the short-term realities of economic growth. Therefore, for the purposes of planning improvements to the T or C water system, the High Growth Scenario of 1% average annual growth until the year 2040 will be used to project population growth in the Project Planning Area. As shown in **Table 4** and **Figure 4**, the resulting projected population of the Project Planning Area in 2040 is approximately 9,372.

The anticipated equivalent dwelling units anticipated for the next 20 years are as shown in **Table** 5.

Table 5: Equivalent Dwelling Units

Year	Number of Connections	Water Income	Residential Connections	Commercial/ Connections	Daily Demand (GPD)	Residential GPD	EDUs
2020	3,009	\$1,057,195	3,009	485	919,208	212	4,336
2021	3,039	\$1,425,357	3,039	490	928,400	212	4,379
2023	3,100	\$1,512,161	3,100	500	947,024	212	4,467
2025	3,162	\$1,542,555	3,162	510	965,993	212	4,557
2030	3,324	\$1,621,241	3,324	536	1,015,418	212	4,790
2040	3,672	\$1,790,859	3,672	592	1,121,703	212	5,291



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2.4 Community Engagement

The City of *T or C* has an established community involvement process built into the basic workings of the overall City management. City infrastructure issues, including those of the Water System, are routinely discussed in two public forums, the Public Utility Advisory Board (PUAB) Meetings and the City Commission Meetings. The PUAB Meetings occur once a month and the City Commission Meetings occur twice a month. The City will give public notice and hold a public meeting to inform the citizens about the project in accordance with the requirements of RD Instruction 1780.

3 EXISTING FACILITIES

3.1 Water System Overview & History

The distribution pipe network of the existing City's water system has components that date back to at least the 1930s, as evidenced by Work Projects Administration (WPA) emblems associated with waterline infrastructure around the City. The components of the distribution system were put in incrementally from the 1930s through the 2000s as the City expanded. Overall, many of the components that make up the Water System are old (more than 50 years in age), specifically the waterline pipes of the distribution system and several of the groundwater supply wells and their associated pumping systems. The current system is fed by several wells located in the southwest portion of the City. The wells are used to fill the Cook Street storage tank. From the Cooks Street storage tank, and by using its booster station, water is pumped into Morgan Street storage tank. The Morgan Street booster pump station, then pumps the water into the upper tanks located at Cemetery Road to feed the City's water distribution system. (See Figure 6)

In addition to the current pipe network system, in 2018 the City added the Municipal Airport water system, which dates back to at least the 1930's. The components of the system were located within buildings that have historical importance and must be preserved from any damage. The system is currently fed by a well located near the historical buildings. The distribution components at the airport are beyond their useful life and in need for replacement. (See **Exhibit 109** in **Appendix 6**)

The following summarizes the history and condition of the Water System components by category.



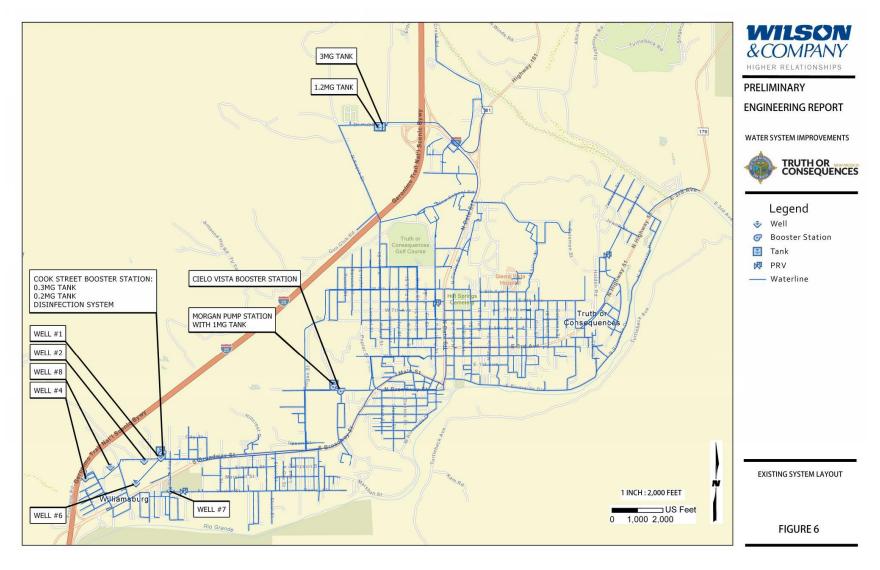


Figure 6: Existing System Layout



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3.2 Condition

The City of T or C's Public Water System ID is NM 3514327. Figure 7 for system layout

The city of T or C's Municipal Airport Water System ID is NM 3501427. Figure 8 for system location

"The Asset Management Plan City of Truth or Consequences" for the existing water system was prepared by Smith Engineering in 2014, revised (2017). Created an inventory of all the water system components with details on capacity, material, age, etc. and assessed the condition of the components based on age (remaining useful life), field investigations, and operator interviews, and water usage.

The Municipal Airport water system was recently activated as a public water system, its inventory of all water system components with details on material, capacity and condition were assessed as part of the PER and per sanitary survey report (**See Appendix 3**) by field investigations, operator interviews, and site visits. The following report section summarizes the condition, capacity/adequacy, and prioritizes replacement of the water system components, organized by component category.



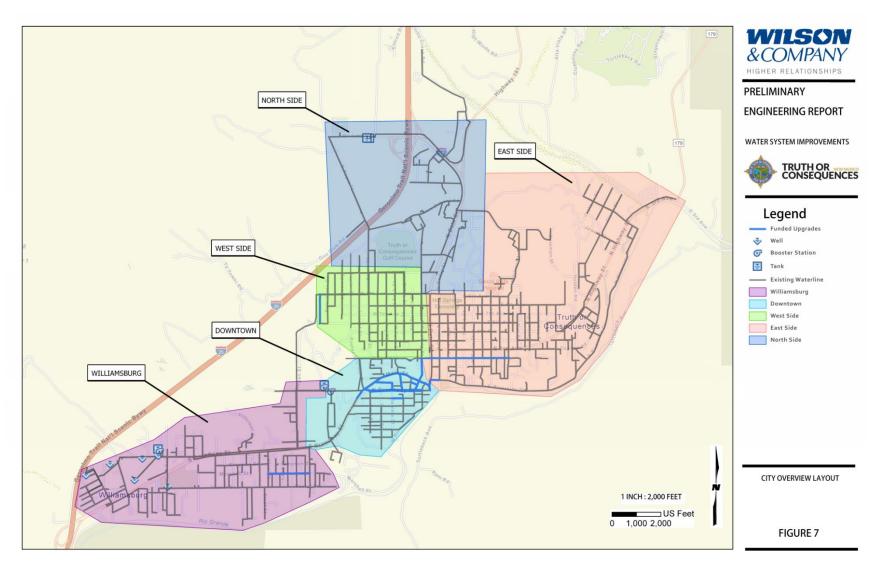


Figure 7: City Overview Layout



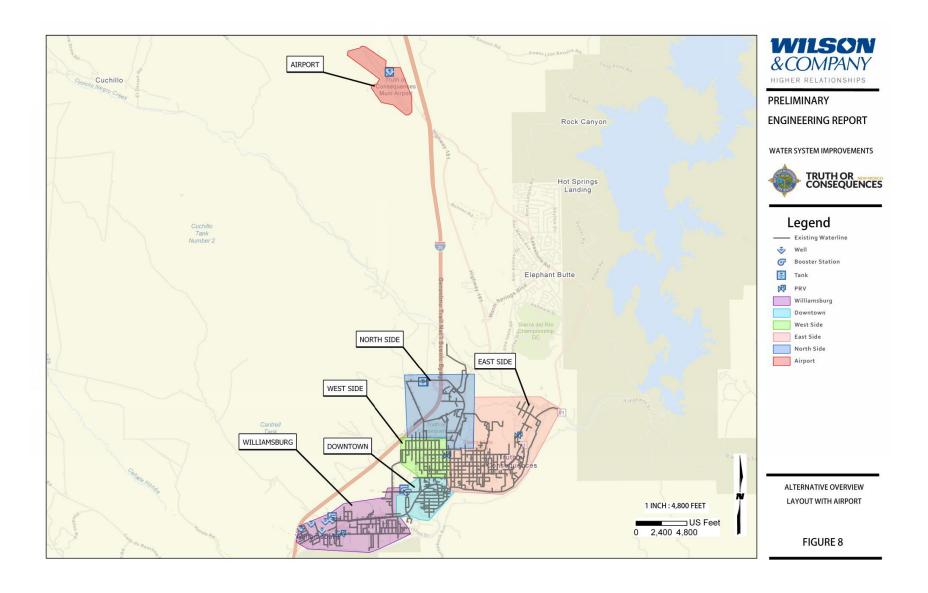


Figure 8: City Overview Layout W/ Airport



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3.2.1 Distribution:

The existing water distribution type consists of waterline pipes, valves, Pressure Reducing Valves (PRV's), hydrants, Air Release Valve (ARV), and meters. Most of the overall water system components within the distribution system vary greatly in condition, primarily as a function of age and material. Many of the distribution system waterlines are considered to be beyond their expected useful life due to age and are considered to be in poor condition. As a result, these old waterlines are leaking significant amounts of water, increasing the overall operation and maintenance cost as well as decrease the overall energy efficiency of the system due to the need for more pumping.

The Water System GIS Inventory database contains records related to the distribution system. Currently, the distribution system provides water to a service area greater than five square miles including 3,538 water meters thorough the City. The City's Asset Management Plan shows that 239,046 linear feet (45.3 miles) of pipeline (approximately 57% of the system) consist of Asbestos Cement (AC), Cast Iron (CI) and Ductile Iron (DI) pipe.

The oldest pipe in the system is asbestos cement (AC) installed primary in the 1930s and again in the 1960s, representing approximately 28% of the distribution system. About 9% of the system is cast iron pipe (CI) that was installed primary in the 1940s and again in the 1970's which causes discoloration of the water system in certain areas of the City. Discoloration is not aesthetically pleasing to customers. 60% of waterline pipe is made of PVC, either schedule 40 or C900, installed throughout the 1970s to the 2000s. The schedule 40, which is a thin wall pipe is susceptible to breakage under pressure surges that are prevalent in the water system. Approximately 47% of the waterlines in the City are older than 50 years old, 59 % of the system is older than 40 years old and almost 77% of the City's waterline system is older than 30 years old. **Figure 9** shows the existing pipe material thorough the City.



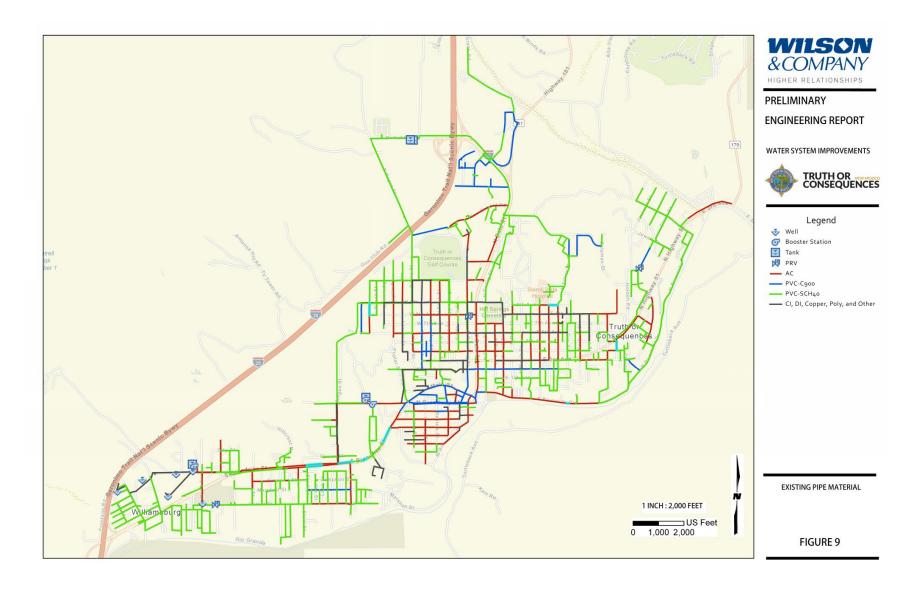


Figure 9: Existing Pipe Material



In 1930 the Initial waterline design for the City of Truth or Consequences satisfied fire flow requirements. Over the course of time, the expansion of the system and the addition of water service connections, coupled with changes in industry fire flow requirements, these requirements are no longer met. Presently, over 33.5% of the current system is under 6 Inch diameter trunk lines that cannot provide adequate pressure under peak demands and fire flow within some of the areas of the City.

The current pumping system arrangement does not have a dedicated transmission line, but instead utilizes the existing aging distribution pipelines to pump water from the Cook Street water storage facility to the Morgan Street water storage facility. The pressure fluctuation is approximately a 30-psi increase in the distribution system when the Cook Street booster pumps are running. The peak pressure surges are causing line breakages in the "Williamsburg" area since it is fed from this zone.

Current reports have shown that the implementation of the tank located in Cemetery Road has increase the water pressure of the system on the "West" area of the City, resulting in waterline breakages and blowouts in these areas. Water breaks are mostly seen when the storage tank is at full capacity. As a precaution, and to minimize waterline repairs, the City decided to only use the water storage tank at half capacity.

Operator interviews, City records, and show existing reports for the year of 2019 indicate 260 water breakages on the City's water system, due to high pressure surges in mostly the "West" and "Williamsburg" areas. Breakages with an average cost of \$1,000 dollars per break including the cost of manpower, materials, fuel, other city resources, road repairs, equipment and water/water loss. The total repair cost for the breaks in 2019 are broken down by; 1) manpower, materials, and fuel is 53% (\$137,800.00) from the water utility budget. 2) other city resources and road repairs is 29% (\$75,400.00) from other City budgets. 3) equipment and water/water loss is 18% (\$46,800.00) which are inherent costs for perspective, such as and equipment hourly rate and the cost of water/water losses because of the break. These equipment and water/water loss cost do not come out of any of the city's budget but are inherent cost of a waterline break. Thus, the actual cost to the City's water operation and maintenance expenses has an approximate value of \$137,800.00 in 2019, which came from salaries/benefits, repair/maintenance, and supplies.

Water breakages in the City have been a noticeable issue due to added monetary expenses as well as water leakages within the piping system. In addition, a water Mass Balance calculation of



years 2016-2019 was completed to estimate the System's approximate water loss percent to estimate condition of the existing water system.

Total Water Total Water Difference Water loss Year Production Consumption (Gallons) Percent (Gallons) (Gallons) 2016 316,158,000 105,123,718 25% 421,281,718 2017 425,646,000 316,963,000 108,683,000 26% 2018 443,881,000 302,863,000 141,018,000 32% 2019 454,209,000 326,675,000 127,534,000 28%

Table 6: System's Approximate Water Loss Percent

As is shown in the above **Table 6**, the water system in 2016 had a loss of 25% and in 2019 that loss percentage increased by almost 3%. The current year of 2019 report had 260 water breakages that can foreseeably increase the water leakage in the years to come.

3.2.2 Fire Hydrant:

The existing fire hydrant system was installed between 1960's and 2000's. A 2013 report IMS by Hurco Technologies Inc. for the City of Truth or consequences fire department presents some flaws concerning residual pressures, age, and fire flow requirements. The system is currently represented by 7.8% of the hydrants are older than 50 years, 20% of the hydrants are older than 40 years, and 35 % of the hydrants are older than 30 years. A big portion of the system is reaching its useful life.

Additionally, Emergency Services Administrator/Sierra county Fire Marshall and the National Fire Protection Association (NFPA) codes, the municipal fire flow should be at least 1,500-gpm with a residual adjacent pressure not less than 20-psi. A report provided by the City's Fire Marshall Indicate 5.36% of the fire hydrants are running with a fire flow below 1500 GPM, and 46% of the hydrants have fire flow below 20 psi. The deficiency of pressure on the fire hydrants also can be affected because 33.5% of the waterline in the City is under 6 Inch diameter. Other minimal diameter trunk lines throughout the City cannot meet the requirement. For further hydrant breakdown report see **Appendix 9**, Additional Fire Hydrant testing report available upon request.

3.2.3 Water Meter:

Truth or Consequences distribution system currently provides water service to 3,538 active water meters. Current field investigations, and operator interviews indicate that meters are



approximately older than 30-40 years old exceeding their useful life. Meter changes are randomized and replaced on an as needed basis. Furthermore, water meter are currently read manually which increases the operation and maintenance cost for the City.

3.2.4 Wells:

The existing water supply for the T or C water system is entirely from six wells located in the southwest portion the system. At present, all the groundwater supply wells are producing consistently except for Well No. 4, and Well No. 8 which are currently off-line. The combined production capacity of the wells is adequate to meet the anticipated current demands and the wells are suitable for continued use.

DEPTH WELL **DEPTH OF THE** WATER **WELL** OF **PUMP RATE** WATER WATCH ID **ELEVATIONS** DIAMETER WATER INSIDE NO. WELL (GPM) (IN.) (FT.) (FT.) 1 HS 001151 4248 475 20 400 38 VHS 00011 or HS 2 4248 350 12.75 405 43 00011-S HS-0011-S-5/HS-4* 4269 200 10 355 0 00019 6 HS 01059 4244 575 12.75 414 52 7 725 HS 01059 4237 12 to 18 431 69 8* HS 00011-S-9 4264 725 12 to 18 367 5

Table 7: Well Information

Well No. 2 and Well No. 4 are the oldest wells in the system, installed in 1945 and 1958, respectively. Well No. 6 is the next oldest well, installed in 1976. The other three wells were all installed in the 1990's and are relatively new. Well No. 4 and Well No. 8 are out of service. City's water maintenance group stated that Well No. 4 had lower pumping rates than expected and its running cost was exceeding the budget. Additionally, Well No. 8 casing slipped causing the well to be inoperable Well No. 8 it is planned to be evaluated and repaired in 2020, if possible.

The Initial design for the water system of the City of T or C met the terms with the water supply based on the City's location and low expansion. But during the course of time, the expansion and escalation in water service connections in different areas of the City has changed the initial requirement. The system currently has a total of six wells, all of them located on the southwest portion of the City. The location of these wells determines the pumping system design to transfer water to three different tanks from the southern part (Cook Street), to the middle part (Morgan



^{*} wells currently offline

^{*}Drinking Water watch website #NM3514327

Street), and to the north part (Cemetery Road). The water transfer between the three zones does not allow for water supply redundancy and increases the failure potential for long water outages in the future.

3.2.5 Pump System:

Pump systems include the following: pumps, motors, manifolds/valves, flow meters, and all associated electrical components. There are essentially two distinct pump systems, separate from the well submersible pumps, currently used to transfer water from the supply wells to the storage tanks and distribution throughout the water system. The two distinct pump systems are the following:

- 1. Cook Street Treatment Facility Pump Station
- 2. Morgan Booster Station

The Cook Street Facility was constructed in 1996 and has two 250-Hp centrifugal pumps. To address water-hammer issues, a new soft-start system and a new electrical system for the two original pumps was installed in early 2014.

The Morgan Booster Station was constructed in 2007. The transfer switch for the electrical system of Morgan Booster Station was replaced in 2013. Replacement of various gauges, valves, and flow meters, as well as motor repairs/modifications, have occurred for multiple pumps systems throughout the years the water system has been operating. Other than what has been previously mentioned, no major pump system replacements have occurred. Morgan Street pumps have failed but the system will be upgraded in 2021.

The individual pump systems of the overall water system all have adequate capacity to provide the expected level of service and were designed in a manner which allows them to be suitable for continued use in the future. In general, the pump systems operate in an energy-efficient manner with the exception of the large booster pumps at the Cook Street Treatment Facility Pump Station. The current operations at the Cook Street Pump Station result in frequent start/stop cycles of the large 250 HP pumps due net flow of the facility and the small volume of on-site storage.

The City currently has a project that includes improvements to the Cook Street Facility and waterlines in the Downtown area. The Cook Street improvements will double the on-site storage and equip the existing 250HP booster pumps with new VFD controllers to run the pumps between 1,000 GPM and 3,000 GPM (full capacity). This will extend the run times of the pumps and reduce



the head loss in the water system when the pumps are running at a lower capacity, instead of when the pumps are running 3,000 GPM (full capacity), with a total dynamic head of 192.3 TDH (a pressure differential of 30 PSI). The total dynamic head when the pumps are running at 1,000 GPM is estimated to be 134.6 TDH (a differential pressure of 5 psi). Improvements for the existing system also include the replacement of Cla-Val Valves with micro switches, Hydraulic Check Valves, concrete repair, new electric heater, and a new Digital Mag Flow meter on the booster station outlet.

3.2.6 Well Pumps:

Corresponding to the oldest well, the oldest pump system is the one for Well No.1 and No. 2, originally installed in 1945. The existing pumps system for Well No.1 and No. 2 is not thought to be the original system from the 1940's, but rather a newer system installed in the 1980's and 1960's respectively. Due to age, the original pump system for Well No. 4 was replaced in 2001 with the current pump system. Similarly, the pump system for Well No. 6 was replaced in 1999 with the current pump system. The existing pump systems for Wells No. 7 and No. 8 were installed new as part of the same project in 1999.

The individual pump systems of the overall water system all have adequate capacity to provide the expected level of service and were designed in a manner which allows them to be suitable for continued use in the future.

3.2.7 Water system SCADA:

Operation of the different pump systems and storage tanks that make up the existing water system is coordinated and controlled using a Supervisory Control and Data Acquisition (SCADA) system with the master control computer located at the T or C WWTP. The hardware and software of the system was initially installed in 1997. Since installation, the SCADA software and computer have been upgraded several times. In contrast, all the existing communication remote terminal units (RTUs), located throughout the Water System at the various pump system and storage tanks sites, are the original units from 1997. The City currently has a project to upgrade the SCADA system in 2022. The plan is to upgrade and incorporate in-to the new WWTP SCADA/HMI system to be fully compatible with the same equipment used for the WWTP system. RTU's will be replaced/upgraded at all locations.



3.2.8 Disinfection System:

The existing disinfection system is a gas-chlorination system, located at the Cook Street Facility. The chlorination system, including all associated piping, equipment, and fiberglass housing (shed) was installed in 1996, at the same time the rest of the facility was constructed. This system is old and beyond its expected useful life. The City is currently replacing the gas chlorination system in its entirety with the Cook Street Facility Improvements project.

3.2.9 Buildings:

The existing T or C water system includes various buildings, primarily used to house pump systems. The water system buildings are as follows:

- 1. Cook Street Treatment Facility Pump Station Building
- 2. Cook Street Treatment Facility Storage Building
- 3. Morgan Street Booster Station

Another existing old building is the one that used to be the Pershing Pump House, estimated to have been constructed in 1945. This building no longer contains pumps but is used to house a critical pressure-reducing valve that separates the upper pressure zone of the water system from the lower pressure zone.

The existing Cook St. Treatment Facility Pump Station Building was constructed in 1996. The existing Storage Building located at the Cook St. Facility site predates the facility and is much older, estimated to have been constructed in the mid-1970s.

The Well pump houses for Wells No. 6, No. 7, and No. 8 were all constructed in 1999 as part of the same project as the pump upgrades. As a result, these building are some of the newest existing buildings of the water system.

The Morgan Booster Pump is the newest building in the water system. This building was constructed in 2007 as part of the overall Morgan Booster Station project in which the pump system, including electrical components and the back-up generator, were installed.

3.2.10 Storage Tank

The existing water system includes the following four operational storage tanks:

- 1. 0.2 MG Storage Tank, located at Cook Street Treatment Facility
- 2. 1.2 MG Storage Tank, located on Cemetery Road
- 3. 3.0 MG Storage Tank, located on Cemetery Road



4. 3.0 MG Storage Tank, located next to Morgan Booster Station

The oldest operational storage tank is the 3.0 MG tank located on Cemetery Road, which was originally constructed in 1978 to provide storage and pressure for the upper distribution zone of the water system. Due to age and corrosion, the tank underwent major rehabilitation and repair including sandblasting, re-priming/repainting, new hatches, access ladders, and cathodic protection in 2013.

The 0.2 MG storage tank located at Cook Street Treatment Facility was constructed at the same time as the rest of the facility, in 1996, and is used to store and transfer chlorinated water to the distribution system and the Morgan storage tank. Due to age, the tank underwent minor repairs in 2012, including new hatches, access ladders and cathodic protection.

The newest operational storage tanks are the 1.2 MG tank located on Cemetery Road and the 3.0 MG tank located next at the Morgan Booster Pump Station, both of which were constructed in 2004. The 1.2 MG tank was added to the water system to provide additional storage and a back-up tank for the upper distribution zone and sits next to older 3.0 MG tank. The newest 3.0 MG tank at the Morgan Street site was added to the water system to provide storage and pressure for the lower distribution zone.

All of the storage tanks have been rehabilitated or installed as new within the last ten years and are in excellent condition. As a result, none of the storage tanks have been scheduled for significant repair or rehabilitation in the next 10 years.

All the storage tanks have been designed with the proper capacity to be suitable for many more years of use, except for the 0.200 MG storage transfer tank at the Cook Street Treatment Facility. The capacity of the tank appears to be undersized resulting in excessive start/stop cycles of the booster pumps that operate with the tank. Improvements to the Cook Street Facility equipment and operations are presently under design and scheduled to be constructed in 2022. In addition to the pump improvements previously discussed, a new additional ground 300,000-gallon steel water storage tank is planned at the Cook Street Booster Station site on year 2022. This tank is to increase the storage capacity at the site to reduce the pump on/off cycles of the booster pumps and increase the run time of the booster pumps. And will extend the life of the existing booster pumps



3.2.11 Municipal Airport Water System:

The existing Municipal Airport Water System was recently added under jurisdiction of the T or C Water Utility and must comply with all the relevant rules and regulations for the public water system. The Municipal Airport water system has been classified as a Non-Community water system with a transient population of 40. Current field investigations, and operator interviews indicates the system is located in an estimate of three Historical buildings. Inside of these historical buildings there are different parts of the existing water system including water pump, vault, control room, pressure gage, meter valve, electrical system, and pressure storage tank. The system is currently presenting a pressure issues, electrical outages, as well as pressure tank bladder issues. (See **Exhibit 109** in **Appendix 6**)

The system is currently in poor conditions and it needs a replacement and the installation of a storage tank to prevent water pumps burnout. The City expressed the importance of preserving these buildings because of its historical status.

3.3 Financial Status of Existing Facilities

3.3.1 Current Energy Consumption

The current energy consumption for the T or C water system is shown the City's FY 19/20 Budget under the Utilities Line item. It is our understanding that electrical power is the only item under utilities. While specific consumption is not known, the total water system consumption can be back calculated using an average rate of \$0.07/kWh. The estimated energy consumption is as follows in **Table 8**:

Table 8: Estimated Energy Consumption

FY	Budget	Rate(\$/kWh)	Est. QTY(kWh)
13/14	\$138,833.00	\$0.07	198,329
14/15	\$124,941.00	\$0.07	1,784,871
15/16	\$107,944.00	\$0.07	1,542,057
16/17	\$98,141.00	\$0.07	1,402,014
17/18	\$91,277.00	\$0.07	1,303,243
18/19	\$131,825.00	\$0.07	1,883,243
19/20	\$95,000.00	\$0.07	1,357,143
AVG	\$112,565.86		1,352,986



3.3.2 Existing Asset Management Plan

The Asset management Plan for the existing water system of the City of Truth or Consequences was prepared by Smith Engineering in 2014, revised (2017). (See **Appendix 11**)

3.3.3 Revenue

The current source of revenue is from utility billings provided by the City of T or C's residents for water system services. The utility rate structure is broken out below in **Table 9** as provided by the City's database.

3.3.4 Operations and Maintenance Cost

Table 9 Shows the operating revenue and operating expenses of the T or C Water System for FY 2011/12 through FY 2019/20. As of December 2019, the water system generates revenue from a total of 43 governmental connections, 485 commercial connections, 1 industrial connection, and 3009 residential connections. As indicated in **Table 9**, from FY 2017/18 to FY 2018/19, the annual water system revenue was very consistent as were the annual operating expenses. In general, the T or C Water System generates adequate annual revenue to cover what is included in its current operating expenses.



Table 9: T or C Water System Five-Year Financial Data

	Item	2015-16	2016-17	2017-18	2018-19	2019-20	projection 2020-2021*
Operating	Revenue from Water Utility Service connections	\$72,952	\$1,006,193	\$955,250	\$945,330	\$1,057,195	\$1,404,617
Revenue	Revenue from Other Water System services	\$0	\$298	\$0	\$3,706	\$0	\$20,740
	TOTAL REVENUE	\$72,952	\$1,006,491	\$955,250	\$949,036	\$1,057,195	\$1,425,357
	Item	2015-16	2016-17	2017-18	2018-19	2019-20	projection 2020-2021
	Salaries/Benefits	\$242,237	\$207,723	\$281,860	\$272,138	\$277,130	\$282,130
	Supplies	\$14,746	\$13,189	\$18,126	\$35,933	\$24,700	\$24,700
	Office	\$76	\$2,114	\$1,812	\$1,529	\$3,500	\$3,500
	Testing	\$0	\$0	\$89	\$3,743	\$2,000	\$2,000
	Repair/Maintenance	\$104,081	\$92,701	\$59,214	\$44,430	\$91,000	\$91,000
	Tax	\$44,902	\$42,302	\$42,905	\$42,587	\$44,000	\$44,000
	Utilities	\$109,449	\$99,672	\$92,736	\$133,268	\$96,000	\$96,000
Operating	Professional Fees	\$4,215	\$12,950	\$53,278	\$30,434	\$22,350	\$22,350
Expenses	Equipment (Incl. Rental)	\$3,426	\$3,313	\$3,593	\$3,880	\$3,000	\$3,000
	Accounting	\$37,691	\$49,307	\$51,792	\$50,765	\$58,958	\$58,958
	Employee Training	\$0	\$0	\$0	\$4,564	\$4,000	\$4,000
	Non-Capital Equipment	\$12,294	\$12,676	\$12,880	\$13,632	\$10,000	\$4,000
	water Conservation	\$715	\$1,461	\$1,385	\$550	\$4,000	\$10,000
	Miscellaneous	\$0	\$0	\$2,098	\$0	\$0	\$0
	Capital outlay-Machinery & Equipment	\$28,500	\$0	\$79,000	\$48,938	\$103,000	\$85,820
	TOTAL OPERATING EXPENSES	\$602,332	\$537,408	\$700,768	\$686,391	\$743,638	\$731,458

Reconciled amount from T or C utilities department

3.3.5 Planned Capital Improvements

Other capital improvements the City of T or C plans to undergo include enhancements to the entire downtown area as described in the City of Truth or Consequences Downtown Master Plan. As the economy of T or C is driven primarily by tourism, recreation, and the business' which support the retirement communities, the upgrades set out by the master plan will create a more inviting and attractive destination for visitors as well as residents. Ensuring that the water system is upgraded prior to additional downtown improvements is vital.

3.3.6 Existing Debt

At present, the water system also has outstanding debt in the form of nine loans, which are shown below in **Table 10.** The total of the nine annual loan repayment amounts is approximately \$520,281.



^{*}Projection 2020-2021 accounts new water rates effective as in July 1 2020

Table 10: T or C Existing Debt

Owed To	Purpose	Balance Owed	Maturity Date	Annual Payment	Annual Reserve	Outstanding Balance Sept 2020	Interest Rate
NMFA (TorC2)	Water Tank Loan	\$1,841,089	2021	\$120,260	N/A	\$224,495	2.00%
NMFA TorC17/WTB- 229)	Ground Storage	\$256,000	2032	\$13,138	N/A	\$13,138	2.50%
NMFA (TorC18)	Ground Storage	\$165,741	2032	\$8,287	\$8,200	\$99,445	0.00%
NMFA (TorC19)	Water Rev Bonds (95,96,98)	\$1,424,865	2033	\$91,185	\$91,500	\$949,380	2.90%
NMFA (TorC22/WTB- 292)	Water PER/AMP	\$64,000	2033	\$3,380	N/A	\$43,185	0.00%
NMED Loan 95-16	Improvements to system	\$504,483	2022	\$33,909	N/A	\$64,884	3.00%
NMFA DW- 4794	High Risk Waterline Replacement	\$620,543	2041	\$31,866	N/A	\$620,543	2.50%
CIF-4927	Water System PER	\$9,000	2041	\$450	N/A	\$9,000	0.00%
NMFA (WPF- 5089)	Booster System Improvements Loan	\$264,155	2042	\$13,208	N/A	\$264,155	2.50%
USDA *	MSD Project	\$5,487,000	2059	\$204,598	\$204,598	\$5,487,000	2.13%
	TOTAL	\$10,636,876		\$520,281	\$304,298		

^{*}Loan to be closed in June 2021

3.3.7 Water Connections

The below **Table 11** provides a tabulation of water connections for the fiscal year 2019/2020 as provided by the City of T or C's Billed Consumption reports.

Table 11: Number of Water Connections (FY 2018/2019)

T or C Water Connections					
Туре	Connections				
Residenti	2769				
Commerc	467				
City		43			
Industria	Industrial				
Williamsburg Water Connection					
Туре	Connections				
Residential	240				
Commercial		18			
	TOTAL	3538			



3.3.8 Water/Energy/Waste Audits

Per the request from the Office of the State Engineer a water audit was necessary for the Water Conservation Plan Verification. An audit was performed on the water system providing 33-percent of losses in the existing water system for the years 2015 and 2016. In order to compare performance of previous years a water system a Mass Balance calculation of 2016-2018 was completed estimating an average of 38-percent of losses. Percent losses have increased during the past three years approximately 5-percent, this five percent could be a result of the poor conditions of the system.

4 NEED FOR PROJECT

4.1 Water System - Health, Sanitation, and Security

The need for the project is due to aging infrastructure inadequate pipe sizes to provide consistent water pressure and adequate fire flow throw-out the City along with addressing reasonable growth discussed within the following sections

4.1.1 Health, Sanitation, Security issues

The health and safety of the citizens of T or C is of great importance when considering future community growth and development. At present, the City Water System is not in compliance with the water quality regulations of the NMED Drinking Water Bureau and has nine compliance issues in recent years of 2018/2019 (See **Appendix 10**). All the critical components of the system responsible for the delivery of good quality, and properly disinfected water to consumers are currently in poor working condition but is being update in 2020 and have appropriate security (fences, lights, etc.).

As another means to protect the health and safety of the public, efforts should be made to ensure proper fire flow capacity in areas of the City that are planned to have new development redevelopment, or scheduled waterline replacement. Since the City is planning to redevelop critical areas in the very near future, the associated planned improvements to waterlines in these areas should be done in a manner to provide proper fire flow to protect the public.

The area on the west side of the City is fed by the Cemetery Road Tank that is prone to water line breaks when the tank is full. This situation produces a health and safety issues, as there is not a continuous supply capacity for on this specific area. Break on the pipes affects not only households but also critical facilities such as high school and hospitals.



During the year 2019, existing reports indicated 260 waterline breakages on the city's water system, most of which are located on the "East," "west," "Williamsburg," and "Downtown" areas of the city. These continuous breakages represent a health and sanitation issue within the city's residents due to water outages that usually range from 4-6 hours while the city crew makes the repairs. An average of 20 -30 residents are affected per each water line repair.

4.1.2 Aging Infrastructure

Aging water infrastructure is the main justification for this project. The condition of the aging infrastructure is corroborated by information provided to Wilson & Company from system operators, and the condition assessment as included in the Water System GIS Inventory database that contains records related to the distribution system. The City intends to redevelop a significant portion of the main transmission lines to provide accurate water distribution flow and prevent high pressures around the city. The development of the "System Performance Upgrade "area includes many planned improvements to infrastructure. Primary elements of infrastructure to be improved are waterlines, valves, fire hydrants, PRV's, and improved redundancy of water supply to critical storage tanks.

Water system mass balance calculations from years 2016, 2017, 2018 and 2019 show loses of 25%, 26%, 32% and 28% respectively in the system. This can be attributed to water line breaks throughout the system. The high number of breaks results in emergency repairs and high maintenance costs. The increase consequence of failure also adds to the high cost.

Additionally, the existing reports for the year of 2019 shows the system had 260 water breakages attributed to high pressures in the system and mostly occurred in the "Williamsburg" area. Breakages with an average cost of \$1,000 dollars per break including the cost of manpower, materials, fuel, other city resources, road repairs, equipment, and water/water loss. The total repair cost for the breaks in 2019 are broken down by; 1) manpower, materials, and fuel is 53% (\$137,800.00) from the water utility budget. 2) other city resources and road repairs is 29% (\$75,400.00) from other City budgets. 3) equipment and water/water loss is 18% (\$46,800.00) which are inherent costs for perspective, such as and equipment hourly rate and the cost of water/water losses because of the break. These equipment and water/water loss cost do not come out of any of the city's budget but are inherent cost of a waterline break. Thus, the actual cost to the City's water operation and maintenance expenses has an approximate value of \$137,800.00 in 2019, which came from salaries/benefits, repair/maintenance, and supplies.



The Municipal Airport Water System operated by the City does not have a chlorination system, the pressure tanks are not functioning, existing well head is not properly protected, and the historical buildings are in need improvements due its aging and poor condition.

4.1.3 Reasonable Growth

The water system needs to have adequate water supply to meet consumer demand and the ability to distribute safe water to all the end-users of the system. As a result, appropriate improvements to the overall system need to be made to ensure efficient system performance.

Table 12: 2009-2019 Annual Water System GPD/Connection

Year	System GPCD
2009	NA
2010	142
2011	170
2012	195
2013	205
2014	203
2015	189
2017	200
2019	212
Average	190

Anticipated future water demand on the overall water system can be estimated by combining historical water usage data, and population projections. As discussed in this report, the future population of the project planning area in 2040 is estimated to be 9,372. Evaluation of recent historical water system production records show a water demand of Gallons per day per connection (GPD) of 212. **Table 12**. A water audit completed in 2014 shows the monthly peaking factor to be 1.71.





Figure 10: 2019 Monthly GPD/Connection

5 ALTERNATIVES CONSIDERED

5.1 Water System Description

Eleven alternatives are considered in this PER to address the aging infrastructure, material type, pressure problems, and water supply redundancy in the City's water system. There are four additional alternatives considered for the Municipal Airport Water system.

5.1.1 Design Criteria

To determine the best approach for water line rehabilitation, three methods were evaluated in this report. Although there are several methods of water line rehabilitation, several were determined to be infeasible and are not considered. The following methods were eliminated prior to full evaluation:

- Pipe Bursting: Pipe bursting option allows water main installation without roadway disruption, however the service interruption caused would be unacceptable and customers would be without water for long periods while the Contractor is, pipe bursting, resorting water service to the main and re-connecting service lines. Pipe Bursting Is more cost effective on transmission lines that run point to point with minimal connections but becomes expensive and evasive when considered on distribution lines. Pipe bursting works best when the existing line can be taken out of service with minimal impact.
- Horizontal Directional Drilling: Allows for the water main to be installed with minimal disruption to roadways, traffic, and other infrastructure. However, on lines with a large number of services it requires excavation to reconnect services and lateral line connections. Horizontal directional drilling is more cost effective on transmission lines that



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run point to point with minimal connections but becomes expensive and evasive when considered on distribution lines.

5.1.2 System Design:

The City's water system has an urgent need for replacement of the critical water system components in within the City. Much of the City's water system infrastructure is beyond its useful life, increasing probability for failure.

The existing system does not have a preliminary model to determine if sufficient pressure is available within the project limits as defined in **Figure 7 and Figure 8.** It is recommended that a hydraulic water model be developed during the design stages of the project to estimate system's capacity and evaluate current and future new pipeline size requirements to meet the community needs.

5.1.3 Contingencies:

Contingencies have been set at 10% for the purposes of this project. Unknown subsurface conditions and shallow groundwater table can impact project costs and are not possible to estimate at this stage without geotechnical investigation. A portion of the replacement area are located near Rio Grande River. As underground conditions are better evaluated, the contingency will be lowered to match the amount of uncertainty.

5.1.4 Cost Evaluation Methodology:

The costs for system replacement are based on historical bid data received by Wilson & Company on projects of similar size, nature, and location. Costs for the system have been supplemented by information from vendors and equipment manufacturers. Dewatering was particularly difficult to evaluate for this PER, as underground conditions are not known. For the purposes if this report it is assumed that the first 2 feet of any trenching does not contain water, as the project is within previously excavated roadways. Below the first 2 feet, 60% of excavation in the "Downtown", southern portion of the "East" side, and "Williamsburg" is assumed to contain groundwater, and for the remaining areas Below the first 2 feet, 5% of excavation "North", "West" side

Operation and maintenance cost for the water line is not accounted for in this PER, as there is no additional cost that the City of T or C would realistically experience; new water lines will decrease O&M costs due to less frequent line breaks.



Costs for water losses are assumed to be equal to the cost per gallon that the City charges its customers. The cost charged to customers represents the amount that the City pays to get the water out of the ground and to the customer.

5.2 Water System Alternatives

The following Alternatives I thru VIII are considered in this report to address system issues with reliability, aging in the system, high pressure issues, and redundant water supply in the City's water system.

The alternatives IX thru XII additionally include the current Municipal Airport Water System infrastructure which is in poor conditions beyond its useful life and in need for replacement.

Alternative I: No action

Alternative II: (Full Replacement). Complete waterline replacement including "System Performance Upgrade", "North", "East", "West", "Downtown", and "Williamsburg" areas of the City, additional water supply wells near the cemetery road tanks in the northern portion of the city, and replacement of all water meters within the City (See Figure 11).

Alternative III: (System performance upgrade). Waterline replacement and installation of water meter pertaining to region characterized as "System Performance Upgrade" which entails the transmission water transmission lines throughout the city with the addition of a water supply wells near the northern tanks (See **Figure 12**).

Alternative III-A: (System High Pressure Solution). Waterline replacement and installation of water meter pertaining to the region characterized as "System High Pressure Solution", which entails the water transmission lines including pressure relief valves (PRV) throughout the city. (See **Figure 13**)

Alternative III-B: (System Redundancy and Hydraulic Performance Enhancements). Waterline replacement and installation of water meter pertaining to region characterized as "System Redundancy and Hydraulic Performance Enhancements", which entails the upsizing transmission water transmission lines located in the "East" and "Williamsburg" areas with the addition of a water supply wells near the northern tanks (See **Figure 14**).

Alternative III-C: (Additional Hydraulic Performance Enhancements). Waterline replacement and installation of water meter pertaining to region characterized as "Additional Hydraulic Performance Enhancements", which entails upsizing the remaining transmission water transmission lines



located in the "East", "North" and "Williamsburg" areas of the city to meet the remaining requirements of alternative III "System Performance Upgrade".(See **Figure 15**).

Alternative IV: (North Side). Waterline replacement and installation of water meter pertaining to region characterized as "North Side" with an addition of water supply wells near the northern tanks. (See **Figure 16**).

Alternative V: (East Side). Waterline replacement and installation of water meter pertaining to the region characterized as "East Side" with the addition of water supply wells near the northern tanks (See **Figure 17**).

Alternative VI: (West Side). Waterline replacement and installation of water meter pertaining to the region characterized as "West Side" with the addition of water supply wells near the northern tanks (See **Figure 18**)

Alternative VII: (Downtown). Waterline replacement and installation of water meter pertaining to region characterized as "Downtown" with the addition of water supply wells near the northern tanks (See **Figure 19**).

Alternative VIII: (Williamsburg). Waterline replacement and installation of water meter pertaining to region characterized as "Williamsburg" with the addition of water supply wells near the northern tanks (See **Figure 20**)

Alternative IX: (Airport Improvements). Replacement of the complete water system with a new building that will enclose two 200-gallon pressure storage tanks, chlorination system, and control panel (See **Figure 21**).

Alternative X: (Airport Improvements without fire flow). Replacement of the complete water system with a new building that will enclose a chlorination system, variable speed booster pack and control panel with an additional 7.2-thousand-gallon steel storage tank. (See **Figure 22**)

Alternative XI: (Airport Improvements with fire flow). Replacement of complete water system with a new building that will enclose a chlorination system, variable speed booster pack, fire booster pump, and control panel with an additional 200 thousand gallon steel storage tank and an additional 8 Inch waterline replacement. (See **Figure 23**)



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Alternative XII: (Airport Improvements). Replacement of the complete water system with a new building that will enclose one 30-gallon pressure storage tank, chlorination system, control panel and a 50 GPM variable speed pump. (See **Figure 24**).

Alternative I to VIII all consider replacement of service connections and distribution piping within the city area due to aging infrastructure, material, and pressure problems. Replacement for each alternative requires service line trenching. Alternatives IX to XII all consider replacement of the control, storage, and water system within the Municipal Airport due to aging infrastructure and pressure problems.



5.2.1 Alternative I: No Action

5.2.1.1 Description

This alternative involves taking no action and leaving the existing system as-is. The system will be patched/fixed as problems are encountered.

Leaving the system as-is creates potential for service disruptions for the existing residents when line breaks need to be repaired, potential for water contamination, as well as financial loss to the City due to line repairs. The estimated cost of water losses in this area has been included in the cost evaluation below.

As evaluated in the "existing facilities" portion of this report the existing system in in poor condition and in need of frequent repairs. As the city does not keep exact records of repair costs it is not easy to quantify the exact amount spent on repairs. Per the City approximately 260 system repairs are required yearly in this area. The per-repair cost has been estimated to be one thousand dollars. A yearly cost for repairs, inflated at 2.25% discount rate, has been used to calculate the present cost for maintaining the system within the 20-year evaluation period.

5.2.1.2 Water and Energy Efficiency

This alternative is the least water/energy efficient option of the eight considered in this report. As stated previously the amount of lost water in this portion of the network is estimated at ~120 Million gallons per year. This amount of water represents \$211,032 in lost revenue per year, at the rate the city charges per gallon of water.

5.2.1.3 Green Infrastructure

As the current system is in poor condition, 120 Million gallons of water are estimated to be lost per year in this portion of the system (See **Appendix 8**) as water conservation is of the highest priority in New Mexico due to limited water supply, it is essential that these large losses be minimized.

5.2.1.4 Land Requirements

No additional land requirements are necessary for this alternative, all existing infrastructure is to remain as-is.

5.2.1.5 Potential Construction Problems

Since the existing system has the potential for a line break at any time, the City must be ready to repair line breaks and orchestrate road closures at any time. This can lead to business disruptions, traffic disruptions, and other service issues that cannot be anticipated.



5.2.1.6 Resiliency and Operational Simplicity

This is the least operator friendly alternative, as it involves the potential for unscheduled line breakages. The system will become more unreliable as time passes and components surpass their operational life expectancy.

5.2.1.7 Cost Estimates

This alternative has no capital costs associated with it.

5.2.1.8 Alternative Pros/Cons

ADVANTAGES:

This option has no capital cost

DISADVANTAGES:

- This option continues the safety issues (in the form of infiltration and lack of fire flow)
- This option continues to waste large amounts of water
- This option does not solve the high pressures that causes the aging infrastructure to break more often
- This option continues to create service outages for residents
- This option continues the constant line break repairs

5.2.1.9 Cost Summary

Table 13: Alternative I Cost Summary

Alternative I-No Action				
20 Yrs O&M PW	\$	11,871,223		
Construction Cost	\$	-		
Non-Construction Cost	\$	-		
Total	\$	11,871,223		

The annual 2020 Operation and maintenance is \$743,638 see breakdown provided in **Appendix** 5



5.2.2 Alternative II: Complete Water System Replacement

5.2.2.1 Description

Alternative II involves replacing 57 percent of the existing waterlines within the city with new pipeline equal or less than 6 inches PVC C-900 DR-18. This alternative will replace 96.6 percent of pipe over 30 years old. All waterlines in this alternative are replaced via open trench by placing the new line and abandon in place the existing waterline except where noted otherwise. Areas of the City of Truth or Consequences were evaluated based on current GIS information, upsizing the existing water line to a 6, 810, 12, and 14 inch will significantly increase available pressure in the city as well as provide for better fire flow capacity. The new water line is assumed to be installed in the shoulder of the road, with 6-12' of pavement removal, and removal off any, sidewalk or curb and gutter as needed. If a water meter is found in the existing roadway and is to be replaced. This portion of line repairs also includes the replacement of the existing casing and crossing pipe underneath any NMDOT ROW's via bore and jack.

The proposed water well will be located on Cemetery Road, the system currently has six wells all of them located on the southern part of the City, and the current water system design uses two pumping stations to feed the water storage tanks on the north part of the city. A new well located on the north end of the City will provide reliable water production back up and prevent water outages, if any of the southern wells or booster stations fail. It would also provide an additional water source when one of City's existing wells fail due to age.

5.2.2.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" portion of this report, it has been determined that most of the pipes within the City should be replaced in their entirety. **Figure 11** shows this alternative's recommendations. Existing flow capacity has been determined to generally be not sufficient per pipe pressure fluctuation during peak flow periods as well as not meeting the fire flow requirement in multiple areas of the city. **Exhibit 110** in **Appendix 6** shows the complete system by pipe size. Increasing pipe size as needed within the neighborhoods is recommended at this time. All pipes are assumed to be replaced with PVC C-900 DR18 with sizes 6-Inch or greater, dewatering will be necessary as described in the previous "Cost Evaluation Methodology" section via open trench.

Six additional Pressure Reducing Valves (PRV) are recommended to be installed within the system on the northern and south part of the city to avoid high pressure peaks which results in waterline breaks within the City's neighborhoods. These new PRV's will also allow the City to use



the full volume of the Cemetery road tanks, which are currently operating at 50% so to minimize the high pressure in the west side area that cause a majority of the waterline breaks.

5.2.2.3 Replacement of City Water Meters

Due to aging inaccurate meter readings and manually reading record described in the "Existing Facilities" portion of this report, it has been determined that all of the water meters should be replaced in their entirely. Water meters are currently older than 40 years, exceeding their useful life. This provides incorrect data regarding water usage, water loss percentages and has a negative impact on the City's billing system. The new meter shall be automatic radio read meters integrated into the city's electrical billing system. This will reduce the manpower needed to reading the meters, which will reduce the labor cost on the water system.

5.2.2.4 Construction of Water Well Northern Area

Presently there is a dependence of Cook Street and Morgan Street Booster stations to provide water to the northern area of the City. Water production back up in the northern area of the city is non-existent making the distribution system open for failure if either of the booster stations do not work as desired. Currently the system has six wells all of them located on the southern part of the City; most of fall past or are near their end of useful life. A new well located in the north will provide reliable water production back up and prevent water outages.

Additionally, most of the water system users in this northern area are currently connected to the northern tanks located on Cemetery Road. This new water source, when connected to the Upper tanks located on Cemetery Road, will provide a reliable back up water supply to this area and the rest of the city if needed under emergency situation.

5.2.2.5 Water and Energy Efficiency

This alternative will cost approximately \$12,000 additional per year in electricity costs due to the new well pump, the addition of a new well will reduce the cost of boosting the water from the southern part of the City to the north tanks, which could counter the O&M cost of this improvement. The amount of lost water in this portion of the system is estimated at ~ 116 Million gallons per year. This amount of water represents \$203,857 in lost revenue per year, at the rate the city charges per gallon of water.



5.2.2.6 **Green Infrastructure**

This alternative will reduce water losses by approximately 116 Million gallons per year that occurs through line breaks within the current system, which is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.

5.2.2.7 <u>Land Requirements</u>

Minimal additional land requirements are anticipated for the installation of the new water well. A small easement may need to be purchased by the City in order to build this water well. If the land is privately owned, if this option is pursued the owner will need to be determined.

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.

5.2.2.8 Potential Construction Problems

The largest potential for construction problems in this alternative lies on the neighborhoods located on each side of I-25 business route which will require service lines crossing all lanes within an NMDOT owned road, Crossings will either require extensive closures, or more likely will require directional. It is assumed that drilling will be required, and a bid item for drilling has been included in the cost estimate for this portion of the alternative.

Dewatering quantities are another large potential concern for this alternative, a large proportion of these main transmission lines run parallel with the Rio Grande which indicate a shallow water table. Existing water levels on excavation trenches cannot be quantified until further examination. As explained in the "existing system" part of this report, waterlines replaced within the southern portion of the "East Side" and "Downtown" areas will have 60 percent dewatering of the trench in other areas of the city 5 percent dewatering will be assumed.

5.2.2.9 Resiliency and Operational Simplicity

The only regular maintenance items for this alternative are the new water well pumping station, and PRV's, which will require periodic maintenance as recommended by the manufacturer. The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.



5.2.2.10 Alternative Pros/Cons

ADVANTAGES:

- This option fixes safety issues (in the form of infiltration and lack of fire flow)
- This option eliminates high pressures issues that caused the aging infrastructure to break more often
- This option conserves largest amounts of water
- This option eliminates service outages for residents
- This option improves the backup and redundancy of the water system
- This option improves approximately 97 percent of the aging water system

DISADVANTAGES:

- This option has the highest capital cost and it is out of the City's budget
- This option requires a large amount of NMDOT crossing permits
- This option has additional O&M for the new well, but pumping cost will be countered by not boosting the water twice to this upper zone
- This option does will modify billing rates

5.2.2.11 Cost Summary

Table 14: Alternative II Cost Summary

Alternative II- Complete System				
20 Yrs O&M PW	\$	9,325,812		
Construction Cost	\$	88,435,392		
Non-Construction Cost	\$	14,137,544		
Total	\$	111,898,748		

The annual 2020 Operation and maintenance is \$584,188. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.



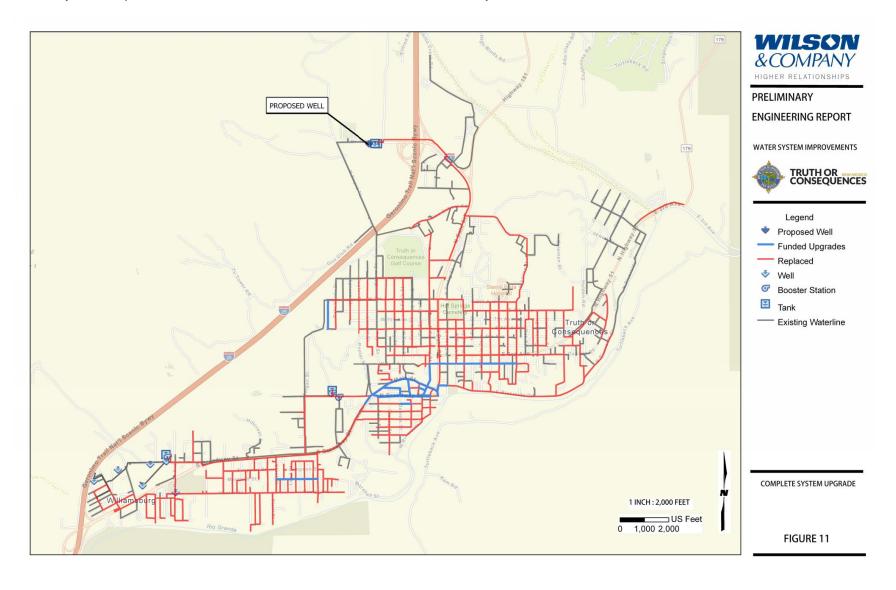


Figure 11: Alternative II Complete System upgrade



5.2.3 Alternative III: System Performance Upgrade

5.2.3.1 Description

Alternative III involves replacing 11.3 percent of the existing waterlines within the city with new pipeline equal to, or greater than, 6-inch PVC C-900 DR-18. This alternative will replace 37.8 percent of pipe this is over 30 years old. This replacement also upgrades around 15 percent of the Asbestos Cement (AC), Cast iron (CI), and Ductile Iron (DI) material in the existing system. All waterlines in this alternative are replaced via open trench by placing the new line parallel to the existing and abandoning the existing water line in place except where noted otherwise. Areas of the City of Truth or Consequences were evaluated based on current GIS information. Upsizing the existing water line to 8, 10, 12, and 14 inch will significantly adjust available pressure in the City, as well as provide for better fire flow capacity important areas such as the City's hospital and high school. This alternative significantly increases available pressure in the City and provide for better fire flow capacity. The new water line is assumed to be installed in the shoulder of the road, with 6-12' of pavement removal and removal for any sidewalk or curb and gutter as needed. If a water meter is found in the existing roadway it is to be replaced. This portion of line repairs also includes the replacement of the existing casing and crossing pipe underneath any NMDOT ROW's via jack and bore construction methods.

The proposed water well will be located on Cemetery Road. The system currently has six wells, all of them located on the south part of the city near "Williamsburg". Additionally, the existing water system uses two pumping stations to feed the water storage tanks on the north part of the city. A new well located in the north will provide reliable water production back up and prevent water outages if any of the southern wells or booster stations fail. It would also provide an additional water source when one of City's existing wells fail due to their age.

5.2.3.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" portion of this report, infrastructure defined as "System Performance Upgrade" (See **Figure 12**) should be replaced. Since these particular waterlines are also known as the main transmission lines, their primary purpose is to ensure that water transmission runs from Cook Street Booster station to Morgan Street Booster station to finally provide water flow to multiple areas of the city. By replacing these particular lines, the water system will reduce the 30 psi pressure fluctuation in the system, mostly in the "Williamsburg" and "East" areas.



The existing flow capacity has been determined to be insufficient due to several breakages reports and the inability to meet fire flow requirement. All pipes are assumed to be replaced with PVC C-900 DR18, sizes 6 Inch greater. Dewatering of groundwater is a consideration in this alternative as described in the previous "Cost Evaluation Methodology" section via open trench.

Six Additional Pressure Reducing Valves (PRV) are recommended to be installed within the system on the northern and south parts of the city. This is to avoid high pressure peaks which results in water breaks within the city's neighborhoods.

5.2.3.3 Replacement of City Water Meters

Due to aging, inaccurate meter readings and manually reading record described in the "Existing Facilities" portion of this report, it has been determined that all of the water meters should be replaced in their entirely. Water meters are currently older than 40 years exceeding their useful life. This provides incorrect data regarding water usage, water loss percentages and has a negative impact on the City's billing system. The new meters shall be automatic radio read meters integrated into the city's electrical billing system. This will reduce the manpower needed to read the meters, which will reduce the labor cost on the system.

5.2.3.4 Construction of Water Well Northern Area

Presently there is a dependence on the Cook Street and Morgan Street Booster stations to provide water to the northern area of the City. Back up water production in the northern area of the City is non-existent, making the distribution system open for failure if any of the booster stations don't work as desired. The system has six wells all of them located on the southern part of the city, most of which fall on are near their end of useful life. A new well located in the north will provide reliable backup water production and prevent water outages.

Additionally, most of the water system users in this northern area are currently connected to the northern tanks located on Cemetery Road. This new water source, when connected to the Upper tanks located on Cemetery Road, will provide a reliable backup water supply to this area and the rest of the city if needed under an emergency situation.

5.2.3.5 Water and Energy Efficiency

This alternative will cost approximately \$12,000 additional per year in electricity costs due to the new well pump, the addition of a new well will reduce the cost of boosting the water from the southern part of the City to the north tanks, which could counter the O&M cost of this improvement. The amount of lost water in this portion of the system is estimated at ~ 45 Million



gallons per year. This amount of water represents \$79,770 in lost revenue per year, at the rate the city charges per gallon of water.

5.2.3.6 Green Infrastructure

This alternative will reduce water losses by approximately 45 Million gallons per year due to line breaks, which is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.

5.2.3.7 **Land Requirements**

Minimal additional land requirements are anticipated for the installation of the new water well. A small easement may need to be purchased by the City in order to build this water well. If the land is privately owned, and if this option is pursued, the owner of this land will need to be determined.

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.

5.2.3.8 Potential Construction Problems

The largest potential for construction problems in this alternative lies on the neighborhoods located on each side of I-25 business route which will require service lines crossing all lanes within an NMDOT owned road, Crossings will either require extensive closures, or more likely will require directional drilling. It is assumed that drilling will be required, and a bid item for drilling has been included in the cost estimate for this portion of the alternative.

Dewatering quantities are another large potential concern for this alternative. A large proportion of these main transmission's lines run parallel with the Rio Grande, which indicate a shallow water table. Existing water levels in excavation trenches cannot be quantified until further examination. As explained in the "existing system" part of this report waterlines replaced within the southern portion of the "East Side" and "Downtown" areas will have 60 percent dewatering of the trench. In other areas of the city 5 percent dewatering will be assumed.

5.2.3.9 Resiliency and Operational Simplicity

The only regular maintenance item for this alternative are the new water well pump, and the PRV's which will require periodic maintenance as recommended by the manufacturer. The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.



5.2.3.10 Alternative Pros/Cons

ADVANTAGES:

- This option fixes a majority of the safety issues (in the form of infiltration and lack of fire flow)
- This option eliminates high pressures issues that caused the aging infrastructure to break more often
- This option conserves a large percentage of water, close to half the water losses
- This option extremely reduces service outages for residents
- This option improves the backup and redundancy to the water system
- This option improves approximately 13 percent of the existing water system
- This option improves approximately 38 percent of the aging water system
- This option doesn't produce any changes on billing charges

DISADVANTAGES:

- This option doesn't have a capital cost that is within the City's budget
- This option has a large dewatering cost for the areas near the Rio Grande
- This option requires a large amount of NMDOT crossing permits
- This option has additional O&M for the new well, but pumping cost will be countered by not boosting the water twice to this upper zone

5.2.3.11 Cost Summary

Table 15: Alternative III Cost Summary

Alternative III- System Performance Update			
20 Yrs O&M PW	\$10,989,446		
Construction Cost	\$21,701,685		
Non-Construction Cost	\$4,093,148		
Total	\$36,784,279		

The annual 2020 Operation and maintenance is \$688,402. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.



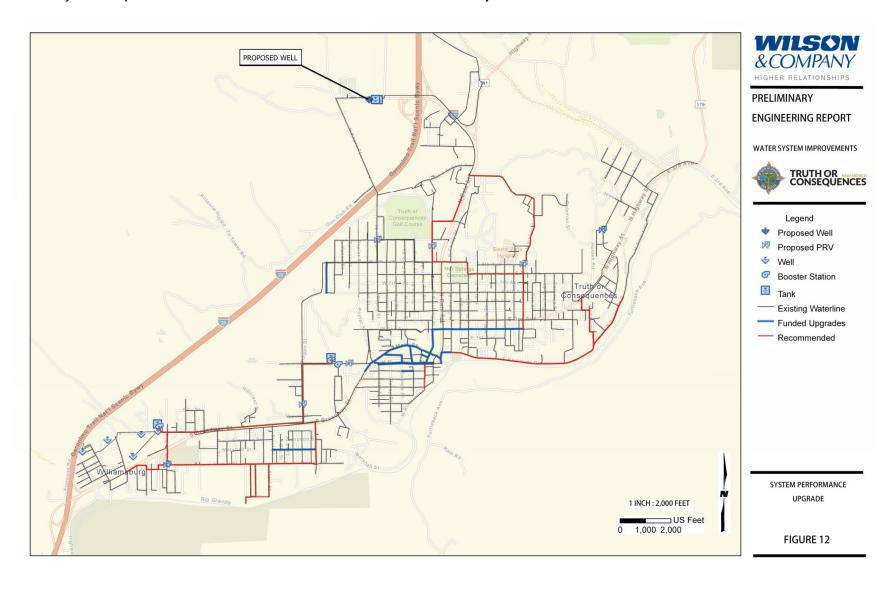


Figure 12: Alternative III System Performance Upgrade



5.2.4 Alternative III-A: System High Pressure Solution

5.2.4.1 Description

Alternative III-A addresses the high-pressure issues in the "West" and "Williamsburg" areas by replacing the Cook St. to Morgan St. main transmission line and installing main lines PRV to eliminate high pressures issues within the City's water system. This involves replacing 6.2 percent of the existing waterlines within the city that are 6 inches or less diameter, with new pipeline PVC C-900 DR -18 pipelines 6 inches or greater. This alternative will replace 26.7 percent of pipe over 30 years old, this replacement also upgrades around 16.9 percent of the Asbestos Cement (AC), Cast iron (CI), and Ductile Iron (DI) material in the existing system. All waterlines in this alternative are replaced via open trench by placing the new line parallel to the existing and abandoning the existing waterline in place; except where noted otherwise. Areas of the City of Truth or Consequences were evaluated based on current GIS information, upsizing the existing water line to an6, 8, 10, and 12, inch will significantly adjust available pressure in the City as well as provide for better fire flow capacity including important areas such as the City's hospital and a City's high school. This alternative significantly increases available pressure in the City and provide for better fire flow capacity. The new water line is assumed to be installed in the shoulder of the road, with 6-12' of pavement removal, and removal of any side walk or, curb and gutter if a water meter is found in the existing roadway and needs to be replaced. This portion of line repairs also includes the replacement of the existing casing and crossing pipe underneath any NMDOT ROW's via jack and bore construction methods.

5.2.4.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" portion of this report, infrastructure defined as "System High Pressure Solutions" (**Figure 13**) should be replaced. Since these four particular waterlines are also known as the main transmission lines, their primary purpose is to ensure that water transmission runs from Cook Street Booster station to Morgan Street Booster station to finally provide water flow to multiple areas of the city. By replacing these particular lines, the water system will reduce the 30 psi pressure fluctuation in the system, mostly in the "Williamsburg" and "East" areas

Waterlines replacements locating on the "North" area feeding the City's high school and hospital, Upsizing a main cast iron waterline located on portions of East 8th and East 9th Streets in the "East" side of the city, and additionally replacing and looping an area in the "Williamsburg" area will also prevent pressure fluctuation in the system and mostly in the "Williamsburg" and "East" areas and will ensure water quality for the "Williamsburg" area.



The existing flow capacity has been determined to be insufficient due to several breakages reports and the inability to meet fire flow requirement. All pipes are assumed to be replaced with PVC C-900 DR18, sizes with 6 Inch or greater. Dewatering of groundwater is a consideration in this alternative as described in the previous "Cost Evaluation Methodology" section via open trench.

Six Additional Pressure Reducing Valves (PRV) are recommended to be installed within system on the northern and south part of the city. This is to avoid high pressure peaks which results in water breaks within the city's neighborhoods.

5.2.4.3 Replacement of City Water Meters

Due to aging, inaccurate meter readings and manually reading record described in the "Existing Facilities" portion of this report, it has been determined that all of the water meters should be replaced in their entirely. Water meters are currently older than 40 years, exceeding their useful life. This provides incorrect data regarding water usage, water loss percentages and has a negative impact on the City's billing system. The new meters shall be automatic radio read meters integrated into the city's electrical billing system. This will reduce the manpower needed to read the meters, which will reduce the labor cost on the system.

5.2.4.4 Water and Energy Efficiency

The amount of lost water in this portion of the system is estimated at ~ 32 Million gallons per year. This amount of water represents \$56,346 in lost revenue per year, at the rate the city charges per gallon of water.

5.2.4.5 Green Infrastructure

This alternative will reduce water losses by approximately 32 Million gallons per year due to line breaks, which is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.

5.2.4.6 Land Requirements

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.

5.2.4.7 Potential Construction Problems

The largest potential for construction problems in this alternative lies on the neighborhoods located on each side of I-25 business route which will require service lines crossing all lanes within an NMDOT owned road. Crossings will either require extensive closures, or more likely, will



require directional drilling. It is assumed that drilling will be required, and a bid item for drilling has been included in the cost estimate for this portion of the alternative.

Dewatering quantities are another large potential concern for this alternative. A large proportion of these main transmission's lines run parallel with the Rio Grande, which indicate a shallow water table. Existing water levels in excavation trenches cannot be quantified until further examination. As explained in the "existing system" part of this report, waterlines replaced within the southern portion of the "East Side" and "Williamsburg" areas will have 60 percent dewatering of the trench. In other areas of the city 5 percent dewatering will be assumed.

5.2.4.8 Resiliency and Operational Simplicity

The only regular maintenance item for this alternative are the PRV 's which will require periodic maintenance as recommended by the manufacturer. The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.

5.2.4.9 Alternative Pros/Cons

ADVANTAGES:

- This option has a capital cost that is within the City's budget
- This option fixes a large percentage of the safety issues (in the form of infiltration and lack of fire flow)
- This option eliminates high pressures issues that caused the aging infrastructure to break more often.
- This option conserves a large percentage of water, close to a third of the water losses
- This option extremely reduces service outages for residents
- This option improves approximately 6.2 percent of the existing water system
- This option improves approximately 27 percent of the aging water system
- This option doesn't produce any changes on billing charges

DISADVANTAGES:

- This option has a large dewatering cost for the "East" and "Williamsburg" areas near the Rio Grande
- This option requires a large amount of NMDOT crossing permits
- This option doesn't improve the backup and redundancy to the water system



5.2.4.10 Cost Summary

Table 16: Alternative III-A Cost Summary

Alternative III A- System High Pressure Solution			
20 Yrs O&M PW	\$11,115,798		
Construction Cost	\$6,208,432		
Non-Construction Cost	\$1,321,898		
Total	\$18,646,128		

The annual 2020 Operation and maintenance is \$696,317. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.



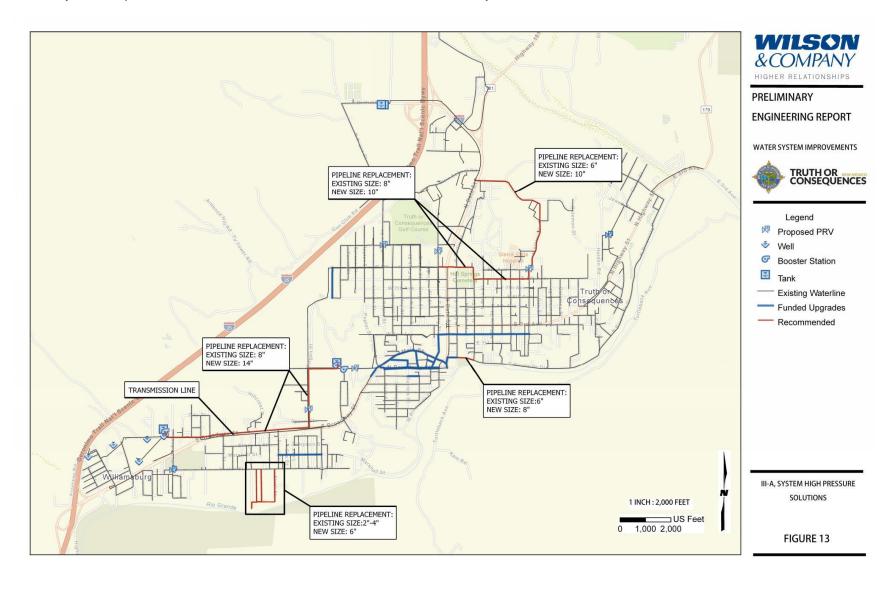


Figure 13: III-A System High Pressure Solutions



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5.2.5 Alternative III-B: System Redundancy and Hydraulic Performance Enhancements

5.2.5.1 Description

Alternative III-B improves the system hydraulics by replacing and upgrading main distribution lines to increase the efficiency of the water distribution throughout the City's. This involves replacing 3.6 percent of the existing waterlines within the city with new pipeline equal to or greater than, 6inch PVC C-900 DR-18. This alternative will replace 6.3 percent of pipe over 30 years old, this replacement also upgrades around 4.7 percent of the Asbestos Cement (AC), Cast iron (CI), and Ductile Iron (DI) materials in the existing system. All waterlines in this alternative are replaced via open trench by placing the new line parallel to the existing and abandoning the existing water line in place except where noted otherwise. Areas of the City of Truth or Consequences were evaluated based on current GIS information. Upsizing the existing water line to an 8, and 10 inch will significantly adjust available pressure in the City as well as provide for better fire flow capacity throughout the City. This alternative significantly increases available pressure in the City and provide for better fire flow capacity. The new water line is assumed to be installed in the shoulder of the road, with 6-12' of pavement removal, and removal for any side walk and curb and gutter if a water meter is found in the existing roadway it is to be replaced. This portion of line repairs also includes the replacement of the existing casing and crossing pipe underneath any NMDOT ROW's via jack and bore construction methods.

The proposed water well will be located on Cemetery Road. The system currently has six wells all of them located on the south part of the city near "Williamsburg". Additionally, the existing water system uses two pumping stations to feed the water storage tanks on the north part of the city. A new well located in the north will provide reliable water production back up and prevent water outages if any of the southern wells or booster stations fail. It would also provide an additional water source when one of City's existing wells fail due to their age.

5.2.5.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" portion of this report, infrastructure defined as "System Redundancy and Hydraulic Performance Enhancements" (See **Figure 14**) should be replaced. Since these particular waterlines located in the "East" and "Williamsburg" areas will continue to help with pressure issues and improving fire flow requirements throughout the city,.

The existing flow capacity has been determined to be insufficient due to several breakage reports and the inability to meet fire flow requirement. All pipes are assumed to be replaced with PVC C-



900 DR18, sizes 6 Inch or greater. Dewatering of groundwater is a consideration in this alternative as described in the previous "Cost Evaluation Methodology" section via open trench.

5.2.5.3 Replacement of City Water Meters

Due to aging, inaccurate meter readings and manually reading record described in the "Existing Facilities" portion of this report, it has been determined that all of the water meters should be replaced in their entirely. Water meters are currently older than 40 years, exceeding their useful life. This provides incorrect data regarding water usage, water loss percentages and has a negative impact on the City's billing system. The new meter shall be automatic radio read meters integrated into the city's electrical billing system. This will reduce the manpower for reading the meters manually, which reduce the labor cost on the system.

5.2.5.4 Construction of Water Well Northern Area

Presently there is a dependence on the Cook Street and Morgan Street Booster stations to provide water to the northern area of the City. Back up water production in the northern area of the City is non-existent making the distribution system open for failure if any of the booster stations don't work as desired. The system has six wells, all of them located on the southern part of the city, most of which fall on are near their end of useful life. A new well located on the north will provide reliable backup water production and prevent water outages.

Additionally, most of the water system users in this northern area are currently connected to the northern tanks located on Cemetery Road. This new water source, when connected to the Upper tanks located on Cemetery Road, will provide a reliable backup water supply to this area and the rest of the city if needed under an emergency situation.

5.2.5.5 Water and Energy Efficiency

This alternative will cost approximately \$12,000 additional per year in electricity costs due to the new well pump, the addition of a new well will reduce the cost of boosting the water from the southern part of the City to the north tanks, which could counter the O&M cost of this improvement. The amount of lost water in this portion of the system is estimated at ~ 7 Million gallons per year. This amount of water represents \$13,295 in lost revenue per year, at the rate the city charges per gallon of water.



5.2.5.6 **Green Infrastructure**

This alternative will reduce water losses by approximately 7Million gallons per year due to line breaks, which is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.

5.2.5.7 <u>Land Requirements</u>

Minimal additional land requirements are anticipated for the installation of the new water well. A small easement may need to be purchased by the City in order to build this water well. If the land is privately owned, and if this option is pursued the owner of this land will need to be determined.

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.

5.2.5.8 Potential Construction Problems

The largest potential for construction problems in this alternative lies on the neighborhoods located on each side of I-25 business route which will require service lines crossing all lanes within an NMDOT owned road. Crossings will either require extensive closures, or more likely will require directional drilling. It is assumed that drilling will be required, and a bid item for drilling has been included in the cost estimate for this portion of the alternative.

Dewatering quantities are another large potential concern for this alternative. A large proportion of these main transmissions lines run parallel with the Rio Grande which indicate a shallow water table. Existing water levels in excavation trenches cannot be quantified until further examination. As explained in the "existing system" part of this report, waterlines replaced within the southern portion of the "East Side" area will have 60 percent dewatering of the trench. In areas of the city 5 percent dewatering will be assumed.

5.2.5.9 Resiliency and Operational Simplicity

The only regular maintenance items for this alternative are the new water well pump which will require periodic maintenance as recommended by the manufacturer. The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.



5.2.5.10 Alternative Pros/Cons

ADVANTAGES:

- This option has a capital cost that is within the City's budget
- This option fixes a majority of the safety issues (in the form of infiltration and lack of fire flow)
- This option continues to eliminate high pressures issues
- This option supplements the conservation of a large percentage of water, close to half water losses
- This option reduces service outages for residents, due to the supply and distribution system redundancy
- This option improves the backup and redundancy to the water system
- This option improves approximately 4 percent of the existing water system
- This option improves approximately 4.7 percent of the aging water system
- This option doesn't produce any changes on billing charges

DISADVANTAGES:

- This option has a large dewatering cost for the "East" area near the Rio Grande
- This option has additional O&M for the new well, but pumping cost will be countered by not boosting the water twice to this upper zone

5.2.5.11 Cost Summary

Table 17: Alternative III B Cost Summary

Alternative III B- System Redundancy and Hydraulic Performance Enhancements		
20 Yrs O&M PW	\$11,880,678	
Construction Cost	\$8,253,588	
Non-Construction Cost	\$1,590,443	
Total	\$21,724,709	

The annual 2020 Operation and maintenance is \$744,230. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.



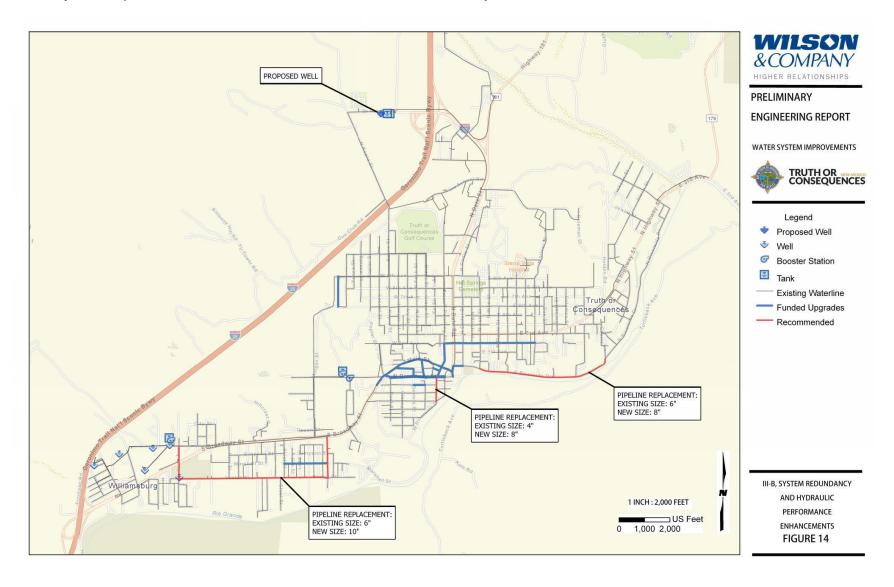


Figure 14: III-B System Redundancy and Hydraulic Enhancements



5.2.6 Alternative III-C: Additional Hydraulic Performance Enhancements

5.2.6.1 Description

Alternative III-C completes the "System Performance Upgrade" alternative to ensure proper water distribution and pressure throughout the City's water system. This involves replacing 2.9 percent of the existing waterlines within the city with new pipeline equal to or greater than 6-inch PVC C-900 DR-18. This alternative will replace 5.2 percent of pipe this is over 30 years old. This replacement also upgrades around 2.9 percent of the Asbestos Cement (AC), Cast iron (CI), and Ductile Iron (DI) material in the existing system. All waterlines in this alternative are replaced via open trench by placing the new line parallel to the existing and abandoning the existing water line in place except where noted otherwise. Areas of the City of Truth or Consequences were evaluated based on current GIS information. Upsizing the existing water line to an 8, 10, and 12 inch will significantly adjust available pressure in the City, as well as provide for better fire flow capacity throughout the City. This alternative significantly increases available pressure in the City and provides for better fire flow capacity. The new water line is assumed to be installed in the shoulder of the road, with 6-12' of pavement removal, and removal for any sidewalk or curb and gutter as needed. If a water meter is found in the existing roadway it is to be replaced. This portion of line repairs also includes the replacement of the existing casing and crossing pipe underneath any NMDOT ROW's via jack and bore construction methods.

5.2.6.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" portion of this report, infrastructure defined as "Additional Hydraulic Performance Enhancements" (See **Figure 15**) should be replaced Since these particular waterlines located in the "East", "Williamsburg" and "North" areas will complete to help with pressure issues and improving fire flow requirements throughout the city.

The existing flow capacity has been determined to be insufficient due to several breakage reports and the inability to meet fire flow requirement. All pipes are assumed to be replaced with PVC C-900 DR18, sizes with 6 Inch or greater. Dewatering of groundwater is a consideration in this alternative as described in the previous "Cost Evaluation Methodology" section via open trench.

5.2.6.3 Replacement of City Water Meters

Due to aging, inaccurate meter readings and manually reading record described in the "Existing Facilities" portion of this report, it has been determined that all of the water meters should be replaced in their entirely. Water meters are currently older than 40 years exceeding their useful



life. This provides incorrect data regarding water usage, water loss percentages and has a negative impact on the City's billing system. The new meters shall be automatic radio read meters integrated into the city's electrical billing system. This will reduce the manpower needed to read the meters, which will reduce the labor cost on the system.

5.2.6.4 Water and Energy Efficiency

The amount of lost water in this portion of the system is estimated at \sim 6 Million gallons per year. This amount of water represents \$10,974 in lost revenue per year, at the rate the city charges per gallon of water.

5.2.6.5 **Green Infrastructure**

This alternative will reduce water losses by approximately 6 Million gallons per year due to line breaks, which is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.

5.2.6.6 Land Requirements

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.

5.2.6.7 Potential Construction Problems

The largest potential for construction problems in this alternative lies on the neighborhoods located on each side of I-25 business route which will require service lines crossing all lanes within an NMDOT owned road, which will either require extensive closures, or more likely, will require directional drilling. It is assumed that drilling will be required, and a bid item for drilling has been included in the cost estimate for this portion of the alternative.

Dewatering quantities are another large potential concern for this alternative. A large proportion of these main transmission's lines run parallel with the Rio Grande which indicate a shallow water table. Existing water levels in excavation trenches cannot be quantified until further examination. As explained in the "existing system" part of this report waterlines replaced within the southern portion of the "East Side" area will have a 60 percent dewatering of the trench. In other areas of the city 5 percent dewatering will be assumed.

5.2.6.8 Resiliency and Operational Simplicity

No additional Maintenance item are added in this alternative. The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.



5.2.6.9 Alternative Pros/Cons

ADVANTAGES:

- This option has a capital cost that is within the City's budget
- This option fixes a majority of the safety issues (in the form of infiltration and lack of fire flow)
- This option intensifies the elimination of pressures issues.
- This option concludes the conservation of a large percentage of water, close to half water losses
- This option continues to reduce service outages for residents, due to distribution system redundancy
- This option improves approximately 3 percent of the existing water system
- This option improves approximately 3 percent of the aging water system
- This option doesn't produce any changes on billing charges

DISADVANTAGES:

- This option has a large dewatering cost for the "East" area near the Rio Grande
- This option requires a significant amount of NMDOT crossing permits

5.2.6.10 Cost Summary

Table 18: Alternative III C Cost Summary

Alternative III C- Additional Hydraulic Performance Enhancements			
20 Yrs O&M PW	\$11,724,099		
Construction Cost	\$5,280,984		
Non-Construction Cost	\$989,461		
Total	\$17,994,544		

The annual 2020 Operation and maintenance is \$734,422. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.



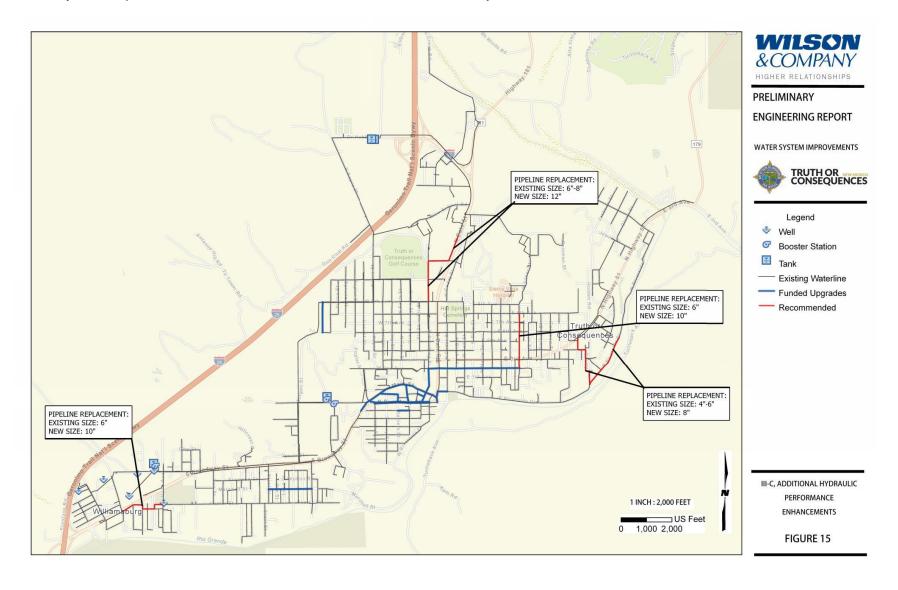


Figure 15: III-C Additional Hydraulic Performance Enhancements



5.2.7 Alternative IV: North Side Replacement

5.2.7.1 Description

Alternative IV involves replacing 2.9 percent of the existing waterlines within the city that are 6 inches or less in diameter, with the new PVC C-900 DR-18. This alternative will replace 5.1 percent of pipe over 30 years old. All waterlines in this alternative are replaced via open trench by constructing the new waterline parallel to the existing, then abandoning the existing waterline in place; except where noted otherwise. Areas of the City of Truth or Consequences were evaluated based on current GIS information, by upsizing the existing water line to 6, 8, 12, and 14-inch diameter. The new water line is assumed to be installed in the shoulder of the road, with 6-12' of pavement removal and removal for sidewalk or, curb and gutter as needed. If an existing water meter is found in the existing road, it is to be replaced. This alternative includes the replacement of the existing casing and crossing pipe underneath any NMDOT ROW's via jack and bore construction methods.

The proposed water well will be located on Cemetery Road, the system currently has six wells, all of them located on the southern part of the City. The current water system uses two pumping stations to feed the water storage tanks on the north part of the City. A new well located in the north will provide reliable back up water production back up and prevent water outages if any of the southern wells or booster stations fail. It would also provide an additional water source when one of City's existing wells finally fail due to their age.

5.2.7.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" the portion of this report defined as "North Side" (See **Figure 16**) should be replaced. Existing flow capacity has been determined to be generally insufficient, per pipe pressure fluctuation during peak flow periods and not meeting fire flow requirements on multiple areas of the city. Pipe size increases within the neighborhoods are recommended to address flow and pressure issues. All pipes are assumed to be replaced with PVC C-900 DR18 with sizes 6 Inch or greater. Dewatering as described in the previous "Cost Evaluation Methodology" section via open trench.

5.2.7.3 Replacement of City Water Meters

Due to aging meters, inaccurate meter readings, and manually reading record described in the "Existing Facilities" portion of this report, it has been determined that all of the water meters should be replaced in their entirely. Many of the water meters are currently older than 40 years exceeding



their useful life, older meters may be providing incorrect data regarding water usage, and water loss percentages and also having a negative impact on the City's billing system.

5.2.7.4 Construction of Water Well Northern Area

Presently there is a dependence on Cook Street and Morgan Street Booster stations to provide, a backup source of water in the northern area of the City. Backup water production in the northern area of the City is non-existent, making the distribution system open for failure if either of the booster stations do not work as desired. Currently the system has six wells, all of them located in the southern part of the City, most of which are near their end of useful life. A new well located in the north will provide reliable backup water production and prevent water outages

Additionally, most of the water system users in this northern area are currently connected to the northern tanks located on Cemetery Road. This new water source, when connected to the Upper tanks located on Cemetery Road, will provide a reliable back up water supply to this area and the rest of the city if needed under an emergency situation.

5.2.7.5 Water and Energy Efficiency

This alternative will cost approximately \$12,000 additional per year in electricity costs due to the new well pump, the addition of a new well will reduce the cost of boosting the water from the southern part of the City to the north tanks, which could counter the O&M cost of this improvement. The amount of lost water in this portion of the system is estimated at ~ 6 Million gallons per year. This amount of water represents \$10,763 in lost revenue per year, at the rate the city charges per gallon of water.

5.2.7.6 Green Infrastructure

This alternative will reduce water losses by approximately 6 Million gallons per year due to 15 within this area reported by the city officials. This is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.

5.2.7.7 Land Requirements

Minimal additional land requirements are anticipated for the installation of the new water well. A small easement may need to be purchased by the City in order to build this water well. If the land is privately owned, and if this option is pursued the owner will need to be determined.

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.



5.2.7.8 Potential Construction Problems

The largest potential for construction problems in this alternative lies in the neighborhoods located on each side of I-25 business route which will require service lines crossing all lanes within n NMDOT owned road. Construction will either require extensive closures, or more likely directional drilling for the new service lines. It is assumed that drilling will be required, and a bid item for drilling has been included in the cost estimate for this portion of the alternative.

Groundwater dewatering is another large potential concern for this alternative. A large portion of these main transmission lines run parallel with the Rio Grande, which contributes to a shallow groundwater table in this area. This alternative will assume 5 percent dewatering in areas that are not near the Rio Grande.

5.2.7.9 Resiliency and Operational Simplicity

The only regular maintenance item for this alternative is the new water well pump, which will require periodic maintenance as recommended by the manufacturer. The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.

5.2.7.10 Alternative Pros/Cons

ADVANTAGES:

- This option has a low capital cost
- This option fixes safety issues on the North Side (in the form of leakage and lack of fire flow)
- This option eliminates high pressures issues that caused the aging infrastructure to break more often
- This option improves water supply to the water system
- This option improves approximately 3 percent of the existing water system
- This option improves approximately 5 percent of the aging water system

DISADVANTAGES:

- This option requires a large amount of NMDOT crossing permits
- This option has additional O&M for the new well, but pumping cost will be countered by not boosting the water twice to this upper zone
- This option doesn't reduce a large amount of water losses
- This option doesn't reduce pressure peaks in the system that causes the water breaks
- This option doesn't prevent service outages for residents



5.2.7.11 <u>Cost Summary</u>

Table 19: Alternative IV Cost Summary

Alternative IV- North Side	
20 Yrs O&M PW	\$11,914,630
Construction Cost	\$7,372,834
Non-Construction Cost	\$1,499,712
Total	\$20,787,176

The annual 2020 Operation and maintenance is \$746,357. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.



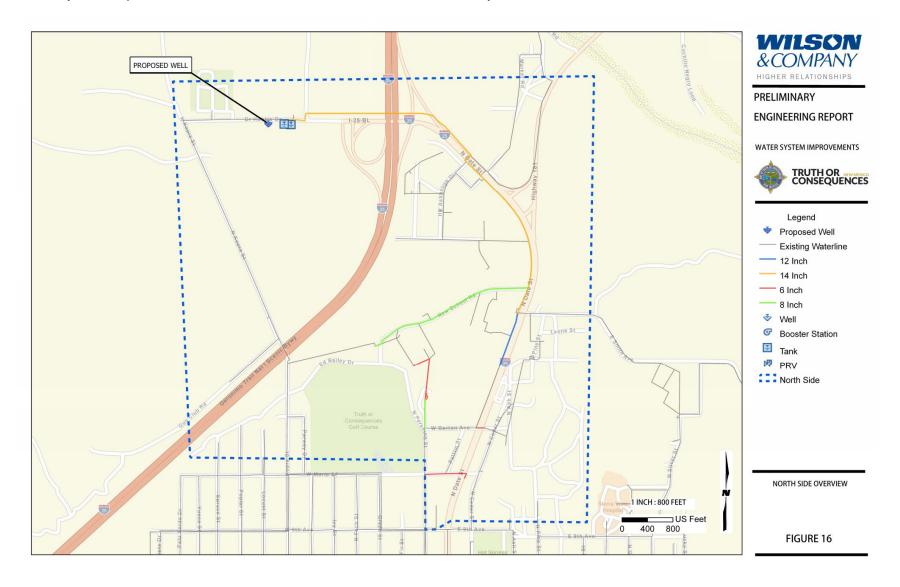


Figure 16: Alternative IV North Side



5.2.8 Alternative V: East Side Replacement

5.2.8.1 Description

Alternative V involves replacing 13.1 percent of the existing waterlines within the City, that are equal or less than 6 inch, with new PVC C-900 DR-18. This alternative will replace 23.2 percent of pipe that is over 30 years old. All waterlines in this alternative are replaced via open trench by placing the new line parallel to the existing and abandoning the existing waterline in place; except where noted otherwise. Areas of the City of Truth or Consequences were evaluated based on current GIS information. Upsizing the existing water line to a 6, and 8-inch diameter will significantly adjust available pressure in the east side of the city as well as provide for better fire flow capacity. The new water line is assumed to be installed in the shoulder of the road, with 6-12' of pavement removal, and an additional removal for any sidewalk or curb and gutter as needed. If a water meter is to be found in the existing road, it will be replaced. This portion of line repairs also includes the replacement of the existing casing and crossing pipe underneath any NMDOT ROW's via jack and bore.

The proposed water well will be located on Cemetery Road. The system currently has six wells all of them located on the southern part of the city. Additionally. The current water system uses two pumping stations to fill the water storage tanks in the north part of the City. A new well located in the north will provide reliable backup water production and prevent water outages if any of the southern wells or booster stations fail. It would also provide an additional water source when one of City's existing wells fail due to their age.

5.2.8.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" portion of this report defined as "East Side" (See **Figure 17**) should be replaced. Existing flow capacity has been determined to be insufficient due to pressure fluctuation during peak flow periods. The available fire flow does not meet NFPA fire flow requirements on multiple areas of the city. Increasing the pipe diameter within the neighborhoods is recommended at this time. All pipes are assumed to be replaced with PVC C-900 DR18 with sizes 6 Inch or greater. Dewatering required as described in the previous "Cost Evaluation Methodology" section via open trench.

5.2.8.3 Replacement of City Water Meters

Due to aging meters that may be inaccurate and/or require manually reading described in the "Existing Facilities" portion of this report, it is been recommended that all of the water meters should be replaced in their entirely. Water meters are currently older than 40 years and exceed



their expected useful life, which provides incorrect data regarding water usage, water loss percentages and has a negative impact on the City's billing system.

5.2.8.4 Construction of Water Well Northern Area

Presently there is a dependence/reliability issue from the Cook Street and Morgan Street Booster stations to provide, to the northern area of the City. Water supply redundant in the northern area of the City is non-existent making the distribution system open for failure if either of the booster stations do not work as desired. Currently the system has six wells all of them located on the southern part of the City. Most of the wells are past, or near their end of useful life. A new well located in the northern area will provide reliable water supply to the Cemetery Tanks, prevent water outages and provide a back-up supply for the rest of the City under an emergency situation.

5.2.8.5 Water and Energy Efficiency

This alternative will cost approximately \$12,000 additional per year in electricity costs due to the new well pump, the addition of a new well will reduce the cost of boosting the water from the southern part of the City to the north tanks, which could counter the O&M cost of this improvement. The amount of lost water in this portion of the system is estimated at ~ 27 Million gallons per year. This amount of water represents \$48,875 in lost revenue per year, at the rate the city charges per gallon of water.

5.2.8.6 Green Infrastructure

This alternative will reduce water losses by approximately 27 Million gallons per year due to 90 within this area reported by the city officials. This is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.

5.2.8.7 Land Requirements

Minimal additional land requirements are anticipated for the installation of the new water well. A small easement may need to be purchased by the City in order to build this water well. If the land is privately owned, and if this option is pursued the owner will need to be determined.

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.

5.2.8.8 Potential Construction Problems

The largest potential for construction problems in this alternative lies on the neighborhoods located on each side of I-25 which will require service lines crossing all lanes within an NMDOT owned road. This, will either require extensive closures, or more likely directional drilling for the



new service lines. It is assumed that drilling will be required, and a bid item for drilling has been included in the cost estimate for this portion of the alternative.

Groundwater dewatering is another large potential concern for this alternative. A large proportion of these main transmissions lines run parallel with the Rio Grande, which is expected to have a shallow groundwater table. However, existing water levels on excavation trenches cannot be quantified until further exploratory borings are conducted as part of the design effort. As explained in the "existing system" part of this report waterlines replaced within the southern-most portion of the East Side Area will require an estimated 60 percent dewatering of the construction trench. In other areas a 5 percent dewatering will be assumed.

5.2.8.9 Resiliency and Operational Simplicity

The only regular maintenance item for this alternative is the new water well house, which will require periodic maintenance as recommended by the manufacturer. The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.

5.2.8.10 Alternative Pros/Cons

ADVANTAGES:

- This option has a capital cost that is within the City's budget
- This option fixes safety issues on the East Side (in the form of leakage and lack of fire flow)
- This option improves water supply to the water system
- This option improves approximately 13 percent of the existing water system
- This option improves approximately 23 percent of the aging water system

DISADVANTAGES:

- This option requires a large amount of NMDOT crossing permits
- This option has additional O&M for the new well, but pumping cost will be countered by not boosting the water twice to this upper zone
- This option doesn't reduce a large amount of water losses
- This option doesn't reduce pressure peaks in the system that causes the water breaks
- This option doesn't prevent service outages for residents

This option has a high dewatering cost for the areas near the Rio Grande



Table 20: Alternative V Cost Summary

Alternative V- East Side	
20 Yrs O&M PW	\$11,402,777
Construction Cost	\$21,132,888
Non-Construction Cost	\$3,990,200
Total	\$36,525,865

The annual 2020 Operation and maintenance is \$714,294. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.



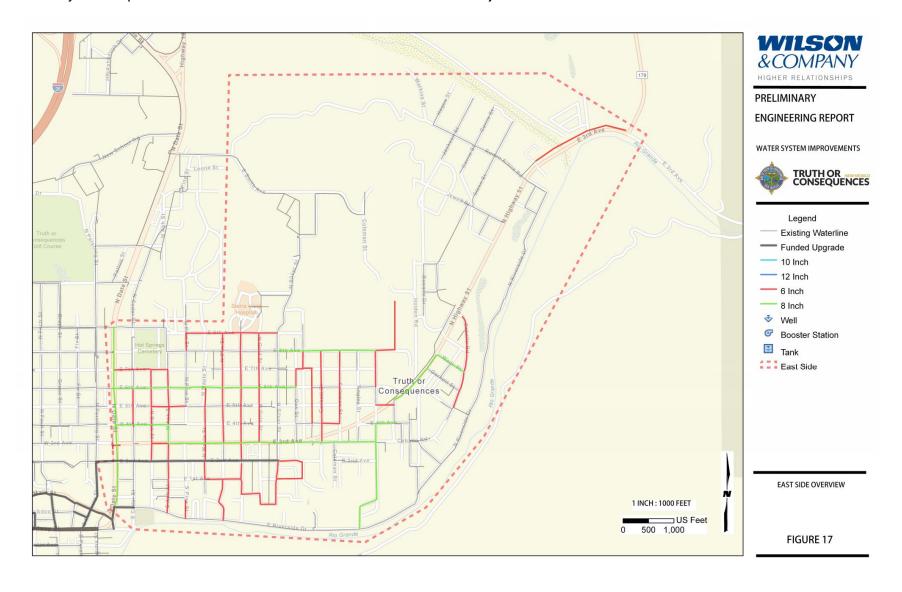


Figure 17: Alternative V East Side



5.2.9 Alternative VI: West Side Replacement

5.2.9.1 Description

Alternative VI involves replacing 7.9 percent of the existing waterlines that are 6 inches or less with PVC C-900 DR-18. This alternative will replace 14.1 percent of pipe over 30 years old. All waterlines in this alternative include pipelines segments with breakages due to high pressure fluctuations. These waterlines are planned to be replaced via open trench by placing the new line parallel to the existing waterline and then abandoning the existing waterline in place except where noted otherwise. Areas in the City of Truth or Consequences were evaluated based on current GIS information. Upsizing the existing waterline to 6, and 8 inches in diameter will significantly increase available pressure in the west of the city and provide for better fire flow capacity. The new water line is assumed to be installed in the shoulder of the road, with 6-12' of pavement removal and additional removal for any sidewalk and/or curb and gutter as needed. If an existing water meter is to be found in the existing roadway, it will be replaced. This portion of line repairs also includes the replacement of the existing casing and crossing pipe underneath any DOT ROW's via bore and jack.

The proposed water well will be located on Cemetery Road), the system currently has six wells, all of them located on the southern part of the City. Additionally, the existing water system uses two pump stations to fill the Cemetery Road water storage tanks in the north part of the City. A new well located in the north area will provide a reliable water supply and prevent water outages for this area. If any of the southern wells or booster stations fail. A northern well would also provide a redundant water supply to the rest of the City's water system.

5.2.9.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" portion of this report defined as "West Side" (See **Figure 18**) should be replaced. Existing flow capacity has been determined to be insufficient for providing pressure and required fire flow to multiple areas of the City. Increasing the pipe size within the neighborhoods is recommended at this time. All pipes are assumed to be replaced with PVC C-900 DR18 with sizes 6 Inch or great with, dewatering as described in the previous "Cost Evaluation Methodology" section via open trench.



5.2.9.3 Replacement of City Water Meters

Due to aging meters that may be inaccurate and/or require manually reading described in the "Existing Facilities" portion of this report, it is been recommended that all of the water meters should be replaced in their entirely. Water meters are currently older than 40 years and exceed their expected useful life, which provides incorrect data regarding water usage, water loss percentages and has a negative impact on the City's billing system. Construction of Water Well Northern Area

Presently there is a dependence on Cook Street and Morgan Street Booster stations to provide water to the northern area of the City. Backup water production in the northern area of the City is non-existent, making the distribution system open for failure if either of the booster stations do not work as desired. Currently the system has six wells, all of them located on the southern part of the City, most of which are near their end of useful life, a new well located in the north will provide reliable water production and prevent water outages.

Additionally, most of the water system users in this northern area are currently connected to the tanks located on Cemetery Road. This new water source, when connected to the new Cemetery tanks, will provide a reliable and consistent water supply to this area and the rest of the City if needed under an emergency situation.

5.2.9.4 Water and Energy Efficiency

This alternative will cost approximately \$12,000 additional per year in electricity costs due to the new well pump, the addition of a new well will reduce the cost of boosting the water from the southern part of the City to the north tanks, which could counter the O&M cost of this improvement. The amount of lost water in this portion of the system is estimated at ~ 16 Million gallons per year. This amount of water represents \$29,713 in lost revenue per year, at the rate the city charges per gallon of water.

5.2.9.5 Green Infrastructure

This alternative will reduce water losses by approximately 16 Million gallons per year due to 30-line breaks within this area reported by the city officials. This is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.



5.2.9.6 Land Requirements

Minimal additional land requirements are anticipated for the installation of the new water well. A small easement may need to be purchased by the City in order to build this water well. If the land is privately owned, and if this option is pursued the owner will need to be determined.

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.

5.2.9.7 Potential Construction Problems

The largest potential for construction problems in this alternative lies on the neighborhoods located on the east side of I-25 business route which will require service lines crossing all lanes within an NMDOT owned road. This will either require extensive closures, or more likely, directional drilling for the new service lines. It is assumed that horizontal directional drilling will be required, and a bid item has been included in the cost estimate for this portion of the alternative.

Groundwater dewatering is another large potential concern for this alternative. A large portion of these main transmission lines run parallel with the Rio Grande, which has a shallow groundwater table in this area. This alternative will assume 5 percent dewatering since it isn't near the Rio Grande.

5.2.9.8 Resiliency and Operational Simplicity

The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.

5.2.9.9 Alternative Pros/Cons

ADVANTAGES:

- This option has a low capital cost
- This option fixes safety issues on the West Side (in the form of leakage and lack of fire flow)
- This option minimizes the high pressures issues that caused the line breaks, since the aging infrastructure is replaced in this area
- This option improves water supply to the water system
- This option improves only approximately 8 percent of the existing water system
- This option improves approximately 14 percent of the aging water system



DISADVANTAGES:

- This option requires a large amount of NMDOT crossing permits
- This option has additional O&M for the new well, but pumping cost will be countered by not boosting the water twice to this upper zone
- This option doesn't reduce a large amount of water losses
- This option doesn't reduce pressure peaks in the system that causes the water breaks
- This option doesn't prevent service outages for residents

5.2.9.10 Cost Summary

Table 21: Alternative VI Cost Summary

Alternative VI- West Side	
20 Yrs O&M PW	\$11,660,118
Construction Cost	\$13,021,208
Non-Construction Cost	\$2,522,034
Total	\$27,203,360

The annual 2020 Operation and maintenance is \$730,414. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.





Figure 18: Alternative VI West Side



5.2.10 Alternative VII: Downtown Replacement

5.2.10.1 Description

Alternative VII involves replacing 6.8 percent of the existing waterlines within the city that are equal or less than, 6 inches with new PVC C-900 DR-18 waterlines. This alternative will replace 12 percent of pipe over 30 years old. All waterlines in this alternative are located on the most populated area of the city, with high business developments. It is planned to be replaced via open trench, placing the new line and abandoning in place the existing waterline; except where noted otherwise. Areas of the City of Truth or Consequences were evaluated based on current GIS information. Upsizing the existing waterlines to 6, 8, 10, and 12-inch diameter waterlines will significantly adjust available pressure in the Downtown area of the City as well as provide improved fire flow capacity. The new waterline is assumed to be installed in the shoulder of the road, with 6-12' of pavement removal and additional removal for any sidewalk or curb and gutter. If an existing water meter is found in the existing road, it is to be replaced. This portion of line repairs also includes the replacement of the existing casing and crossing pipe underneath any NMDOT ROW's via bore and jack.

The proposed water well will be located on Cemetery Road. The system currently has six wells, all of them located on the southern part of the City. The current water system uses two pump stations to fill the Cemetery Road water storage tanks in the northern part of the City. A new well located in the northern area will provide a reliable water supply to this area and prevent water outages. If any of the southern wells or booster stations fail, it would also provide a redundant water supply to the remainder of the City.

5.2.10.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" portion of this report defined as "Downtown" (See **Figure 19**) should be replaced. Existing flow capacity has been determined to be insufficient due to pipe pressure fluctuations during peak flow periods and not meeting the fire flow requirement in multiple areas of the City. Increasing pipe size within the neighborhoods are recommended at this time. All pipes are assumed to be replaced with PVC C-900 DR18 with sizes 6 Inch or greater. Dewatering considerations are described in the previous "Cost Evaluation Methodology" section via open trench.



5.2.10.3 Replacement of City Water Meters

Due to aging meters that may be inaccurate and/or require manually reading described in the "Existing Facilities" portion of this report, it is been recommended that all of the water meters should be replaced in their entirely. Water meters are currently older than 40 years and exceed their expected useful life, which provides incorrect data regarding water usage, water loss percentages and has a negative impact on the City's billing system. Construction of Water Well Northern

Presently there is a dependence on Cook Street and Morgan Street Booster stations to provide, a backup source of water in the northern area of the City. Backup water production in the northern area of the City is non-existent, making the distribution system open for failure if either of the booster stations do not work as desired. Currently the system has six wells all of them located on the southern part of the City, most of which are near their end of useful life. A new well located in the north will provide reliable water production and prevent water outages.

Additionally, most of the water system users in this northern area are currently connected to the northern tanks located on Cemetery Road. This new water source, when connected to the Upper tanks located on Cemetery Road, will provide a reliable back up water supply to this area and the rest of the city if needed under an emergency situation.

5.2.10.4 Water and Energy Efficiency

This alternative will cost approximately \$12,000 additional per year in electricity costs due to the new well pump, the addition of a new well will reduce the cost of boosting the water from the southern part of the City to the north tanks, which could counter the O&M cost of this improvement. The amount of lost water in this portion of the system is estimated at ~14 Million gallons per year. This amount of water represents \$25,366 in lost revenue per year, at the rate the city charges per gallon of water.

5.2.10.5 **Green Infrastructure**

This alternative will reduce water losses by approximately 14 Million gallons per year due to 35, line breaks within this area reported by the city officials. This is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.



5.2.10.6 Land Requirements

Minimal additional land requirements are anticipated for the installation of the new water well. A small easement may need to be purchased by the City in order to build this water well. If the land is privately owned, and if this option is pursued the owner will need to be determined.

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.

5.2.10.7 Potential Construction Problems

The largest potential for construction problems in this alternative lies in the neighborhoods located on each side of I-25 business route which will require service lines crossing all lanes within an NMDOT owned road. This will either require extensive closures, or more likely, directional drilling for the new service lines. It is assumed that drilling will be required, and a bid item for drilling has been included in the cost estimate for this portion of the alternative.

Groundwater dewatering is another large potential concern for this alternative. A large portion of these main transmission's lines run parallel with the Rio Grande which has a shallow groundwater table. As explained in the "existing system" part of this report waterlines replaced within the southern portion of the Downtown area will have a 60 percent dewatering of the trench. If located in other areas a 5 percent dewatering will be assumed.

5.2.10.8 Resiliency and Operational Simplicity

The only regular maintenance item for this alternative is the new water well which will require periodic maintenance as recommended by the manufacturer. The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.

5.2.10.9 Alternative Pros/Cons

<u>ADVANTAGES:</u>

- This option has a low capital cost
- This option fixes safety issues in the Downtown area (in the form of leakage and lack of fire flow)
- This option improves water supply to the water system
- This option improves approximately 7 percent of the existing water system
- This option improves approximately 12 percent of the aging water system



DISADVANTAGES:

- This option requires a large amount of NMDOT crossing permits
- This option has additional O&M for the new well, but pumping cost will be countered by not boosting the water twice to this upper zone
- This option doesn't reduce a large amount of water losses
- This option doesn't reduce pressure peaks in the system that causes the water breaks
- This option doesn't prevent service outages for residents
- This option has a high dewatering cost

5.2.10.10 **Cost Summary**

Table 22: Alternative VII Cost Summary

Alternative VII- Downtown	
20 Yrs O&M PW	\$11,719,281
Construction Cost	\$12,157,023
Non-Construction Cost	\$2,365,623
Total	\$26,241,927

The annual 2020 Operation and maintenance is \$734,120. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.



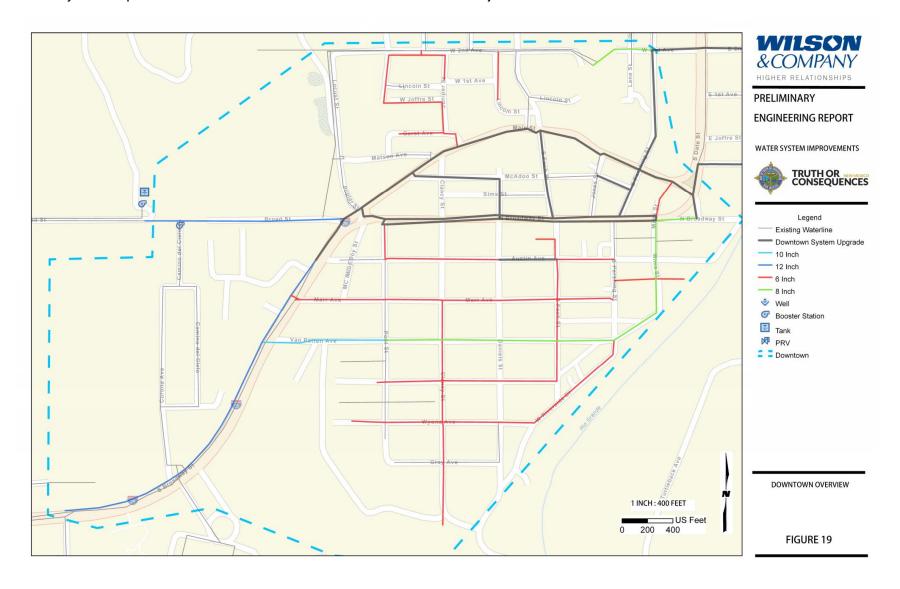


Figure 19: Alternative VII Downtown



5.2.11 Alternative VIII: Williamsburg Replacement

5.2.11.1 Description

Alternative VIII involves replacing 12.5 percent of the existing waterlines within new pipeline equal or less than 6 inches with new PVC C-900 DR-18 pipeline. This alternative will replace 22 percent of pipe over 30 years old. All waterlines in this alternative present a high-pressure breakages along record along Veater St. This alternative will be replaced via open trench by placing the new line and abandoning in place the existing waterline except where noted otherwise. Areas of the City of Truth or Consequences were evaluated based on current GIS information. Upsizing the existing water lines to be 6, 8, and 14-inch diameters will significantly adjust available pressure in the Downtown area and provide for better fire flow capacity. The new waterlines are assumed to be installed in the shoulder of the road, with 6-12' of pavement removal and an additional removal for any sidewalk or curb and gutter as needed. If an existing water meter is found in the existing road, it will need to be replaced. This portion of line repairs also includes the replacement of the existing casing and crossing pipe underneath any DOT ROW's via bore and jack.

The proposed water well will be located on Cemetery Road. The system currently has six wells all of them located on the southern part of the city. The existing water system uses two pumping stations to feed the water storage tanks on the north part of the city. A new well located in the north will provide reliable water production and prevent water outages if any of the southern wells or booster stations fail. It would also provide an additional water source when one of City's existing wells fail due to their age.

5.2.11.2 Replacement of City Water Lines

Due to extensive leaks and pipe breaks as described in the "Existing Facilities" portion of this report and defined as "Williamsburg" (See **Figure 20**) should be replaced. Existing flow capacity has been determined to generally be not sufficient per pipe pressure fluctuations during peak flow periods, as well as not meeting the fire flow requirement on multiple areas of the city. Increasing the pipe size within the neighborhoods is recommended at this time. All pipes are assumed to be replaced with PVC C-900 DR18 with sizes 6 Inch or greater via open trench, with dewatering as described in the previous "Cost Evaluation Methodology" section.

5.2.11.3 Replacement of City Water Meters

Due to aging meters that may be inaccurate and/or require manually reading described in the "Existing Facilities" portion of this report, it is been recommended that all of the water meters should be replaced in their entirely. Water meters are currently older than 40 years and exceed



their expected useful life, which provides incorrect data regarding water usage, water loss percentages and has a negative impact on the City's billing system. Construction of Water Well Northern Area.

Presently there is a dependence on Cook Street and Morgan Street Booster stations to provide, a backup source of water in the northern area of the City. Backup water production in the northern area of the City is non-existent making the distribution system open for failure if either of the booster stations do not work as desired. Currently the system has six wells all of them located on the southern part of the City, most of which are near their end of useful life, a new well located in the north will provide reliable water production and prevent water outages.

Additionally, most of the water system users in this northern area are currently connected to the northern tanks located on Cemetery Road. This new water source, when connected to the Upper tanks located on Cemetery Road, will provide a reliable backup supply to this area and the rest of the city if needed under an emergency situation.

5.2.11.4 Water and Energy Efficiency

This alternative will cost approximately \$12,000 additional per year in electricity costs due to the new well pump, the addition of a new well will reduce the cost of boosting the water from the southern part of the City to the north tanks, which could counter the O&M cost of this improvement. The amount of lost water in this portion of the system is estimated at ~ 26 Million gallons per year. This amount of water represents \$46,849 in lost revenue per year, at the rate the city charges per gallon of water.

5.2.11.5 Green Infrastructure

This alternative will reduce water losses by approximately 26 Million gallons per year due to 90-line breaks within this area reported by the city officials. This is an essential consideration in New Mexico with limited water supply available. See **Appendix 8** for justification on water loss numbers.

5.2.11.6 Land Requirements

Minimal additional land requirements are anticipated for the installation of the new water well. A small easement may need to be purchased by the City in order to build this water well. If the land is privately owned, and if this option is pursued the owner will need to be determined.

No additional land requirements are anticipated for the replacement of the water lines, as all new water lines are within existing right-of-way.



5.2.11.7 Potential Construction Problems

The largest potential for construction problems in this alternative lies on the neighborhoods located on each side of I-25 business route which will require service lines crossing all lanes within an NMDOT owned road. This will either require extensive closures, or more likely, directional drilling for the new service lines. It is assumed that drilling will be required, and a bid item for drilling has been included in the cost estimate for this portion of the alternative.

Groundwater dewatering is another large potential concern for this alternative. A large portion of these main transmission's lines run parallel with the Rio Grande which has a shallow groundwater table in this area. This alternative will assume 5 percent dewatering since it isn't near the Rio Grande.

5.2.11.8 Resiliency and Operational Simplicity

The only regular maintenance item for this alternative is the new water well pump which will require periodic maintenance as recommended by the manufacturer. The new pipelines and water meter replacements are anticipated to greatly reduce the operations costs associated with pipe repairs in this area.

5.2.11.9 Alternative Pros/Cons

ADVANTAGES:

- This option has a low capital cost
- This option fixes safety issues in the Williamsburg area (in the form of infiltration and lack of fire flow)
- This option improves the back up and redundancy to the water system
- This option improves approximately 11 percent of the water system
- This option improves approximately 22 percent of the aging water system

DISADVANTAGES:

- This option requires a large amount of NMDOT crossing permits
- This option has additional O&M for the new well, but pumping cost will be countered by not boosting the water twice to this upper zone
- This option doesn't reduce a large amount of water losses
- This option doesn't reduce pressure peaks in the system that causes the water breaks
- This option doesn't prevent service outages for residents



5.2.11.10 <u>Cost Summary</u>

Table 23: Alternative VIII Cost Summary

Alternative VIII- Williamsburg	
20 Yrs O&M PW	\$11,430,818
Construction Cost	\$18,538,532
Non-Construction Cost	\$3,520,636
Total	\$33,489,986

The annual 2020 Operation and maintenance is \$716,050. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.



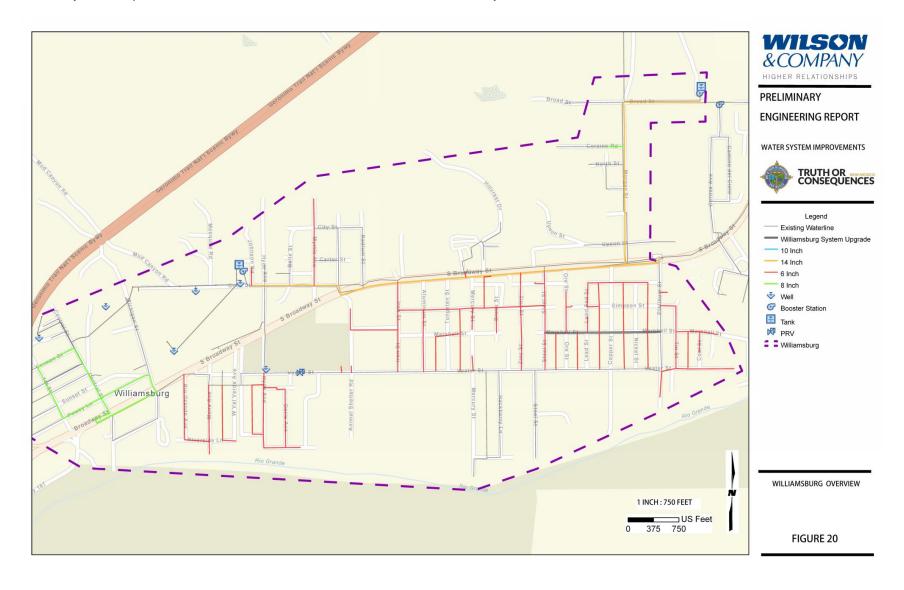


Figure 20: Alternative VIII Williamsburg



5.2.12 Alternative IX: Pressure Tank Replacement

5.2.12.1 Description

Alternative IX will consist of installing a new water system, with a building located near the existing water well which will; enclose two 200-gallon capacity pressure tanks, a chlorination system, and a control panel at the municipal airport. The existing well will include a new 8 Inch sanitary pitiless seal unit to protect the wellhead from contamination (surface water, debris, insects, vermin and other contaminants).

5.2.12.2 Design Layout Map

A map illustrating the schematic design layout of Alternative IX is shown in **Figure 21.** The new pressure tanks will be on the south-west side of the building. As show in the design layout a new waterline will be connected to the well. Isolation valves will be installed with the new inlets and outlets piping to allow the new tank to be isolated for maintenance and repairs. In addition, the chlorination system and control panels will be design and located within the building and comply with health and safety requirements.

5.2.12.3 Green Infrastructure

This alternative is not expected to have any environmental impacts. This is because the new pressure tanks will be located within the existing boundaries of the new building. Installation of this system will not add any further impact on endangered species, flood plains, wetlands, historical or archaeological sites due that existing buildings will not be affected by this upgrade.

5.2.12.4 Land Requirements

Minimal additional land requirements are anticipated for the installation of the new building and fence of 30' by 45'.

5.2.12.5 Potential Construction Problems

Construction of Alternative IX is not expected to have any significant problems. There are no known utilities in the direct vicinity of the proposed locations of the new building and its associated pipeline.

5.2.12.6 Resiliency and Operational Simplicity

Installation and future operation of the new pressure tank is anticipated to have a beneficial impact on the system because it will provide redundancy to the system and will allow for more energy efficient use of the well pump. No interruption in the delivery of chlorinated water to distribution system means there is a much smaller chance of water consumers ingesting water that has not



been properly disinfected. The new water system replacements are anticipated to greatly reduce the operations costs associate with repairs of the failing system.

5.2.12.7 Alternative Pros/Cons

ADVANTAGES:

- This option has a low capital cost
- This option fixes safety issues at the Municipal airport of no chlorination and lack of secure building for the facilities
- This option prevents service outages for the airport buildings

DISADVANTAGES:

- This option has additional O&M for the new chlorination system
- This option doesn't provide enough to back up water storage to the water system if power outage occurs
- This option only addresses a small transient population
- This option doesn't repair the small existing distribution system

5.2.12.8 Cost Summary

Table 24: Alternative IX Cost Summary

Alternative IX-Airport 1	
20 Yrs O&M PW	\$33,305
Construction Cost	\$342,862
Non-Construction Cost	\$90,328
Total	\$466,495

The annual 2020 Operation and maintenance is \$2,086. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.







PRELIMINARY

ENGINEERING REPORT

WATER SYSTEM IMPROVEMENTS



Legend

- 1.Two Room Storage Building 12'x15'
- 2.Six6 Inch Waterline
- 3. Well Sanitary Seal Pitless Unit With 8 Inch Concrete Slab
- 4. Chlorination System
- 5.Panel Control
- 6.2-200 Gallon Pressurized
- 7. 30'x45' chain-link fence
- 8. Existing Building

AIRPORT IMPROVEMENTS -PRESSURE TANK REPLACEMENT

FIGURE 21

Figure 21: Airport Alternative IX Pressure Tank Replacement



5.2.13 Alternative X: Airport Improvements – New ground water storage tank without fire flow

5.2.13.1 <u>Description</u>

Alternative X will consist of installing a new Municipal Airport water System with a new building located near the existing water well. The building will enclose a chlorination system, Variable speed booster system and a control panel. A new 7,200 gallons steel storage tank will be located near the building. The existing well will include a new 8 Inch sanitary pitiless seal unit to protect the wellhead from contamination (surface water, debris, insects, vermin and other contaminants).

5.2.13.2 Design Layout Map

A map illustrating the schematic design layout of Alternative X is shown in **Figure 22.** The new steel storage tank will be nearby the new building. As show in the design layout, a new water system will be connected to the well. Isolation valves will be installed with the new inlets and outlets piping to allow the new tank to be isolated for maintenance and repairs. In addition, chlorination system, booster system, and control panels will be design and located within the building following hazards protocols.

5.2.13.3 Green Infrastructure

This alternative is not expected to have any environmental impacts. This is because the new storage tanks will be located within the existing boundaries of the existing system. Installation of this system will not add any further impact on endangered species, flood plains, wetlands, historical or archaeological sites since the existing buildings will not be affected by this upgrade.

5.2.13.4 Land Requirements

Minimal additional land requirements are anticipated for the installation of the new storage tank, building, and fence of 30' by 80'.

5.2.13.5 Potential Construction Problems

Construction of Alternative X is not expected to have any significant problems. There are no known utilities in the direct vicinity of the proposed location of the new building and its associated pipeline.

5.2.13.6 Resiliency and Operational Simplicity

Installation and future operation of the new tank is anticipated to have a beneficial impact on the environment because it will provide a 3-day storage in the system and will allow for more energy efficient use of the new water pump. No interruption in the delivery of chlorinated water to distribution means there is a much smaller chance of water consumers ingesting water that has



not been properly disinfected. The new water system replacements is anticipated to greatly reduce the operations costs associated with the failing system.

5.2.13.7 Alternative Pros/Cons

ADVANTAGES:

- This option has a low capital cost
- This option fixes safety issues at the Municipal Airport of no chlorination and lack of secure building for the facilities
- This option improves the water storage requirement and reliability of water supply if a power outage occurs
- This option prevents service outages for the airport buildings.

DISADVANTAGES:

- This option has additional O&M for the new chlorination system, booster pumps, and storage tank
- This option only addresses a small transient population
- This option doesn't repair the small existing distribution system

5.2.13.8 Cost Summary

Table 25: Alternative X Cost Summary

Alternative X-Airport 2	
20 Yrs O&M PW	\$33,305
Construction Cost	\$447,772
Non-Construction Cost	\$107,461
Total	\$588,538

The annual 2020 Operation and maintenance is \$2,086. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.





Figure 22: Airport Alternative X New Ground Water Storage Tank without Fire Flow



5.2.14 Alternative XI: Airport Improvements- with Fire flow

5.2.14.1 Description

Alternative XI will consist of installing a new Municipal Airport water System with a new building located near the existing water well. The building will enclose a chlorination system, variable speed booster system, a 1,600 GPM fire pump, and a control panel. A new 190,000-gallon steel storage tank will be constructed near the building. The existing well will include a new 8 Inch sanitary pitiless seal unit to protect the wellhead from contamination (surface water, debris, insects, vermin and other contaminants).

5.2.14.2 Design Layout Map

A map illustrating the schematic design layout of Alternative XI is shown in **Figure 23.** The new steel storage tank will be located near the new building. As shown in the design layout, a new waterline will be connected to the outlet of the water well. Isolation valves will be installed with the new inlet and outlet piping to allow the new tank to be isolated for maintenance and repairs. The chlorination system, booster system, and control panels will be designed and located within the building complying with health and safety requirements.

5.2.14.3 Green Infrastructure

This alternative is not expected to have any environmental impacts. This is because the new storage tanks will be located within the existing boundaries of the existing system. Installation of this system will not add any further impact on endangered species, flood plains, wetlands, historical and archaeological sites due that existing buildings will not be affected by this upgrade.

5.2.14.4 **Land Requirements**

Minimal additional land requirements are anticipated for the installation of the new storage building and fence of 30' by 90'.

5.2.14.5 Potential Construction Problems

Construction of Alternative XI is not expected to have any significant problems. There are no known utilities in the direct vicinity of the proposed locations of the new building and its associated pipeline.

5.2.14.6 Resiliency and Operational Simplicity

Installation and future operation of the new tank is anticipated to have a beneficial impact on the environment because it will provide a three day redundancy in the system, comply with fire flow requirements, and will allow for more energy efficient use of the new water pump. No interruption



in the delivery of chlorinated water to distribution means there is a much smaller chance of water consumers ingesting water that has not been properly disinfected. The new water system replacements is anticipated to greatly reduce the operations costs associated with pipe repairs in this area.

5.2.14.7 Alternative Pros/Cons

ADVANTAGES:

- This option fixes safety issues at the Municipal Airport of no chlorination and lack of secure building for the facilities
- This option improves the water storage requirement and reliability of water supply if a power outage occurs
- This option complies with fire flow requirement
- This option prevents service outages for the airport buildings.
- This option repairs the small existing distribution system to meet fire flow requirements

DISADVANTAGES:

- This option has a high capital cost
- This option has additional O&M for the new chlorination system, booster pumps, fire flow pump, and storage tank
- This option only addresses a small transient population

5.2.14.8 Cost Summary

Table 26: Alternative XI Cost Summary

Alternative XI-Airport 3	
20 Yrs O&M PW	\$647,893
Construction Cost	\$1,850,550
Non-Construction Cost	\$336,534
Total	\$2,834,977

The annual 2020 Operation and maintenance is \$40,585. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.





Figure 23: Airport Alternative XI New Ground Water Storage Tank with Fire Flow



5.2.15 Alternative XII: Airport Improvements – VFD Well pump

5.2.15.1 Description

Alternative IX consist of installing a new water system with a building located near the existing water well at the municipal airport. The building will house one 30 gpm capacity pressure tank, a chlorination system, 50 gpm variable speed pumps, and a control panel. The existing well will include a new 8 Inch sanitary pitiless seal unit to protect the wellhead from contamination (surface water, debris, insects, vermin and other contaminants).

5.2.15.2 Design Layout Map

A map illustrating the schematic design layout of Alternative XII is shown in **Figure 24.** The new pressure tanks will be on the south-west side of the building. As show in the design layout a new waterline will be connected to the outlet of the water well. Isolation valves will be installed with the new inlet and outlet piping to allow the new tank to be isolated for maintenance and repairs. In addition, the chlorination system, control panels, and VFD will be design and located within the building meeting health and safety requirements.

5.2.15.3 Green Infrastructure

This alternative is not expected to have any environmental impacts. This is because the new pressure tank will be located within the existing boundaries of the new building. Installation of this system will not add any further impact on endangered species, flood plains, wetlands, historical or archaeological sites due that existing buildings will not be affected by this upgrade.

5.2.15.4 Land Requirements

Minimal additional land requirements are anticipated for the installation of the new building and fence of 30' by 45'.

5.2.15.5 Potential Construction Problems

Construction of Alternative XII is not expected to have any significant problems. There are no known utilities in the direct vicinity of the proposed locations of the new building and its associated pipeline.

5.2.15.6 Resiliency and Operational Simplicity

Installation and future operation of the new pressure tank is anticipated to have a beneficial impact on the system because it will allow for more energy efficient use of the well pump. No interruption in the delivery of chlorinated water to distribution means there is a much smaller chance of water consumers ingesting water that has not been properly disinfected. The new



water system replacements is anticipated to greatly reduce the operations costs associate repairs of the failing system.

5.2.15.7 Alternative Pros/Cons

ADVANTAGES:

- This option has a low capital cost
- This option fixes safety issues at the Municipal Airport of no chlorination and lack secure building for the facilities
- This option reduces service outages for the airport buildings

DISADVANTAGES:

- This option has additional O&M for the new chlorination system and well pump VFD
- This option doesn't provide enough to back up water storage to the water system if power outage occurs
- This option only addresses a small transient population
- This option doesn't repair the small existing distribution system

5.2.15.8 Cost Summary

Table 27: Alternative XII Cost Summary

Alternative XII -Airport 4	
20 Yrs O&M PW	\$38,021
Construction Cost	\$393,623
Non-Construction Cost	\$98,618
Total	\$530,262

The annual 2020 Operation and maintenance is \$2,382. See the breakdown of Annual Operation and Maintenance cost is provided in **Appendix 5** and full construction and non-construction cost breakdown is in **Appendix 4**.





Figure 24: Airport Alternative XII VFD Well Pump



6 SELECTION OF AN ALTERNATIVE

6.1 Life Cycle and Capital Cost Analysis

When analyzing project alternatives, different avenues for selecting the best project must be evaluated. In selecting the most feasible and functional project for the water system, two features were considered. The first is a life cycle cost analysis (LCCA) which was developed as a tool to assist asset managers with decisions solely based off monetary value. Other non-monetary factors help analyze and selecting an alternative are listed in the section below.

6.1.1 Capital & Life Cycle Cost Summary

All alternatives were evaluated on a lifecycle cost basis with estimated future maintenance, electricity, and water losses accounted for. All alternatives use a 2.25% (2 years) discount rate, to calculate future and present values. All alternatives are evaluated for a 20-year period and the total net present value for this period is calculated.

Present worth is the future value, capital and annual O&M costs, of a project for its entire operational or design life discounted to reflect its current value. It is a useful tool for comparing cash flows that don't necessarily occur at the same time. When developing the present worth of each alternative, a 2.25% Real Interest Rate was used for 20 years based on Discount Rates for Cost-Effectiveness, Lease Purchase, Related Analysis, and OMB Circular No. A-94 (US Office of Management and Budget).

The present worth of the annual O&M costs is calculated using the equation below

$$PV = A * \frac{(1+i)^n - 1}{i(1+i)^n}$$

PV: Present Value

A: Annual Cost (O&M costs)

I: Real Interest Rate: 2.25%

N: number of years: 20 years

The Net Present Value was calculated as the sum of the Capital Cost plus the present worth of the uniform series of annual O&M (USPW (O&M)).

Evaluation of the T or C water system Alternatives I through VIII, the Capital Costs for Alternative II is the highest and the Annual O&M Costs for Alternative II is the highest Alternative I has the



lowest Capital Costs and Annual O&M Costs, as one would expect resulting in a Net Present Value less than the other eight alternatives.

Alternatives I through XII presented herein are comprised of installing new facilities in addition to upgrading the existing facilities. As such the existing equipment, tanks, pumps, PRV, piping, buildings, valves, and appurtenances, will remain in service through the end of their useful life. Therefore, the salvage value for alternatives I through XII is \$0.

Table 28 : Cost Estimate Summary

	Annual O&M Present Worth 20 Yrs	Capital Cost	Net Present Value	2020 Annual O&M Cost
Alternative I- No Action	\$11,871,223	\$0	\$11,871,223	\$743,638
Alternative II- Complete System*	\$9,325,812	\$102,572,936	\$111,898,748	\$584,188
Alternative III- System Performance Update*	\$10,989,446	\$25,794,833	\$36,784,279	\$688,402
Alternative III A- System High Pressure Solutions	\$11,115,798	\$7,530,330	\$18,646,128	\$696,317
Alternative III B- System Redundancy and Hydraulic Performance Enhancements *	\$11,880,678	\$9,844,031	\$21,724,709	\$744,230
Alternative III C- Additional Hydraulic Performance Enhancements	\$11,724,099	\$6,270,445	\$17,994,544	\$734,422
Alternative IV- North Side*	\$11,914,630	\$8,872,546	\$20,787,176	\$746,357
Alternative V- East Side*	\$11,402,777	\$25,123,088	\$36,525,865	\$714,294
Alternative VI- West Side*	\$11,660,118	\$15,543,242	\$27,203,360	\$730,414
Alternative VII- Downtown*	\$11,719,281	\$14,522,646	\$26,241,927	\$734,120
Alternative VIII- Williamsburg*	\$11,430,818	\$22,059,168	\$33,489,986	\$716,050
Alternative IX-Airport 1 - Pressure tank Replacement **	\$33,305	\$433,190	\$466,495	\$2,086
Alternative X-Airport 2- Without Fire Flow**	\$33,305	\$555,233	\$588,538	\$2,086
Alternative XI-Airport 3-With Fire Flow**	\$647,893	\$2,187,084	\$2,834,977	\$40,585
Alternative XII-Airport 4-VFD Well Pump**	\$38,021	\$492,241	\$530,262	\$2,382

^{*}Alternative includes new well

6.2 Non-Monetary Factors

It is important to not only evaluate alternatives on a cost basis; all alternatives are also evaluated on a Non-Monetary basis. All alternatives are scored based on six factors, then given a total score based on the sum of all weighted factors. The Scores and Score Weights are as explained below:

6.2.1 Score Weights

• Environmental Impacts: (Score Weight: 5) This factor was given high importance, as environmental safety and water conservation is a large concern for long-term sustainability. Table 29 is a summary of the T or C water system alternatives.



^{**}Alternative 9-13 the airport system is newly acquired, so the O&M cost for these alternatives will be added to additional O&M cost to the city's O&M budget.

Table 29: Water Reduction and Cost savings

	Water Losses (G)	Monetary Losses
Alternative I - No Action	120,589,680	\$ 211,032
Alternative II - Complete System	116,489,630	\$ 203,857
Alternative III - System Performance Update	45,582,899	\$ 79,770
Alternative III A- System High Pressure Solutions	32,197,444	\$ 56,346
Alternative III B- System Redundancy and Hydraulic Performance Enhancements)	7,597,150	\$ 13,295
Alternative III C- Additional Hydraulic Performance Enhancements	6,270,663	\$ 10,974
Alternative IV - North Side	6,150,074	\$ 10,763
Alternative V - East Side	27,928,570	\$ 48,875
Alternative VI - West Side	16,979,027	\$ 29,713
Alternative VII - Downtown	14,494,879	\$ 25,366
Alternative VIII - Williamsburg	26,770,909	\$ 46,849

Operation and Maintenance: (Score Weight: 2) This factor was given a low importance, operation and maintenance cost is relatively low for distribution system infrastructure.
 Table 30 summarizes all the alternative's O&M cost.

Table 30: Operation and maintenance cost

	2020 Annual O&M Cost	2021 Annual O&M Cost	2023 Annual O&M Cost
Alternative I- No Action	\$743,638	\$760,370	\$794,971
Alternative II- Complete System	\$584,188	\$597,332	\$624,515
Alternative III- System Performance Update	\$688,402	\$703,891	\$735,922
Alternative III A- System High Pressure Solutions	\$696,317	\$711,984	\$744,383
Alternative III B- System Redundancy and Hydraulic Performance Enhancements)	\$744,230	\$760,975	\$795,605
Alternative III C- Additional Hydraulic Performance Enhancements	\$734,422	\$750,946	\$785,119
Alternative IV- North Side	\$746,357	\$763,150	\$797,878
Alternative V- East Side	\$714,294	\$730,365	\$763,601
Alternative VI- West Side	\$730,414	\$746,848	\$780,834
Alternative VII- Downtown	\$734,120	\$750,638	\$784,796
Alternative VIII- Williamsburg	\$716,050	\$732,161	\$765,479
Alternative IX-Airport 1 - Pressure tank Replacement	\$2,086	\$2,133	\$2,230
Alternative X-Airport 2- Without Fire Flow	\$2,086	\$2,133	\$2,230
Alternative XI-Airport 3-With Fire Flow	\$40,585	\$41,499	\$43,387
Alternative XII-Airport 4-VFD Well Pump	\$2,382	\$2,435	\$2,546

^{* 2023} annual O&M Cost is projected at the estimated project completion.



^{• &}lt;u>Constructability:</u> **(Score Weight: 4)** This factor was given a high score, as constructability in this area can potentially have a high construction cost for dewatering.

- <u>Capital Cost:</u> (Score Weight: 5) This factor was given a high score as securing funding is the most important step to getting a project started.
- <u>Public Safety:</u> (Score Weight: 5) This was given a high score; public safety is always a major concern.
- <u>Disruption of Service</u>: **(Score Weight: 4)** This factor was given a slightly lower score because temporary service outages can be endured. However, long term service disruption is unacceptable.

6.2.2 Alternate I: No Action

Scores for Alternate I are justified as follows:

- Environmental Impacts: (Score: 1) This alternative was given a low environmental score, as taking no action will not mitigate leaks, and make any water conservation problems worse.
- Operation and Maintenance: (Score: 2) This alternative was given a low score for ease of
 maintenance; this alternative requires a large amount of maintenance due to frequent line
 breaks in the areas which will require constant repairs.
- Constructability: (Score: 5) This alternative was given a high constructability score, as no construction is required.
- <u>Capital Cost:</u> (Score: 5) This alternative is no cost and was given a high Capital Cost score.
- <u>Public Safety:</u> (Score: 2) This alternative puts existing customers at risk of water outages
 as well as the possibility of contamination due to line breaks and was given a low public
 safety score.
- <u>Disruption of Service</u>: **(Score: 2)** This alternative puts customers at a high risk of outages and was given a low score for service disruption.

6.2.3 Alternate II: Complete Water Replacement

Scores for Alternate II are justified as follows:

- Environmental Impacts: (Score: 5) This option will reduce leaks and aid in water conservation, construction takes place in previously disturbed areas and should have minimal impact.
- Operation and Maintenance: (Score: 5) This option will greatly improve operation and maintenance (O&M) compared to the existing system by replacing the old infrastructure.



- Constructability: (Score: 1) This option is within existing right-of-way, and it is assumed that trenching dewatering will be extensive on the East area and Downtown area. Extensive geotechnical investigations are highly recommended prior to construction. four points were deducted for the difficulty of installing service lines on these previously mention areas. Points were deducted because of the longevity of the construction phase.
- <u>Capital Cost:</u> (Score: 1) This is the most expensive option and was given a low score for initial capital cost.
- <u>Public Safety:</u> **(Score: 5)** This alternative will greatly reduce the risk of contamination due to line breaks, repairs and water redundancy for the water system.
- <u>Disruption of Service</u>: **(Score: 3)** This option will greatly diminish the amount of service disruptions but will create a long temporary disruption during construction.

6.2.4 Alternate III: System Performance Upgrade

Scores for Alternate III are justified as follows:

- Environmental Impacts: (Score: 4) This option will address flow and pressure surges that
 are prevalent in the water system, it will reduce leaks and aid in water conservation.
 Construction takes place in previously disturbed areas and should have minimal impact.
 A point was deducted compared to Alternative II because it doesn't assess the aging
 waterline replacement within the neighborhoods.
- Operation and Maintenance: (Score: 4) This option will greatly improve operation and maintenance compared to the existing system. A point was deducted as since the aging infrastructure that causes the breaks is not being replaced.
- <u>Constructability:</u> **(Score: 3)** This is the most constructible option; a small percentage of service lines are to be installed where shallow groundwater table has lower probability to impact the construction.
- <u>Capital Cost:</u> **(Score: 1)** This option was given a low score due to high initial capital cost which exceeds the city's budget.
- <u>Public Safety:</u> **(Score: 4)** This alternative will greatly reduce the risk of contamination due to reduction in pressure surges which reduces line breaks and repairs of the aging infrastructure.
- <u>Disruption of Service</u>: **(Score: 4)** This option will greatly diminish the amount of service disruptions but will create a temporary disruption during construction.



6.2.5 Alternate III-A: System High Pressure Solutions

Scores for Alternate III-A are justified as follows:

- Environmental Impacts: (Score: 4) This option will address flow and pressure surges that
 are prevalent in the transmission line from Cook Street to Morgan booster Station in water
 system, it will reduce leaks and aid in water conservation. Construction takes place in
 previously disturbed areas and should have minimal impact. A point was deducted
 compared to Alternative II because it doesn't assess the aging waterline replacement
 within the neighborhoods.
- Operation and Maintenance: (Score: 3) This option will greatly improve operation and maintenance compared to the existing system. Two points were deducted as since the aging infrastructure that causes the breaks is not being replaced.
- <u>Constructability:</u> **(Score: 4)** This is the most constructible option, as a low percentage of service lines are to be installed where shallow groundwater table has lower probability to impact the construction.
- <u>Capital Cost:</u> **(Score: 4)** This option was given a relatively high score due to high initial capital cost but still the cost doesn't exceed the city's budget.
- <u>Public Safety:</u> (Score: 4) This alternative will greatly reduce the risk of contamination due
 to reduction in pressure surges which reduces line breaks and repairs of the aging
 infrastructure.
- <u>Disruption of Service</u>: **(Score: 4)** This option will greatly diminish the amount of service disruptions due to waterline breakages caused by pressure surges but will create a temporary disruption during construction.

6.2.6 Alternate III-B: System Redundancy and Hydraulic Performance Enhancements

Scores for Alternate III-B are justified as follows:

- Environmental Impacts: (Score: 4) This option will address flow and pressure surges that are prevalent in the "Williamsburg" area of the water system, it will reduce leaks and aid in water conservation. Construction takes place in previously disturbed areas and should have minimal impact. A point was deducted compared to Alternative II because it doesn't assess the aging waterline replacement within the neighborhoods.
- Operation and Maintenance: (Score: 3) This option will greatly improve operation and maintenance compared to the existing system. Two points were deducted as since the aging infrastructure that causes the breaks is not being replaced.



- Constructability: (Score: 3) This option supplements and address pressure issues in the system, two points were deducted as major service lines are to be installed where shallow groundwater table has higher probability to impact the construction.
- <u>Capital Cost:</u> **(Score: 4)** This option was given a relatively high score due to high initial capital cost but still the cost doesn't exceed the city's budget.
- <u>Public Safety:</u> **(Score: 4)** This alternative will greatly reduce the risk of contamination due to reduction in pressure surges which reduces line breaks and repairs of the aging infrastructure.
- <u>Disruption of Service</u>: **(Score: 4)** This option will greatly diminish the amount of service disruptions due to waterline breakages in the "Williamsburg" area caused by pressure surges but will create a temporary disruption during construction.

6.2.7 Alternate III-C: Additional Hydraulic Performance Enhancements

Scores for Alternate III-C are justified as follows:

- Environmental Impacts: (Score: 3) This option will address part of the flow and pressure surges that are in the water system, it will reduce leaks and aid in water conservation. Construction takes place in previously disturbed areas and should have minimal impact. Two points were deducted compared to Alternative III-A because it doesn't assess the aging waterline replacement within the neighborhoods and entails small speeded areas within the city.
- Operation and Maintenance: (Score: 4) This option will greatly improve operation and maintenance compared to the existing system. A point was deducted as since the aging infrastructure that causes the breaks is not being replaced.
- <u>Constructability:</u> **(Score: 3)** This option has a small percentage of service lines are to be installed where shallow groundwater table has lower probability to impact the construction.
- <u>Capital Cost:</u> **(Score: 4)** This option was given a relatively high score due to high initial capital cost but still the cost doesn't exceed the city's budget.
- <u>Public Safety:</u> (Score: 4) This alternative will greatly reduce the risk of contamination due
 to reduction in pressure surges which reduces line breaks and repairs of the aging
 infrastructure.
- <u>Disruption of Service:</u> **(Score: 4)** This option will greatly diminish the amount of service disruptions, but will create a temporary disruption during construction



6.2.8 Alternate IV: North Side

Scores for Alternate IV are justified as follows:

- Environmental Impacts: (Score: 2) This option will address flow, reduce leaks, and aid in water conservation. Construction takes place in previously disturbed areas and should have minimal impact. three points were deducted due to the small percentage of pipeline replacements compared to other alternatives and compared to alternative III because it doesn't assess pressure surges within the water system.
- Operation and Maintenance: (Score: 2) This option will improve a small part of the operation and maintenance compared to the existing system, but it doesn't address the remaining issues of the water system.
- <u>Constructability:</u> (Score: 5) This alternative was given a relative high score, as no service
 lines are to be installed where shallow groundwater table that lowers the probability to
 impact the constructability.
- <u>Capital Cost:</u> **(Score: 4)** This option was given a relatively high score due to low initial capital cost.
- <u>Public Safety:</u> (Score: 2) This alternative will greatly reduce the risk of contamination due
 to line breaks to the specific area but does not address rest of the existing aging
 infrastructure.
- <u>Disruption of Service</u>: **(Score: 2)** This option will reduce the amount of service disruptions but will create a temporary disruption during construction. Three points were deducted due to trench dewatering possibly extending the period of disruption while in construction.

6.2.9 Alternate V: East Side

Scores for Alternate V are justified as follows:

- Environmental Impacts: (Score: 2) This option will address flow, reduce leaks, and aid in
 water conservation. Construction takes place in previously disturbed areas and should
 have minimal impact. Three points were deducted compared to alternative III because it
 doesn't assess pressure surges within the water system.
- Operation and Maintenance: (Score: 2) This option will improve operation and maintenance compared to the existing system but doesn't address the remaining part of the water system or resolves pressure issues.



- <u>Constructability:</u> **(Score: 3)** This alternative was given a relative low score, as almost 60 percent of the service lines will require extensive trenching dewatering. Broad geotechnical investigations are highly recommended prior to construction.
- <u>Capital Cost:</u> (Score: 2) This option was given a relatively low score due to high initial capital cost.
- <u>Public Safety:</u> **(Score: 2)** This alternative will greatly reduce the risk of contamination due to line breaks, repairs, and water redundancy in the East area only.
- <u>Disruption of Service</u>: **(Score: 2)** This option will reduce the amount of service disruptions but will create a temporary disruption during construction. Three points were deducted due to trench dewatering possibly extending the period of disruption while in construction.

6.2.10 Alternate VI: West Side

Scores for Alternate VI are justified as follows:

- Environmental Impacts: (Score: 3) This option will address flow, reduce leaks, and aid in
 water conservation. Construction takes place in previously disturbed areas and should
 have minimal impact. Two points were deducted compared to alternative III because it
 doesn't assess pressure surges within the water system.
- Operation and Maintenance: (Score: 3) This option will improve operation and maintenance compared to the existing system, but it doesn't address the remaining issues of the water system.
- <u>Constructability:</u> (Score: 4) This alternative was given a relative high score, as no service
 lines are to be installed where shallow groundwater table that lowers the probability to
 impact the constructability.
- <u>Capital Cost:</u> **(Score: 3)** This option was given a medium score due to high initial capital cost, but the cost still doesn't exceed the city's budget.
- <u>Public Safety:</u> **(Score: 3)** This alternative will greatly reduce the risk of contamination due to line breaks, repairs, water redundancy, and addresses aging infrastructure in the West area only. This helps mitigate the issues with the high-pressure surges in this area.
- <u>Disruption of Service:</u> **(Score: 3)** This option will reduce the amount of service disruptions but will create a temporary disruption during construction.



6.2.11 Alternate VII: Downtown

Scores for Alternate VII are justified as follows:

- Environmental Impacts: (Score: 2) This option will address flow, reduce leaks, and aid in
 water conservation. Construction takes place in previously disturbed areas and should
 have minimal impact. Three points were deducted compared to alternative III because it
 doesn't assess pressure surges within the water system.
- Operation and Maintenance: (Score: 2) this option will improve operation and maintenance compared to the existing system, but it doesn't address the remaining issues of the water system or resolves pressure issues.
- <u>Constructability:</u> **(Score: 2)** This alternative was given a low score, as almost 90 percent of the service lines will require extensive trenching dewatering. Broad geotechnical investigations are highly recommended prior to construction.
- <u>Capital Cost:</u> **(Score: 3)** This option was given a medium score due to high initial capital cost, but the cost doesn't exceed the city's budget.
- <u>Public Safety:</u> **(Score: 3)** This alternative will greatly reduce the risk of contamination due to line breaks, repairs and water redundancy in the Downtown area. This area is one of the busiest areas of the city.
- <u>Disruption of Service</u>: **(Score: 2)** This option will reduce the amount of service disruptions but will create a temporary disruption during construction. Three points were deducted due to trench dewatering possibly extending the period of disruption during construction.

6.2.12 Alternate VIII: Williamsburg

Scores for Alternate VIII are justified as follows:

- Environmental Impacts: (Score: 3) This option will address flow reduce leaks, and aid in
 water conservation. Construction takes place in previously disturbed areas and should
 have minimal impact. Two points were deducted compared to alternative III because it
 doesn't assess pressure surges within the water system.
- Operation and Maintenance: (Score: 3) This option will improve operation and maintenance compared to the existing system, but it doesn't address the remaining part of the water system or resolves pressure issues.
- <u>Constructability:</u> (Score: 4) This alternative was given a relative high score, as no service
 lines are to be installed where shallow groundwater table that lowers the probability to
 impact the constructability.



- <u>Capital Cost:</u> **(Score: 2)** This option was given a relative low score due to high initial capital which is out of the city's budget.
- <u>Public Safety:</u> (Score: 3) This alternative will greatly reduce the risk of contamination due
 to line breaks, repairs, water redundancy, and addresses aging infrastructure in the
 Williamsburg area only. This helps mitigate the issues with the high-pressure surges in
 this area.
- <u>Disruption of Service:</u> **(Score: 3)** This option will reduce the amount of service disruptions but will create a temporary disruption during construction.

6.2.13 Alternate IX: Airport Improvements Pressure Tank Replacement

Scores for Alternate IX are justified as follows:

- <u>Environmental Impacts:</u> **(Score: 4)** This option will reduce leaks and aid in water conservation, construction takes place in a undisturbed areas owned by the city and should have small impact due to the size of required building.
- Operation and Maintenance: (Score: 3) This option will significantly improve operation
 and maintenance compared to the existing system. Two points were deducted per
 absence replacement of the existing waterlines.
- <u>Constructability:</u> **(Score: 4)** This option was given a relatively high constructability scored due to no additional land requirements are anticipated for the installation of the new building and fence.
- <u>Capital Cost:</u> **(Score: 5)** This option was given a relatively high score due to low initial capital cost (common with Alternative XIII due to similar cost).
- <u>Public Safety:</u> (Score: 2) This alternative will reduce the risk of contamination due to line breaks and repairs as well as provide constant flow. Three points were deducted compared to Alternative XI due to not complying with fire flow requirements.
 <u>Disruption of Service:</u> (Score: 2) This option will greatly reduce the amount of service

disruptions but will create a temporary disruption during construction.

6.2.14 Alternate X: Airport Improvements without Fire Flow

Scores for Alternate X are justified as follows:

• Environmental Impacts: (Score: 4) This option will reduce leaks and aid in water conservation, construction takes place in a undisturbed areas owned by the city and should have small impact due to size of the required tank and building.



- Operation and Maintenance: (Score: 4) This option will significantly improve operation
 and maintenance compared to the existing system. A point was deducted per absence
 replacement of the existing waterlines.
- <u>Constructability:</u> (Score: 4) This option was given a relatively high constructability scored
 due to no additional land requirements are anticipated for the installation of the new
 storage tank, building, and fence.
- <u>Capital Cost:</u> **(Score: 3)** This option was given a neutral score due to higher initial capital cost than alternative IX and XII.
- <u>Public Safety:</u> (Score: 3) This alternative will reduce the risk of contamination due to line breaks and repairs as well as provide constant flow. Two points were deducted compared to Alternative XI due to not complying with fire flow requirements.
 <u>Disruption of Service:</u> (Score: 4) This option will greatly diminish the amount of service disruptions but will create a temporary disruption during construction.

6.2.15 Alternate XI: Airport Improvements with Fire Flow

Scores for Alternate XI are justified as follows:

- Environmental Impacts: (Score: 4) This option will reduce leaks and aid in water conservation, construction takes place in a undisturbed areas owned by the city and should have small impact due to size of the required tank and building.
- Operation and Maintenance: (Score: 4) This option will significantly improve operation and maintenance compared to the existing system. A point was deducted due to additional maintenance of the fire pump system.
- <u>Constructability:</u> **(Score: 4)** This option was given a relatively high constructability scored due to no additional land requirements are anticipated for the installation of the new storage tank, building, and fence.
- <u>Capital Cost:</u> **(Score: 2)** This option was given a relatively low due to higher initial capital cost.
- <u>Public Safety:</u> (Score: 5) This alternative will reduce the risk of contamination due to line breaks and repairs as well as provide constant flow. This alternative complies with fire flow requirement.
 - <u>Disruption of Service:</u> **(Score: 5)** This option will greatly diminish the amount of service disruptions but will create a temporary disruption during construction.



6.2.16 Alternate XII: Airport Improvements –VFD Well Pump

Scores for Alternate XII are justified as follows:

- Environmental Impacts: (Score: 4) This option will reduce leaks and aid in water conservation, construction takes place in a undisturbed areas owned by the city and should have small impact due to the size of required building.
- Operation and Maintenance: (Score: 3) This option will significantly improve operation and maintenance compared to the existing system. Two points were deducted per absence replacement of the existing waterlines.
- <u>Constructability:</u> **(Score: 4)** This option was given a relatively high constructability scored due to no additional land requirements are anticipated for the installation of the new building and fence.
- <u>Capital Cost:</u> (Score: 5) This option was given a relatively high score due to low initial capital cost (common with Alternative IX due to similar cost).
- <u>Public Safety:</u> (Score: 1) This alternative will reduce the risk of contamination due to line breaks and repairs as well as provide constant flow. Four points were deducted compared to Alternative IX due to not complying with fire flow requirements as well as not providing redundancy.

<u>Disruption of Service:</u> **(Score: 2)** This option will greatly diminish the amount of service disruptions, but will create a temporary disruption during construction

6.2.17 Non-Monetary Evaluation

Based on Table 31 below, Alternative III is the recommended option on a non-cost basis.

Table 31: Non-Monetary Factors Scoring

	WEIGHT	WEIGHT Weighted Score														
	FACTOR	ı	II	=	IIIA	IIIB	IIIC	IV	٧	VI	VII	VIII	IX	Х	ΧI	XII
ENVIRONMENTAL IMPACTS	5	5	25	20	20	20	15	10	10	15	10	15	20	20	20	20
OPERATION & MAINTENANCE	2	4	10	8	6	6	8	4	4	6	4	6	6	8	8	6
CONSTRUCTABILITY	4	20	4	12	16	12	12	20	12	16	8	16	16	16	16	16
CAPITAL COST	5	25	5	5	20	20	20	20	10	15	15	10	25	15	10	25
PUBLIC SAFETY	5	10	25	20	20	20	20	10	10	15	15	15	10	15	25	5
DISRUPTION SERVICE	4	8	12	16	16	16	16	8	8	12	8	12	8	16	20	8
TOTAL		72	81	81	98	94	91	72	54	79	60	74	85	90	99	80

"I= No Action; II=Complete System; III= System Performance; IV=North Side; V=East Side; VI= West Side; VII= Downtown; VIII= Williamsburg; IX= Airport - Pressure Tank Replacement; X= Airport Without Fire Flow; XI= Airport With Fire flow; XI= Airport VFD Well Pump



7 PROPOSED PROJECT (RECOMMENDED ALTERNATIVE)

Based on the above analysis, the recommended alternative for construction is Alternative IIIA - System High Pressure Solutions (See **Figure 13**). This alternative scored the highest in non-cost factors, due to the greater impact on the relieving pressure fluctuation problems within the system. This will ensure appropriate water transmission from the southern area from Cook Street to the Morgan Street booster station, and address aging infrastructure on key water transmission and distribution lines.

It is recommended in the near future that Alternatives III-B and III-C become completed when funding becomes available to successfully manage and address pressure issues within the city. These two alternatives address the remaining 11.5% of the pressure issues within main transmission lines in the water system. Both alternatives will provide water redundancy in the "North" area as well as continue to help with pressure issues and improving fire flow requirements throughout the City.

7.1 Project Schedule

The below preliminary schedule is provided pending PER approval.

Table 32: Project Schedule

Milestones	Start	Finish
PER & Environmental Review & Approval	8/1/2020	9/1/2020
Funding Application & Approval	1/1/2021	5/1/2021
Engineering Services	7/1/2021	6/31/2022
Final Design Approval	7/1/2022	9/1/2022
Bidding Phase	9/1/2022	11/1/2022
Construction Phase	11/2/2022	11/2/2026
Project Closeout	11/3/2026	1/1/2026



7.2 Total Project Cost Estimate

Table 33: Alternative III A- System High Pressure Solutions

Alternative III A- System High Pressure Solution									
Open Trench Waterline									
	ITEMS LIST	QTY	UNIT COST	EXTEND COST					
	General	I	I						
1	Mob/Demob. (5% of General Cost)	LS	1	\$194,215.79	\$194,215.79				
2	Traffic Control (3.43% of General Cost)	LS	1	\$310,745.27	\$310,745.27				
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$84,289.65	\$84,289.65				
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$135,951.05	\$135,951.05				
5	Materials Testing (0.2% of General Cost)	LS	1	\$77,686.32	\$77,686.32				
6	Subsurface Utility Locating	LS	1	\$10,670.76	\$10,670.76				
7	Utility Relocation	LS	1	\$10,670.76	\$10,670.76				
8	AC Pipe Removal and Disposal	LS	1	\$7,554.90	\$7,554.90				
	Waterline								
8	6" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	5,500	\$28.78	\$158,290.00				
9	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	671	\$35.70	\$23,954.70				
10	10" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	9,797	\$36.50	\$357,590.50				
11	12" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	9,862	\$42.30	\$417,162.60				
12	14"Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF		\$50.77	\$0.00				
13	Jack and Bore w/ 18-inch Casing pipe, CIP	LF	647	\$220.00	\$142,340.00				
14	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	32	\$3,500.00	\$111,776.00				
15	6" Gate Valves w/ Valve Can, CIP	EA	19	\$935.00	\$17,765.00				
16	8" Gate Valves w/ Valve Can, CIP	EA	56	\$1,205.00	\$67,204.57				
17	10" Gate Valves w/ Valve Can, CIP	EA	54	\$2,500.00	\$135,704.65				
18	12" Gate Valves w/ Valve Can, CIP	EA	4	\$3,263.00	\$12,871.88				
19	14" Gate Valves w/ Valve Can, CIP	EA		\$4,000.00	\$0.00				
20	Furnish and Install 6"x2"" PRV Assembly (including PRV, vault, excavation, labor and all required appurtenances for a complete installation)	EA	6	\$80,000.00	\$480,000.00				
21	Pressurized waterline connections, CIP	EA	36	\$1,184.22	\$43,014.43				
22	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	12,001	\$3.00	\$36,001.69				
23	Joint Restraints, CIP	EA	510	\$77.75	\$39,667.94				



	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST		
24	1" Water Service, New single connection to existing watermain, cip. SD 2362	EA	95	\$1,329.00	\$126,255.00		
25	Water Meter Box Remove & Replace	EA	95	\$1,000.00	\$95,000.00		
26	Dewatering of Trench, CIP	LF	4,686	\$53.00	\$248,334.15		
25	Valve/Pipeline abandonment	LS	1	\$46,150.34	\$46,150.34		
26	Hydrant removal and abandonment	LS	1	\$13,524.90	\$13,524.90		
	Roadway	1					
27	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	8,394	\$42.00	\$352,562.00		
28	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP	SY	8,394	\$62.00	\$520,448.67		
29	Excavate and Dispose of Unsuitable Material, CIP	CY	12,342	\$15.00	\$185,130.00		
30	Import of Engineered Fill	CY	12,342	\$15.00	\$185,130.00		
31	Geogrid Base Roadway Reinforcement	SY	8,394	\$5.50	\$46,168.83		
32	Remove and replace Curb and Gutter @ Services, CIP	LF	380	\$25.00	\$9,500.00		
33	Remove and replace Sidewalk @ Services, CIP	CY	266	\$48.00	\$12,768.00		
				Cost Subtotal:	\$4,716,100.35		
	2-YR Inflation @ 4.	55% + Con			\$4,930,683.00		
				ngency - 10%:	\$493,068.00		
				1GRT @ 8.5%:	\$461,019.00		
	TOTAL			ance Interest:	\$323,662.00		
	ENGINEERING SERV		CONSTRU	JCTION COST:	\$6,208,432.00		
34	Bridge Loan @ 5.5%	LS	1	\$56,899.00	\$56,899.00		
35	Preliminary Engineering Report-PER	LS	1	\$35,000.00	\$35,000.00		
36	Environmental w/ Report	LS	1	\$12,000.00	\$12,000.00		
37	Additional Engineering - Data Collection*	LS	1	\$108,475.00	\$108,475.00		
38	Additional Engineering - Computer hydraulic model and calibration*	LS	1	\$60,000.00	\$60,000.00		
39	Engineering Design Services	LS	1	\$542,375.00	\$542,375.00		
40	Engineering - Bid Phase	LS	1	\$12,475.00	\$12,475.00		
41	Engineering - Construction Inspection	LS	1	\$235,270.00	\$235,270.00		
42	Engineering - Construction Management	LS	1	\$75,933.00	\$75,933.00		
Engineering Services Subtotal:							
				RT @ 7.875%:	\$89,651.00		
1			Engi	neering Total:	\$1,228,078.00		



	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST		
FINANCING SERVICES							
43	Loan Origination Fee	LS	1	\$55,470.00	\$55,470.00		
		Fina	ncing Serv	ices Subtotal:	\$55,470.00		
		Fin	ancing NN	1GRT @ 8.5%:	\$4,715.00		
Legal Services Total:							
	LEGAL SERVICES	5					
44	Legal Fees - Project Attorney	LS	1	\$10,000.00	\$10,000.00		
45	Legal Fees - Bond Counsel	LS	1	\$21,000.00	\$21,000.00		
			Legal Serv	ices Subtotal:	\$31,000.00		
Legal NMGRT @ 8.5%:							
Legal Services Total:							
			G	RAND TOTAL:	<u>\$7,530,330</u>		

7.3 Annual Operation and Maintenance Cost/Budget

See **Table 35** for the total O&M cost estimate for the selected alternative. The O&M costs have also been evaluated as compared to the overall system costs. As seen **Table 34** the impacts to the complete System's O&M costs are significantly reduced by choosing Alternative III.

Table 34: Full System O&M Cost Analysis

FULL SYSTEM ANNUAL O&M COST									
	2020		2020 2021			2023			
O&M Cost for No Action Alternative	\$	743,638	\$	760,370	\$	794,971			
O&M Cost for Selected Alternative	\$	696,317	\$	711,984	\$	744,383			
O&M Cost Net Change	-;	-\$47,321 -\$48,386		-\$50,588					
Total O&M Cost W/ Selected Alternative "System Performance Upgrade"	\$	696,317	\$	711,984	\$	744,383			

Table 35: Alternative III-A O&M Estimate



O&M Alternative III A- System High Pressure Solutions WATERLINES

Input Variables							
Discount Rate:	2.25%						
Repair Costs:	\$101,007						
Water Losses	\$28,905						
O&M	\$566,404						

Year:	1	2	3	4	5
Repair Costs:	\$103,280.07	\$105,603.87	\$107,979.96	\$110,409.50	\$112,893.72
Water Loss:	\$29,555.32	\$30,220.32	\$30,900.28	\$31,595.53	\$32,306.43
O&M	\$579,148.31	\$592,179.15	\$605,503.18	\$619,127.00	\$633,057.36
Future Value	\$711,983.70	\$728,003.34	\$744,383.41	\$761,132.04	\$778,257.51
Net Present Value:	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58

\$15,667.12

Year:	6	7	8	9	10
Repair Costs:	\$115,433.83	\$118,031.09	\$120,686.79	\$123,402.24	\$126,178.79
Water Loss:	\$33,033.33	\$33,776.58	\$34,536.55	\$35,313.62	\$36,108.18
0&M	\$647,301.15	\$661,865.43	\$676,757.40	\$691,984.44	\$707,554.09
Future Value	\$795,768.30	\$813,673.09	\$831,980.73	\$850,700.30	\$869,841.06
Net Present Value:	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58

Year:	11	12	13	14	15
Repair Costs:	\$129,017.81	\$131,920.71	\$134,888.93	\$137,923.93	\$141,027.22
Water Loss:	\$36,920.61	\$37,751.33	\$38,600.73	\$39,469.25	\$40,357.31
0&M	\$723,474.06	\$739,752.22	\$756,396.65	\$773,415.57	\$790,817.42
Future Value	\$889,412.48	\$909,424.26	\$929,886.31	\$950,808.75	\$972,201.95
Net Present Value:	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58

Year:	16	17	18	19	20
Repair Costs:	\$144,200.33	\$147,444.84	\$150,762.35	\$154,154.50	\$157,622.98
Water Loss:	\$41,265.35	\$42,193.82	\$43,143.18	\$44,113.90	\$45,106.46
O&M	\$808,610.81	\$826,804.56	\$845,407.66	\$864,429.33	\$883,878.99
Future Value	Future Value \$994,076.49 \$1,016,443.21 \$1,039,313.18 \$1,062,697.73				\$1,086,608.43
Net Present Value:	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58
Total Lifetime Maintenance Cost (20 years):					\$ 17,736,596
Total Lifetime Maintenance Cost (present value):					\$ 11,115,798

ANNUAL TOTAL O&M ALT III-A

\$696,317



7.3.1 Debt Repayment and Debt Service Reserve

The debt repayment will vary based on the loan to grant ratio that the City receives. Below a 25%/75% loan/grant ratio is assumed for the purposes of this report **Table 36.** An interest rate of 3% and a 40-year term was also assumed for the purposes of this report per USDA loan terms.

Table 36: Loan Scenarios

LOAN SCENARIOS	RATIO 25:75	RATIO 57:43
Project Cost	\$ 7,530,330	\$ 7,530,330
Estimated Loan Cost (25 % and 57%)	\$ 1,882,583	\$ 4,292,288
Estimated Interest Rate & Term	3%	3%
Estimated Annual Loan Payment	\$ 81,445	\$ 185,695
Estimated Reserve (10% Annual Payment)	\$ 8,144	\$ 18,569
Number of Connections	\$ 3,538	\$ 3,538
Estimated Annual Cost Per Connection	\$ 23.02	\$ 52.49
Estimated Montly Cost Per Connection	\$ 1.92	\$ 4.37

7.3.2 Short-Lived Asset Reserve

Short lived assets are the system assets that are expected to need replacement or frequent maintenance. Based on the information provided by the City's Asset Management Plan, the assets as shown in **Table 37** Summary table below identified to be the most likely assets in need of short-term replacement.

Table 37: Short lived Asset Summary

	Estimated Life Cycle		
	1-5 years	6-10 years	
Subtotal of Short-Lived Assets (per period)	\$ 97,530.00	\$ 1,628,407.06	
Subtotal of Short-Lived Assets (per year)	\$ 19,506.00	\$ 162,840.71	
Subtotal of Short-Lived Assets (per month)	\$ 1,625.50	\$ 13,570.06	
Total of Short-Lived Assets (1-10 years)	\$	1,725,937	
Total Annual Reserve Deposit, Short-Lived Assets (1-10 years, per year)	\$	182,347	
Total Monthly Reserve Deposit, Short-Lived Assets (1-10 years, per month)	\$	15,196	



8 CONCLUSIONS AND RECOMMENDATIONS

The city of Truth or Consequences currently presents a significant amount of water losses due to pipe breakage as well as a substantial yearly expense for wasted water. The system has components dating as far back as the 1930's to 1940's, as no significant work has been done in the area. Based upon the analysis conducted in this PER and following funding agencies guidelines, it is recommended that the City of Truth or Consequences immediately pursue funding for the Alternative III-A System High Pressure Solution. It will mitigate significant pressure issues in the city and ensure proper water transmission between booster stations. The recommended alternative has accounted for the capital costs required, the ease of maintenance, public safety, and environmental considerations. Alternative III-A has a relatively a lower capital cost compared to some of the alternatives, but it is still recommended due to the greater benefit to the public, as well as ease of maintenance and O&M cost. The "System High Pressure Solution" it is supplemented by two more projects which entails both alternatives III-B and III-C to obtain a progressed benefit from each project without disrupting the performance of the water system as well as obtain the best outcomes for the benefit of the community.

Although Alternative XI Airport Improvements with fire flow is the most viable alternative to upgrade the airport water system since it provides fire flow in the area as well as a three day water storage backup for the system, it is not recommended that the City of Truth or Consequences pursue funding for this section of the project. It doesn't do any benefit to the City's community and it doesn't affect the City's water system performance.



APPENDIX 1- ENVIRONMENTAL RESOURCES

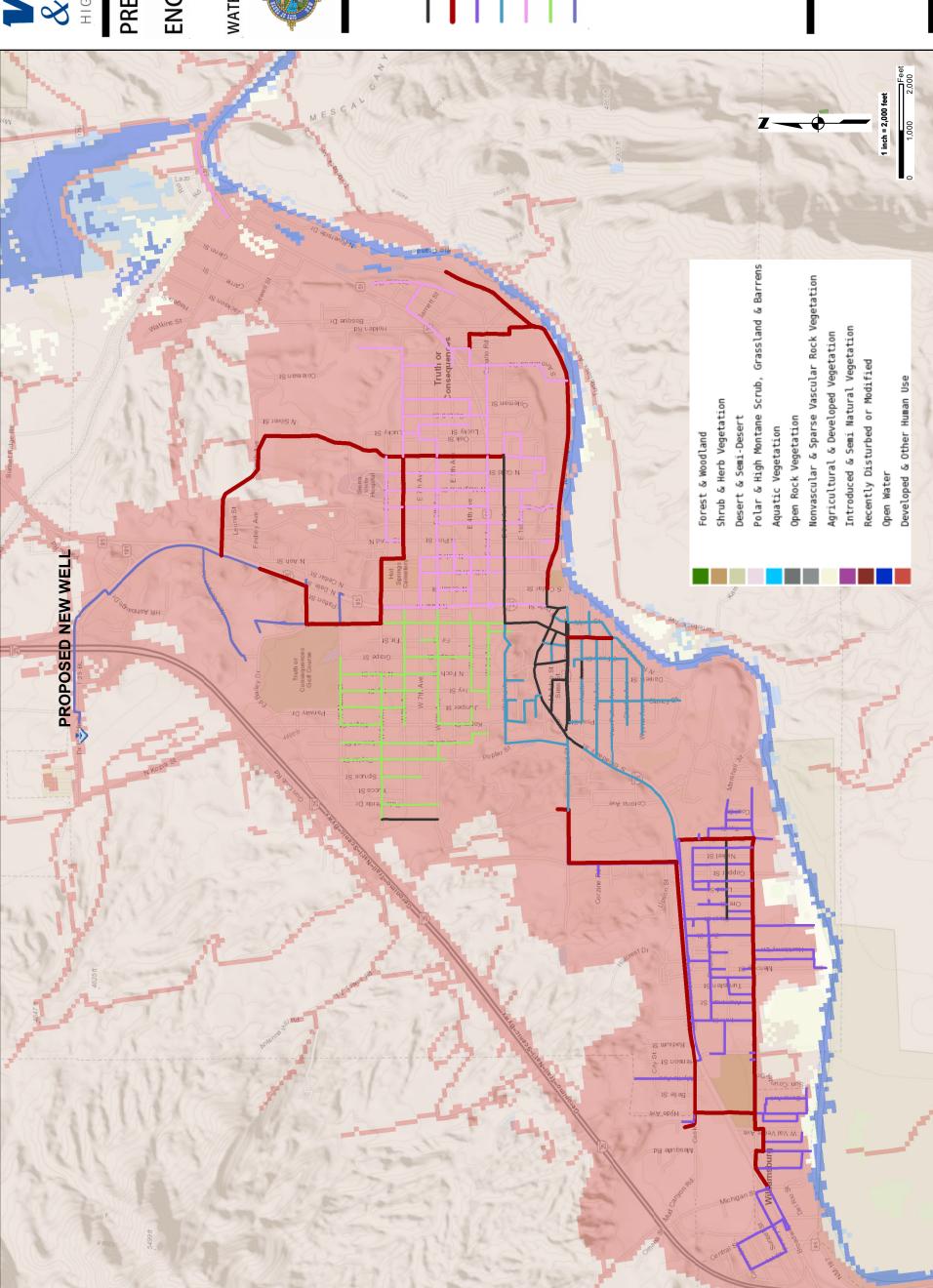
TABLE OF CONTENT:

- o EXHIBIT 101: CITY OF T OR C LAND COVERAGE
- o EXHIBIT 102: CITY OF T OR C LAND COVERAGE AIRPORT
- o EXHIBIT 103: FOREST LOCATION
- o EXHIBIT 104: HISTORICAL PLACES
- o EXHIBIT 105: FLOOD HAZARDS
- o EXHIBIT 106: WETLANDS
- SPECIES OF GREATEST CONSERVATION NEED AND FEDERAL OR STAT THREATENED/ENDANGERED – SIERRA COUNTY
- o IPAC RESOURCE LIST
- SOIL MAP



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PRELIMINARY

ENGINEERING REPORT

WATER SYSTEM IMPROVEMENTS



TRUTH OR MENCES CONSEQUENCES

Legend

- CURRENT

System Performance UpgradeWilliamsburg

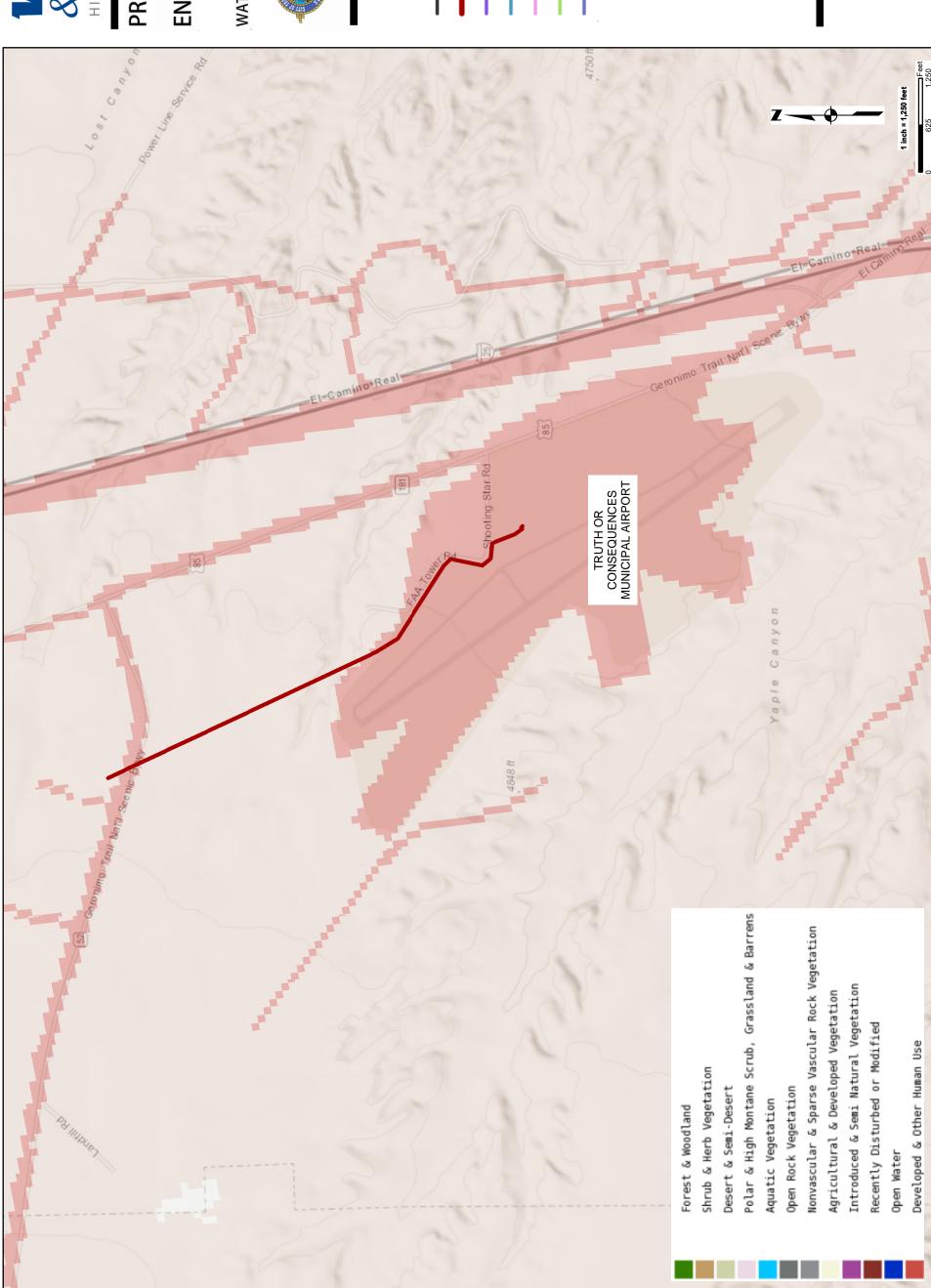
Downtown

East Side

— West Side

North Side

CITY OF T OR C LAND COVERAGE





PRELIMINARY

ENGINEERING REPORT

WATER SYSTEM IMPROVEMENTS



TRUTH OR MENTERS CONSEQUENCES

Legend

·CURRENT

System Performance Upgrade

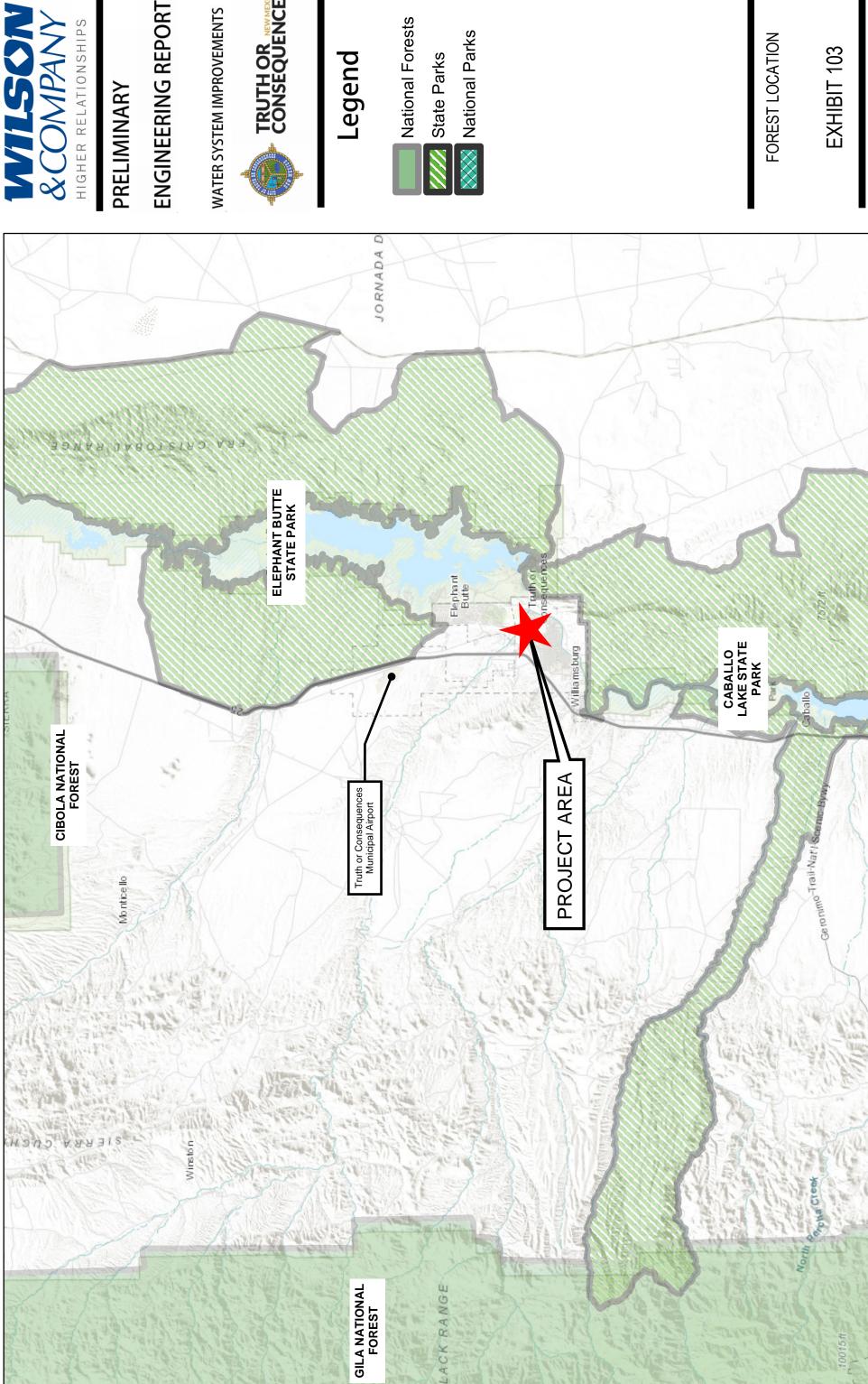
- Williamsburg

Downtown

East Side

West SideNorth Side

T OR C AIRPORT LAND COVERAGE





PRELIMINARY



TRUTH OR CONSEQUENCES

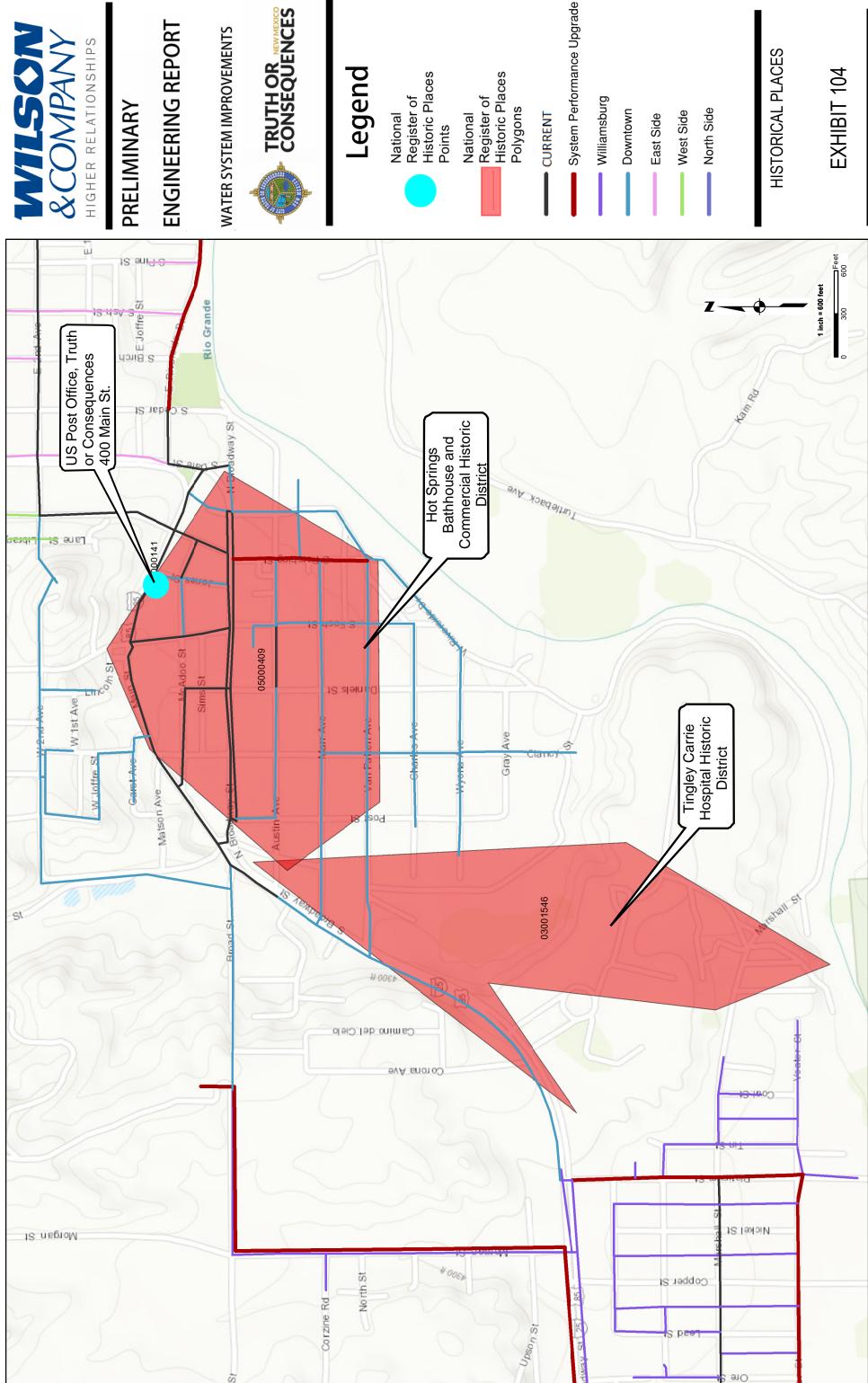
Legend



State Parks

National Parks

FOREST LOCATION



*COMPANY HIGHER RELATIONSHIPS

WATER SYSTEM IMPROVEMENTS

TRUTH OR NEWNESS CONSEQUENCES

Legend

National Register of Historic Places Points

National Register of Historic Places

Polygons

CURRENT

Williamsburg

Downtown

East Side

North Side

HISTORICAL PLACES



HIGHER RELATIONSHIPS

ENGINEERING REPORT PRELIMINARY

WATER SYSTEM IMPROVEMENTS









Legend















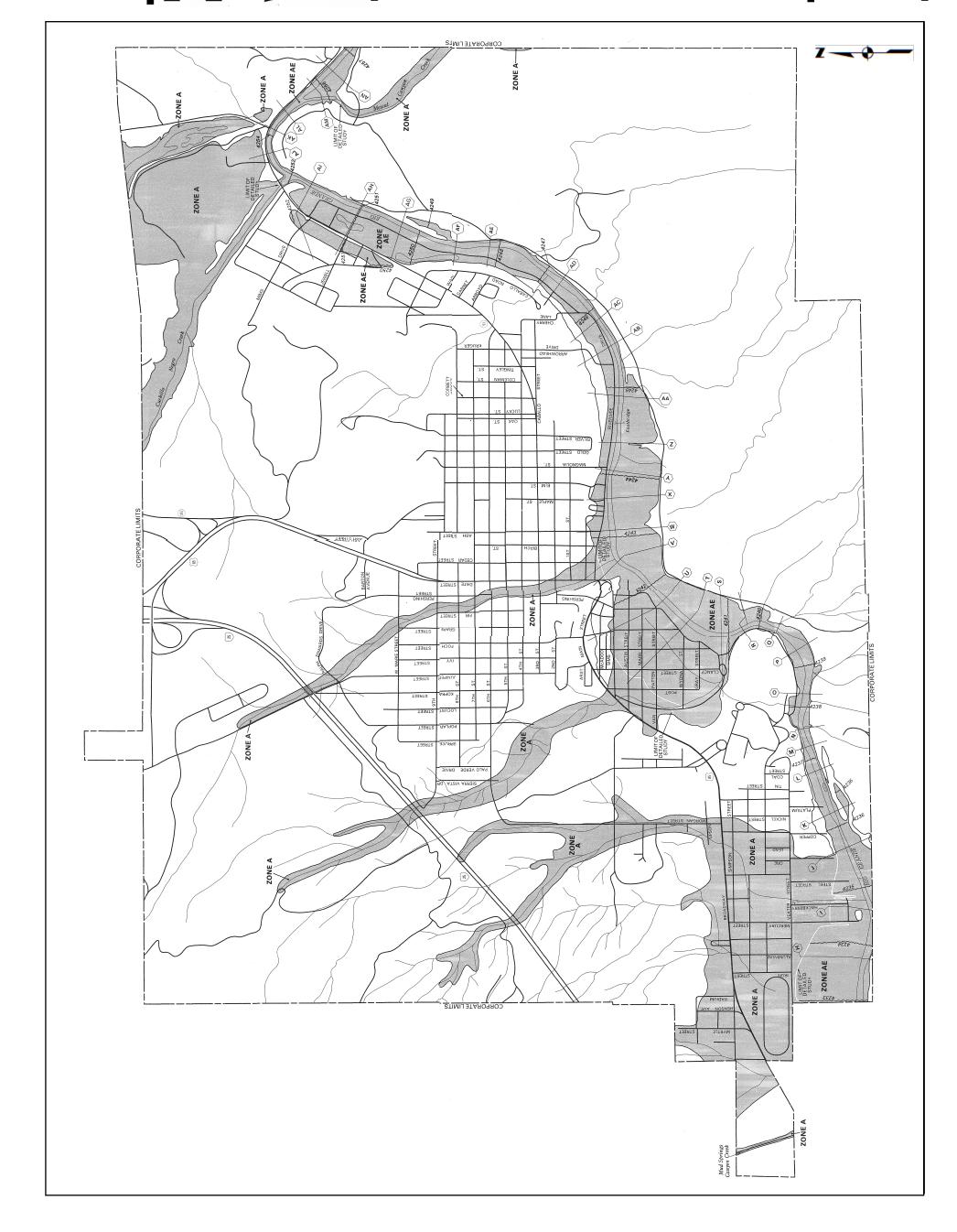


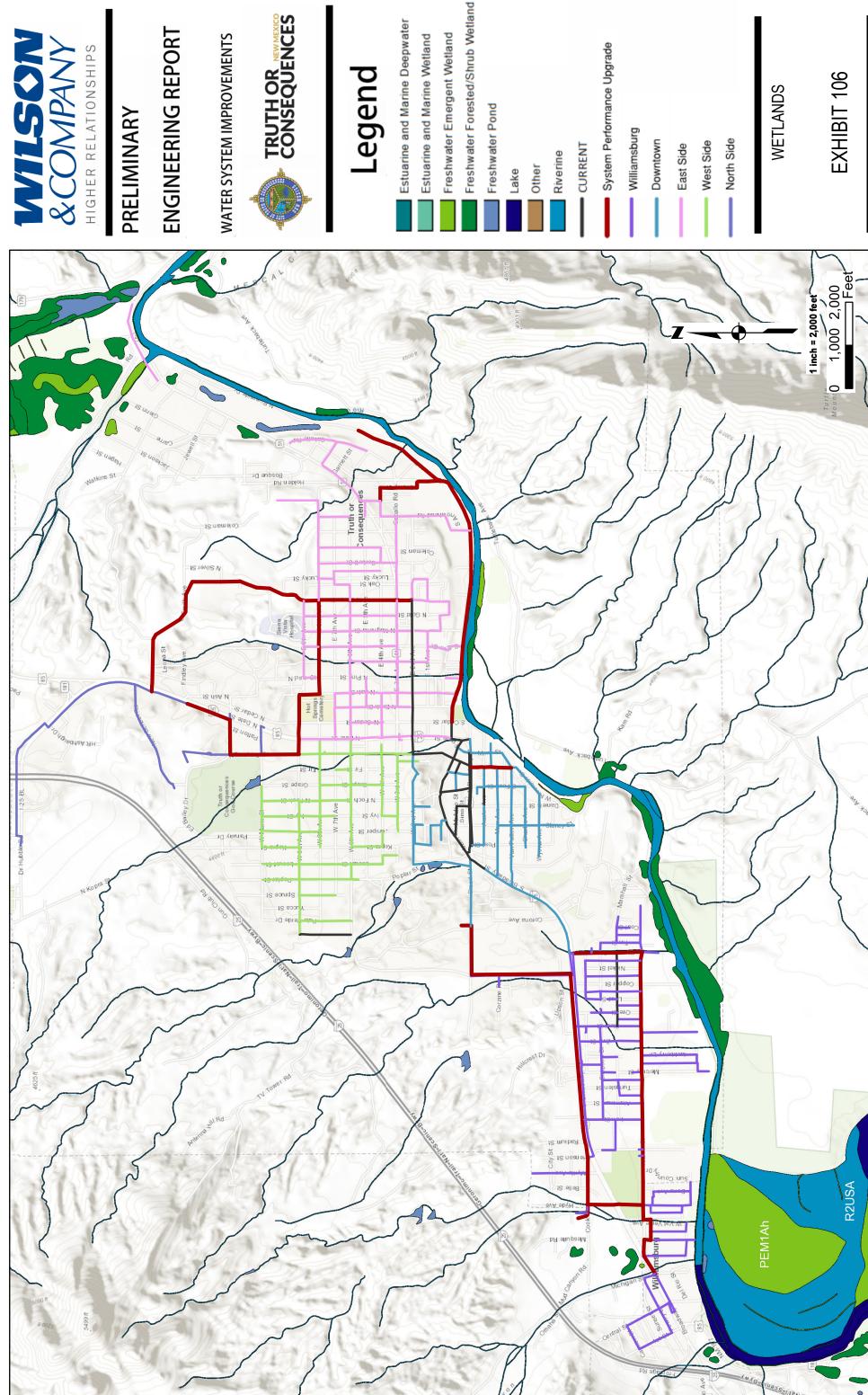






FLOOD HAZARDS







ENGINEERING REPORT

WATER SYSTEM IMPROVEMENTS

Legend

Estuarine and Marine Deepwater Estuarine and Marine Wetland

Freshwater Emergent Wetland

Freshwater Pond

WETLANDS

SPECIES OF GREATEST CONSERVATION NEED AND FEDERAL OR STATE THREATENED/ENDANGERED – SIERRA COUNTY







Species of Greatest Conservation Need and Federal or State Threatened/Endangered Sierra

<u>Taxonomic Group</u>	<u>#Species</u>	<u>TaxonomicGroup</u>	<u>#Species</u>
Amphibians	1	Birds	19
Fish	2	Mammals	2
Molluscs	1		

TOTAL SPECIES: 25

Common Name	<u>Scientific Name</u>	<u>NMGF</u>	<u>USFWS</u>	Critical <u>Habitat</u>	SGON	<u>Photo</u>
Mexican Gray Wolf	Canis lupus baileyi	E	E		Υ	<u>View</u>
Penasco Least Chipmunk	Neotamias minimus atristriatus	Е	С		Υ	<u>View</u>
Common Ground Dove	Columbina passerina	E			Υ	<u>View</u>
Yellow-billed Cuckoo (western pop)	Coccyzus americanus occidentalis		T		Υ	<u>View</u>
<u>Lucifer Hummingbird</u>	Calothorax lucifer	T			Υ	<u>View</u>
Costa's Hummingbird	Calypte costae	T			Υ	<u>View</u>
Broad-billed Hummingbird	Cynanthus latirostris	T			Υ	<u>View</u>
<u>Least Tern</u>	Sternula antillarum	E	E		Υ	<u>View</u>
Neotropic Cormorant	Phalacrocorax brasilianus	T			Υ	<u>View</u>
Bald Eagle	Haliaeetus leucocephalus	T			Υ	<u>View</u>
Common Black Hawk	Buteogallus anthracinus	T			Υ	<u>View</u>
Mexican Spotted Owl	Strix occidentalis lucida		T	Υ	Υ	<u>View</u>
Elegant Trogon	Trogon elegans	E			Υ	<u>View</u>
Aplomado Falcon	Falco femoralis	E	E		Υ	<u>View</u>
Peregrine Falcon	Falco peregrinus	T			Υ	<u>View</u>
Thick-billed Kingbird	Tyrannus crassirostris	E			Υ	<u>View</u>
Southwestern Willow Flycatcher	Empidonax traillii extimus	E	Е	Υ	Υ	<u>View</u>
Bell's Vireo	Vireo bellii	T			Υ	<u>View</u>
<u>Gray Vireo</u>	Vireo vicinior	T			Υ	<u>View</u>
Baird's Sparrow	Centronyx bairdii	T			Υ	<u>View</u>
Varied Bunting	Passerina versicolor	T			Υ	<u>View</u>
Chiricahua Leopard Frog	Lithobates chiricahuensis		T	Υ	Υ	<u>View</u>
<u>Gila Trout</u>	Oncorhynchus gilae	T	Т		Υ	<u>View</u>
White Sands Pupfish	Cyprinodon tularosa	T			Υ	No Photo

Species of Greatest Conservation Need and Federal or State Threatened/Endangered Sierra

Common Name	<u>ScientificName</u>	<u>NMGF</u>	<u>USFWS</u>	Critical <u>Habitat</u>	<u>SGCN</u>	<u>Photo</u>
Mineral Creek Mountainsnail	Oreohelix pilsbryi	T			Υ	No Photo

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Project No. 19-600-211-00

IPAC RESOURCE LIST



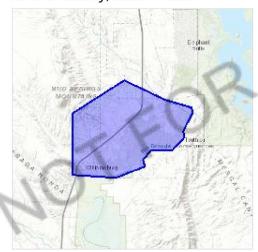
IPaC resource list

This report is an automatically generated list of species and other resources such as critical habitat (collectively referred to as *trust resources*) under the U.S. Fish and Wildlife Service's (USFWS) jurisdiction that are known or expected to be on or near the project area referenced below. The list may also include trust resources that occur outside of the project area, but that could potentially be directly or indirectly affected by activities in the project area. However, determining the likelihood and extent of effects a project may have on trust resources typically requires gathering additional site-specific (e.g., vegetation/species surveys) and project-specific (e.g., magnitude and timing of proposed activities) information.

Below is a summary of the project information you provided and contact information for the USFWS office(s) with jurisdiction in the defined project area. Please read the introduction to each section that follows (Endangered Species, Migratory Birds, USFWS Facilities, and NWI Wetlands) for additional information applicable to the trust resources addressed in that section.

Location

Sierra County, New Mexico



Local office

New Mexico Ecological Services Field Office

(505) 346-2525

(505) 346-2542

2105 Osuna Road Ne Albuquerque, NM 87113-1001

http://www.fws.gov/southwest/es/NewMexico/ http://www.fws.gov/southwest/es/ES_Lists_Main2.html

Endangered species

This resource list is for informational purposes only and does not constitute an analysis of project level impacts.

The primary information used to generate this list is the known or expected range of each species. Additional areas of influence (AOI) for species are also considered. An AOI includes areas outside of the species range if the species could be indirectly affected by activities in that area (e.g., placing a dam upstream of a fish population, even if that fish does not occur at the dam site, may indirectly impact the species by reducing or eliminating water flow downstream). Because species can move, and site conditions can change, the species on this list are not guaranteed to be found on or near the project area. To fully determine any potential effects to species, additional site-specific and project-specific information is often required.

Section 7 of the Endangered Species Act **requires** Federal agencies to "request of the Secretary information whether any species which is listed or proposed to be listed may be present in the area of such proposed action" for any project that is conducted, permitted, funded, or licensed by any Federal agency. A letter from the local office and a species list which fulfills this requirement can **only** be obtained by requesting an official species list from either the Regulatory Review section in IPaC (see directions below) or from the local field office directly.

For project evaluations that require USFWS concurrence/review, please return to the IPaC website and request an official species list by doing the following:

- 1. Draw the project location and click CONTINUE.
- 2. Click DEFINE PROJECT.
- 3. Log in (if directed to do so).
- 4. Provide a name and description for your project.
- 5. Click REQUEST SPECIES LIST.

Listed species¹ and their critical habitats are managed by the <u>Ecological Services Program</u> of the U.S. Fish and Wildlife Service (USFWS) and the fisheries division of the National Oceanic and Atmospheric Administration (NOAA Fisheries²).

Species and critical habitats under the sole responsibility of NOAA Fisheries are **not** shown on this list. Please contact <u>NOAA Fisheries</u> for <u>species under their jurisdiction</u>.

- 1. Species listed under the <u>Endangered Species Act</u> are threatened or endangered; IPaC also shows species that are candidates, or proposed, for listing. See the <u>listing status page</u> for more information.
- 2. <u>NOAA Fisheries</u>, also known as the National Marine Fisheries Service (NMFS), is an office of the National Oceanic and Atmospheric Administration within the Department of Commerce.

The following species are potentially affected by activities in this location:

Mammals

NAME STATUS

6/11/2020

Mexican Wolf Canis lupus baileyi

No critical habitat has been designated for this species.

https://ecos.fws.gov/ecp/species/3916

EXPN

Birds

NAME STATUS

Mexican Spotted Owl Strix occidentalis lucida

There is **final** critical habitat for this species. Your location is outside the critical habitat.

https://ecos.fws.gov/ecp/species/8196

Threatened

Northern Aplomado Falcon Falco femoralis septentrionalis

No critical habitat has been designated for this species.

https://ecos.fws.gov/ecp/species/1923

EXPN

Southwestern Willow Flycatcher Empidonax traillii extimus

There is **final** critical habitat for this species. Your location is outside the critical habitat.

https://ecos.fws.gov/ecp/species/6749

Endangered

Yellow-billed Cuckoo Coccyzus americanus

There is **proposed** critical habitat for this species. Your location is outside the critical habitat.

https://ecos.fws.gov/ecp/species/3911

Threatened

Reptiles

NAME STATUS

Narrow-headed Gartersnake Thamnophis rufipunctatus
There is proposed critical habitat for this species. Your location is

outside the critical habitat. https://ecos.fws.gov/ecp/species/2204

Threatened

Amphibians

NAME STATUS

Chiricahua Leopard Frog Rana chiricahuensis

There is **final** critical habitat for this species. Your location is outside the critical habitat.

https://ecos.fws.gov/ecp/species/1516

Threatened

Fishes

NAME STATUS

Gila Trout Oncorhynchus gilae

Threatened

No critical habitat has been designated for this species.

https://ecos.fws.gov/ecp/species/781

Rio Grande Silvery Minnow Hybognathus amarus

Endangered

There is **final** critical habitat for this species. Your location is outside the critical habitat.

https://ecos.fws.gov/ecp/species/1391

Flowering Plants

NAME STATUS

Todsen's Pennyroyal Hedeoma todsenii

Endangered

There is **final** critical habitat for this species. Your location is outside the critical habitat.

https://ecos.fws.gov/ecp/species/1081

Critical habitats

Potential effects to critical habitat(s) in this location must be analyzed along with the endangered species themselves.

THERE ARE NO CRITICAL HABITATS AT THIS LOCATION.

Migratory birds

Certain birds are protected under the Migratory Bird Treaty Act^{1} and the Bald and Golden Eagle Protection Act^{2} .

Any person or organization who plans or conducts activities that may result in impacts to migratory birds, eagles, and their habitats should follow appropriate regulations and consider implementing appropriate conservation measures, as described <u>below</u>.

- 1. The Migratory Birds Treaty Act of 1918.
- 2. The Bald and Golden Eagle Protection Act of 1940.

Additional information can be found using the following links:

- Birds of Conservation Concern http://www.fws.gov/birds/management/managed-species/birds-of-conservation-concern.php
- Measures for avoiding and minimizing impacts to birds
 http://www.fws.gov/birds/management/project-assessment-tools-and-guidance/conservation-measures.php
- Nationwide conservation measures for birds
 http://www.fws.gov/migratorybirds/pdf/management/nationwidestandardconservationmeasures.pdf

The birds listed below are birds of particular concern either because they occur on the <u>USFWS Birds of Conservation Concern</u> (BCC) list or warrant special attention in your project location. To learn more about the levels of concern for birds on your list and how this list is generated, see the FAQ <u>below</u>. This is not a list of every bird you may find in this location, nor a guarantee that every bird on this list will be found in your project area. To see exact locations of where birders and the general public have sighted birds in and around your project area, visit the <u>E-bird data mapping tool</u> (Tip: enter your location, desired date range and a species on your list). For projects that occur off the Atlantic Coast, additional maps and models detailing the relative occurrence and abundance of bird species on your list are available. Links to additional information about Atlantic Coast birds, and other important information about your migratory bird list, including how to properly interpret and use your migratory bird report, can be found <u>below</u>.

For guidance on when to schedule activities or implement avoidance and minimization measures to reduce impacts to migratory birds on your list, click on the PROBABILITY OF PRESENCE SUMMARY at the top of your list to see when these birds are most likely to be present and breeding in your project area.

NAME

BREEDING SEASON (IF A
BREEDING SEASON IS INDICATED
FOR A BIRD ON YOUR LIST, THE
BIRD MAY BREED IN YOUR
PROJECT AREA SOMETIME WITHIN
THE TIMEFRAME SPECIFIED,
WHICH IS A VERY LIBERAL
ESTIMATE OF THE DATES INSIDE
WHICH THE BIRD BREEDS
ACROSS ITS ENTIRE RANGE.
"BREEDS ELSEWHERE" INDICATES
THAT THE BIRD DOES NOT LIKELY
BREED IN YOUR PROJECT AREA.)

Bald Eagle Haliaeetus leucocephalus

This is not a Bird of Conservation Concern (BCC) in this area, but warrants attention because of the Eagle Act or for potential susceptibilities in offshore areas from certain types of development or activities.

https://ecos.fws.gov/ecp/species/1626

Breeds Oct 15 to Jul 31

Black Throated Sparrow Amphispiza bilineata

This is a Bird of Conservation Concern (BCC) only in particular Bird Conservation Regions (BCRs) in the continental USA

Breeds Mar 15 to Sep 5

Black-chinned Sparrow Spizella atrogularis

This is a Bird of Conservation Concern (BCC) throughout its range in the continental USA and Alaska.

https://ecos.fws.gov/ecp/species/9447

Breeds Apr 15 to Jul 31

Chestnut-collared Longspur Calcarius ornatus

This is a Bird of Conservation Concern (BCC) throughout its range in the continental USA and Alaska.

Breeds elsewhere

Common Black-hawk Buteogallus anthracinus

This is a Bird of Conservation Concern (BCC) only in particular Bird Conservation Regions (BCRs) in the continental USA

Breeds Apr 1 to Sep 20

Grace's Warbler Dendroica graciae

This is a Bird of Conservation Concern (BCC) only in particular Bird Conservation Regions (BCRs) in the continental USA

Breeds May 20 to Jul 20

Lark Bunting Calamospiza melanocorys

This is a Bird of Conservation Concern (BCC) only in particular Bird Conservation Regions (BCRs) in the continental USA

Breeds elsewhere

Virginia's Warbler Vermivora virginiae

This is a Bird of Conservation Concern (BCC) throughout its range in the continental USA and Alaska.

https://ecos.fws.gov/ecp/species/9441

Breeds May 1 to Jul 31

Probability of Presence Summary

The graphs below provide our best understanding of when birds of concern are most likely to be present in your project area. This information can be used to tailor and schedule your project activities to avoid or minimize impacts to birds. Please make sure you read and understand the FAQ "Proper Interpretation and Use of Your Migratory Bird Report" before using or attempting to interpret this report.

Probability of Presence (■)

Each green bar represents the bird's relative probability of presence in the 10km grid cell(s) your project overlaps during a particular week of the year. (A year is represented as 12 4-week months.) A taller bar indicates a higher probability of species presence. The survey effort (see below) can be used to establish a level of confidence in the presence score. One can have higher confidence in the presence score if the corresponding survey effort is also high.

How is the probability of presence score calculated? The calculation is done in three steps:

- 1. The probability of presence for each week is calculated as the number of survey events in the week where the species was detected divided by the total number of survey events for that week. For example, if in week 12 there were 20 survey events and the Spotted Towhee was found in 5 of them, the probability of presence of the Spotted Towhee in week 12 is 0.25.
- 2. To properly present the pattern of presence across the year, the relative probability of presence is calculated. This is the probability of presence divided by the maximum probability of presence across all weeks. For example, imagine the probability of presence in week 20 for the Spotted Towhee is 0.05, and that the probability of presence at week 12 (0.25) is the maximum of any week of the year. The relative probability of presence on week 12 is 0.25/0.25 = 1; at week 20 it is 0.05/0.25 = 0.2.

3. The relative probability of presence calculated in the previous step undergoes a statistical conversion so that all possible values fall between 0 and 10, inclusive. This is the probability of presence score.

To see a bar's probability of presence score, simply hover your mouse cursor over the bar.

Breeding Season (

Yellow bars denote a very liberal estimate of the time-frame inside which the bird breeds across its entire range. If there are no yellow bars shown for a bird, it does not breed in your project area.

Survey Effort (1)

Vertical black lines superimposed on probability of presence bars indicate the number of surveys performed for that species in the 10km grid cell(s) your project area overlaps. The number of surveys is expressed as a range, for example, 33 to 64 surveys.

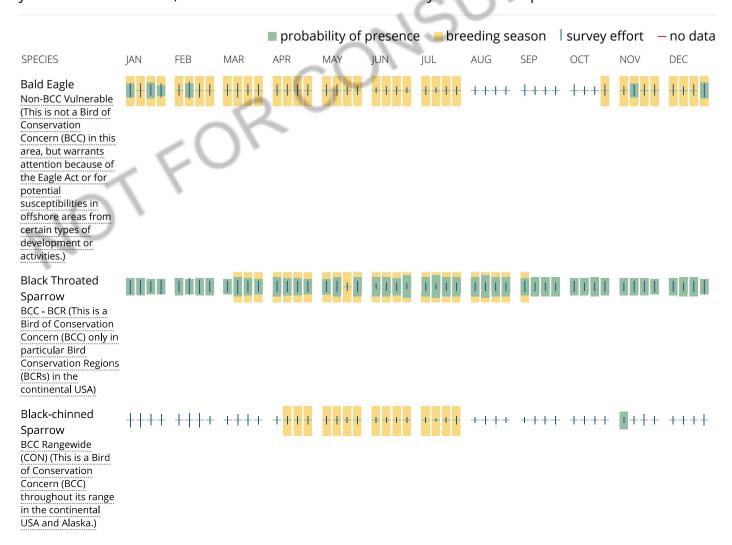
To see a bar's survey effort range, simply hover your mouse cursor over the bar.

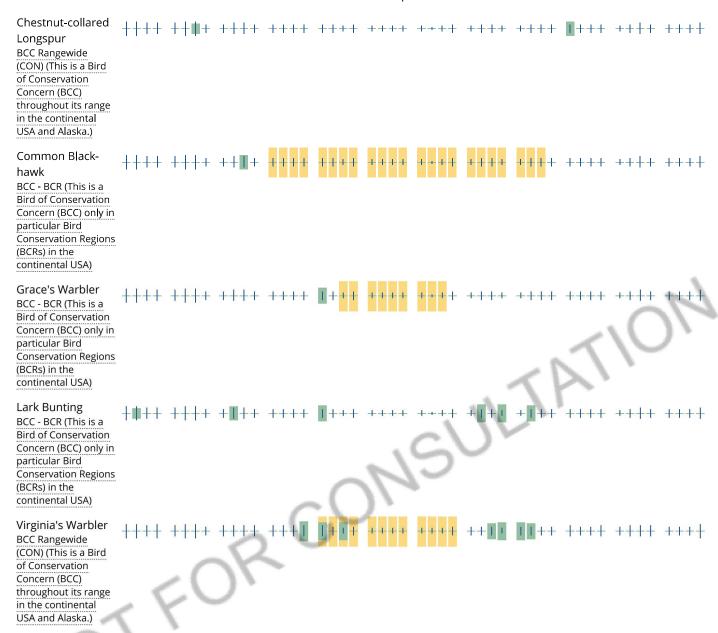
No Data (-)

A week is marked as having no data if there were no survey events for that week.

Survey Timeframe

Surveys from only the last 10 years are used in order to ensure delivery of currently relevant information. The exception to this is areas off the Atlantic coast, where bird returns are based on all years of available data, since data in these areas is currently much more sparse.





Tell me more about conservation measures I can implement to avoid or minimize impacts to migratory birds.

Nationwide Conservation Measures describes measures that can help avoid and minimize impacts to all birds at any location year round. Implementation of these measures is particularly important when birds are most likely to occur in the project area. When birds may be breeding in the area, identifying the locations of any active nests and avoiding their destruction is a very helpful impact minimization measure. To see when birds are most likely to occur and be breeding in your project area, view the Probability of Presence Summary. Additional measures and/or permits may be advisable depending on the type of activity you are conducting and the type of infrastructure or bird species present on your project site.

What does IPaC use to generate the migratory birds potentially occurring in my specified location?

The Migratory Bird Resource List is comprised of USFWS <u>Birds of Conservation Concern (BCC)</u> and other species that may warrant special attention in your project location.

The migratory bird list generated for your project is derived from data provided by the <u>Avian Knowledge Network</u> (<u>AKN</u>). The AKN data is based on a growing collection of <u>survey</u>, <u>banding</u>, <u>and citizen science datasets</u> and is queried and filtered to return a list of those birds reported as occurring in the 10km grid cell(s) which your project

intersects, and that have been identified as warranting special attention because they are a BCC species in that area, an eagle (<u>Eagle Act</u> requirements may apply), or a species that has a particular vulnerability to offshore activities or development.

Again, the Migratory Bird Resource list includes only a subset of birds that may occur in your project area. It is not representative of all birds that may occur in your project area. To get a list of all birds potentially present in your project area, please visit the AKN Phenology Tool.

What does IPaC use to generate the probability of presence graphs for the migratory birds potentially occurring in my specified location?

The probability of presence graphs associated with your migratory bird list are based on data provided by the <u>Avian Knowledge Network (AKN)</u>. This data is derived from a growing collection of <u>survey</u>, <u>banding</u>, <u>and citizen science datasets</u>.

Probability of presence data is continuously being updated as new and better information becomes available. To learn more about how the probability of presence graphs are produced and how to interpret them, go the Probability of Presence Summary and then click on the "Tell me about these graphs" link.

How do I know if a bird is breeding, wintering, migrating or present year-round in my project area?

To see what part of a particular bird's range your project area falls within (i.e. breeding, wintering, migrating or year-round), you may refer to the following resources: The Cornell Lab of Ornithology All About Birds Bird Guide, or (if you are unsuccessful in locating the bird of interest there), the Cornell Lab of Ornithology Neotropical Birds guide. If a bird on your migratory bird species list has a breeding season associated with it, if that bird does occur in your project area, there may be nests present at some point within the timeframe specified. If "Breeds elsewhere" is indicated, then the bird likely does not breed in your project area.

What are the levels of concern for migratory birds?

Migratory birds delivered through IPaC fall into the following distinct categories of concern:

- 1. "BCC Rangewide" birds are <u>Birds of Conservation Concern</u> (BCC) that are of concern throughout their range anywhere within the USA (including Hawaii, the Pacific Islands, Puerto Rico, and the Virgin Islands);
- 2. "BCC BCR" birds are BCCs that are of concern only in particular Bird Conservation Regions (BCRs) in the continental USA; and
- 3. "Non-BCC Vulnerable" birds are not BCC species in your project area, but appear on your list either because of the <u>Eagle Act</u> requirements (for eagles) or (for non-eagles) potential susceptibilities in offshore areas from certain types of development or activities (e.g. offshore energy development or longline fishing).

Although it is important to try to avoid and minimize impacts to all birds, efforts should be made, in particular, to avoid and minimize impacts to the birds on this list, especially eagles and BCC species of rangewide concern. For more information on conservation measures you can implement to help avoid and minimize migratory bird impacts and requirements for eagles, please see the FAQs for these topics.

Details about birds that are potentially affected by offshore projects

For additional details about the relative occurrence and abundance of both individual bird species and groups of bird species within your project area off the Atlantic Coast, please visit the <u>Northeast Ocean Data Portal</u>. The Portal also offers data and information about other taxa besides birds that may be helpful to you in your project review. Alternately, you may download the bird model results files underlying the portal maps through the <u>NOAA NCCOS Integrative Statistical Modeling and Predictive Mapping of Marine Bird Distributions and Abundance on the Atlantic Outer Continental Shelf project webpage.</u>

Bird tracking data can also provide additional details about occurrence and habitat use throughout the year, including migration. Models relying on survey data may not include this information. For additional information on marine bird tracking data, see the <u>Diving Bird Study</u> and the <u>nanotag studies</u> or contact <u>Caleb Spiegel</u> or <u>Pam Loring</u>.

What if I have eagles on my list?

If your project has the potential to disturb or kill eagles, you may need to <u>obtain a permit</u> to avoid violating the Eagle Act should such impacts occur.

Proper Interpretation and Use of Your Migratory Bird Report

The migratory bird list generated is not a list of all birds in your project area, only a subset of birds of priority concern. To learn more about how your list is generated, and see options for identifying what other birds may be in your project area, please see the FAQ "What does IPaC use to generate the migratory birds potentially occurring in my specified location". Please be aware this report provides the "probability of presence" of birds within the 10 km grid cell(s) that overlap your project; not your exact project footprint. On the graphs provided, please also look carefully at the survey effort (indicated by the black vertical bar) and for the existence of the "no data" indicator (a red horizontal bar). A high survey effort is the key component. If the survey effort is high, then the probability of presence score can be viewed as more dependable. In contrast, a low survey effort bar or no data bar means a lack of data and, therefore, a lack of certainty about presence of the species. This list is not perfect; it is simply a starting point for identifying what birds of concern have the potential to be in your project area, when they might be there, and if they might be breeding (which means nests might be present). The list helps you know what to look for to confirm presence, and helps guide you in knowing when to implement conservation measures to avoid or minimize potential impacts from your project activities, should presence be confirmed. To learn more about conservation measures, visit the FAQ "Tell me about conservation measures I can implement to avoid or minimize impacts to migratory birds" at the bottom of your migratory bird trust resources page.

Facilities

National Wildlife Refuge lands

Any activity proposed on lands managed by the <u>National Wildlife Refuge</u> system must undergo a 'Compatibility Determination' conducted by the Refuge. Please contact the individual Refuges to discuss any questions or concerns.

THERE ARE NO REFUGE LANDS AT THIS LOCATION.

Fish hatcheries

THERE ARE NO FISH HATCHERIES AT THIS LOCATION.

Wetlands in the National Wetlands Inventory

Impacts to <u>NWI wetlands</u> and other aquatic habitats may be subject to regulation under Section 404 of the Clean Water Act, or other State/Federal statutes.

For more information please contact the Regulatory Program of the local <u>U.S. Army Corps of Engineers District</u>.

Please note that the NWI data being shown may be out of date. We are currently working to update our NWI data set. We recommend you verify these results with a site visit to determine the actual extent of wetlands on site.

This location overlaps the following wetlands:

FRESHWATER EMERGENT WETLAND PEM1Ah PEM1A FORCONSULTATIO PEM1C PEM1F FRESHWATER FORESTED/SHRUB WETLAND PSS2A PSS2Ah PFO1A PSS2Ax FRESHWATER POND **PUBF PUBHx PUSC PUBFh PUSAh** LAKE L1UBHh L2USAh RIVERINE R4SBC R2USA R2UBH R4SBA R4SBAx **R5UBH**

A full description for each wetland code can be found at the National Wetlands Inventory website

Data limitations

The Service's objective of mapping wetlands and deepwater habitats is to produce reconnaissance level information on the location, type and size of these resources. The maps are prepared from the analysis of high altitude imagery. Wetlands are identified based on vegetation, visible hydrology and geography. A margin of error is inherent in the use of imagery; thus, detailed on-the-ground inspection of any particular site may result in revision of the wetland boundaries or classification established through image analysis.

The accuracy of image interpretation depends on the quality of the imagery, the experience of the image analysts, the amount and quality of the collateral data and the amount of ground truth verification work conducted. Metadata should be consulted to determine the date of the source imagery used and any mapping problems.

Wetlands or other mapped features may have changed since the date of the imagery or field work. There may be occasional differences in polygon boundaries or classifications between the information depicted on the map and the actual conditions on site.

Data exclusions

Certain wetland habitats are excluded from the National mapping program because of the limitations of aerial imagery as the primary data source used to detect wetlands. These habitats include seagrasses or submerged aquatic vegetation that are found in the intertidal and subtidal zones of estuaries and nearshore coastal waters. Some deepwater reef communities (coral or tuberficid worm reefs) have also been excluded from the inventory. These habitats, because of their depth, go undetected by aerial imagery.

Data precautions

Federal, state, and local regulatory agencies with jurisdiction over wetlands may define and describe wetlands in a different manner than that used in this inventory. There is no attempt, in either the design or products of this inventory, to define the limits of proprietary jurisdiction of any Federal, state, or local government or to establish the geographical scope of the regulatory programs of government agencies. Persons intending to engage in activities involving modifications within or adjacent to wetland areas should seek the advice of appropriate federal, state, or local agencies concerning specified agency regulatory programs and proprietary jurisdictions that may affect such activities.

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Project No. 19-600-211-00

SOIL MAP



Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

33° 9′47″ N

33° 6′18″ N

Web Soil Survey National Cooperative Soil Survey

MAP LEGEND

Very Stony Spot Stony Spot Spoil Area Wet Spot Other W Soil Map Unit Polygons Area of Interest (AOI) Soil Map Unit Points Soil Map Unit Lines Special Point Features Area of Interest (AOI) Soils



































Borrow Pit

Blowout

Clay Spot

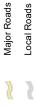


Closed Depression



Gravelly Spot

Gravel Pit







Aerial Photography

Miscellaneous Water

Perennial Water

Rock Outcrop

Saline Spot Sandy Spot Severely Eroded Spot

Slide or Slip Sodic Spot

Sinkhole

Marsh or swamp

Lava Flow

Landfill

Mine or Quarry

Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

distance and area. A projection that preserves area, such as the Maps from the Web Soil Survey are based on the Web Mercator

projection, which preserves direction and shape but distorts

Source of Map: Natural Resources Conservation Service

Coordinate System: Web Mercator (EPSG:3857)

Web Soil Survey URL:

The soil surveys that comprise your AOI were mapped at

MAP INFORMATION

Please rely on the bar scale on each map sheet for map

measurements.

This product is generated from the USDA-NRCS certified data as

of the version date(s) listed below.

Soil Survey Area: Sierra County Area, New Mexico Survey Area Data: Version 15, Sep 15, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Dec 31, 2009—Dec

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

USDA

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
2	Agua silt loam, 0 to 2 percent slopes	19.8	0.2%
3	Agustin gravelly sandy loam, 1 to 9 percent slopes	347.4	3.2%
6	Anapra clay loam	16.9	0.2%
7	Anthony-Vinton fine sandy loam	234.2	2.2%
8	Anthony-Vinton loams, 0 to 1 percent slopes	64.0	0.6%
12	Arizo-Riverwash complex, 1 to 3 percent slopes	81.1	0.8%
13	Arizo and Canutio soils, gently sloping	439.1	4.1%
16	Badland-Nickel complex, extremely steep	255.3	2.4%
20	Bluepoint loamy sand, 0 to 5 percent slopes	322.2	3.0%
21	Bluepoint loamy fine sand, moderately rolling	567.1	5.3%
23	Brazito loamy fine sand, gently sloping	59.0	0.5%
24	Brazito very fine sandy loam	34.1	0.3%
25	Caliza-Bluepoint-Yturbide association, very steep	970.8	9.0%
26	Canutio-Pajarito association, moderately rolling	7.9	0.1%
28	Courthouse-Rock outcrop association, very steep	126.8	1.2%
30	Delnorte-Cave-Tencee complex, moderately rolling	193.7	1.8%
35	Glendale loam	28.0	0.3%
36	Glendale clay loam, 0 to 1 percent slopes	61.0	0.6%
37	Glendale-Gila complex, nearly level	66.4	0.6%
41	Harkey loam	248.2	2.3%
42	Harkey loam, saline and alkali	246.0	2.3%
52	Lozier-Rock outcrop association, hilly	26.1	0.2%
62	Nickel very gravelly fine sandy loam, very steep	3,986.5	37.1%
63	Nickel-Chamberino association, gently sloping	221.4	2.1%

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	
64	Nickel-Tencee-Delnorte complex, moderately sloping	44.3	0.4%	
66	Pajarito fine sandy loam	240.9	2.2%	
70	Rock outcrop, extremely steep	313.4	2.9%	
71	Rock outcrop-Courthouse complex, extremely steep	78.0	0.7%	
75	Rock outcrop-Torriorthents association, extremely steep	834.4	7.8%	
77	Simona loamy fine sand, gently sloping	99.2	0.9%	
83	Urban land	319.8	3.0%	
W	Water	196.6	1.8%	
Totals for Area of Interest		10,749.7	100.0%	

Sierra County Area, New Mexico

3—Agustin gravelly sandy loam, 1 to 9 percent slopes

Map Unit Setting

National map unit symbol: 1wss Elevation: 4,100 to 5,300 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Agustin and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Agustin

Setting

Landform: Alluvial fans

Landform position (three-dimensional): Rise

Down-slope shape: Linear Across-slope shape: Linear Parent material: Mixed alluvium

Typical profile

H1 - 0 to 6 inches: gravelly sandy loam H2 - 6 to 60 inches: gravelly sandy loam

Properties and qualities

Slope: 1 to 9 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00

in/hr

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum in profile: 5 percent

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Low (about 4.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7e

Hydrologic Soil Group: A

Ecological site: Gravelly (R042XB010NM)

Hydric soil rating: No

Minor Components

Arizo

Percent of map unit:

Ecological site: Gravelly Sand (R042XB024NM)

Hydric soil rating: No

Canutio

Percent of map unit:

Ecological site: Gravelly Sand (R042XB024NM)

Hydric soil rating: No

6—Anapra clay loam

Map Unit Setting

National map unit symbol: 1wtz Elevation: 4,050 to 5,200 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Anapra and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Anapra

Setting

Landform: Flood plains

Landform position (three-dimensional): Talf

Down-slope shape: Linear Across-slope shape: Concave Parent material: Mixed alluvium

Typical profile

H1 - 0 to 29 inches: clay loam

H2 - 29 to 60 inches: stratified sand to loamy sand

Properties and qualities

Slope: 0 to 1 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20

to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: Occasional Frequency of ponding: None

Calcium carbonate, maximum in profile: 10 percent

Salinity, maximum in profile: Very slightly saline to moderately saline (2.0 to 8.0

mmhos/cm)

Sodium adsorption ratio, maximum in profile: 4.0

Available water storage in profile: Moderate (about 7.7 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 7s

Hydrologic Soil Group: C

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Minor Components

Vinton

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Glendale

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Harkey

Percent of map unit:

Ecological site: Clayey (R042XB023NM)

Hydric soil rating: No

Brazito

Percent of map unit:

Ecological site: Deep Sand (R042XB011NM)

Hydric soil rating: No

Agua

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

7—Anthony-Vinton fine sandy loam

Map Unit Setting

National map unit symbol: 2sps4 Elevation: 4,100 to 4,350 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Anthony and similar soils: 50 percent Vinton and similar soils: 35 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Anthony

Setting

Landform: Flood plains

Landform position (three-dimensional): Talf

Down-slope shape: Linear Across-slope shape: Concave Parent material: Mixed alluvium

Typical profile

H1 - 0 to 12 inches: fine sandy loam H2 - 12 to 60 inches: fine sandy loam

Properties and qualities

Slope: 0 to 1 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: Occasional Frequency of ponding: None

Calcium carbonate, maximum in profile: 5 percent

Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Moderate (about 7.8 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 7s

Hydrologic Soil Group: A

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Description of Vinton

Setting

Landform: Flood plains

Landform position (three-dimensional): Talf

Down-slope shape: Linear Across-slope shape: Concave Parent material: Mixed alluvium

Typical profile

H1 - 0 to 15 inches: fine sandy loam H2 - 15 to 60 inches: loamy fine sand

Properties and qualities

Slope: 0 to 1 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Somewhat excessively drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: Occasional

Frequency of ponding: None

Calcium carbonate, maximum in profile: 5 percent

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Moderate (about 6.1 inches)

Interpretive groups

Land capability classification (irrigated): 3s Land capability classification (nonirrigated): 7s

Hydrologic Soil Group: A

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Minor Components

Harkey

Percent of map unit:

Ecological site: Loamy (R042XB014NM)

Hydric soil rating: No

Vinton

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Anthony

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

8—Anthony-Vinton loams, 0 to 1 percent slopes

Map Unit Setting

National map unit symbol: 2tm52 Elevation: 3,740 to 4,980 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 57 to 64 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Anthony and similar soils: 50 percent Vinton and similar soils: 30 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Anthony

Setting

Landform: Flood plains

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Talf

Down-slope shape: Linear Across-slope shape: Linear Parent material: Mixed alluvium

Typical profile

Ap1 - 0 to 9 inches: loam Ap2 - 9 to 17 inches: loam

C1 - 17 to 39 inches: fine sandy loam C2 - 39 to 60 inches: loamy fine sand

Properties and qualities

Slope: 0 to 1 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20

to 0.57 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum in profile: 10 percent

Gypsum, maximum in profile: 2 percent

Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Moderate (about 7.4 inches)

Interpretive groups

Land capability classification (irrigated): 2s Land capability classification (nonirrigated): 7s

Hydrologic Soil Group: C

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Description of Vinton

Setting

Landform: Flood plains

Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Talf

Down-slope shape: Linear Across-slope shape: Linear Parent material: Mixed alluvium

Typical profile

Ap - 0 to 14 inches: silt loam
C1 - 14 to 22 inches: fine sand
C2 - 22 to 45 inches: loamy fine sand
C3 - 45 to 50 inches: fine sand
C4 - 50 to 60 inches: loamy sand

Properties and qualities

Slope: 0 to 1 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Somewhat excessively drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum in profile: 4 percent

Gypsum, maximum in profile: 2 percent

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Moderate (about 6.3 inches)

Interpretive groups

Land capability classification (irrigated): 3s Land capability classification (nonirrigated): 7s

Hydrologic Soil Group: A

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Minor Components

Harkey

Percent of map unit:

Agua

Percent of map unit: Hydric soil rating: No

13—Arizo and Canutio soils, gently sloping

Map Unit Setting

National map unit symbol: 1ws6 Elevation: 4.050 to 5.300 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Not prime farmland

Map Unit Composition

Arizo and similar soils: 40 percent Canutio and similar soils: 40 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Arizo

Setting

Landform: Alluvial fans, flood plains

Landform position (three-dimensional): Rise, talf

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Mixed gravelly alluvium

Typical profile

H1 - 0 to 4 inches: very gravelly sandy loam

H2 - 4 to 60 inches: stratified sand to very gravelly loamy sand

Properties and qualities

Slope: 1 to 9 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Excessively drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: Occasional Frequency of ponding: None

Calcium carbonate, maximum in profile: 5 percent

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Very low (about 2.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7w

Hydrologic Soil Group: A

Ecological site: Gravelly Sand (R042XB024NM)

Hydric soil rating: No

Description of Canutio

Setting

Landform: Alluvial fans, flood plains

Landform position (three-dimensional): Rise, talf

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Mixed gravelly alluvium

Typical profile

H1 - 0 to 4 inches: very gravelly sandy loam H2 - 4 to 60 inches: very gravelly sandy loam

Properties and qualities

Slope: 1 to 9 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: Occasional Frequency of ponding: None

Calcium carbonate, maximum in profile: 5 percent

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Very low (about 3.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7w

Hydrologic Soil Group: A

Ecological site: Gravelly Sand (R042XB024NM)

Hydric soil rating: No

Minor Components

Bluepoint

Percent of map unit:

Ecological site: Deep Sand (R042XB011NM)

Hydric soil rating: No

Yturbide

Percent of map unit:

Ecological site: Deep Sand (R042XB011NM)

Hydric soil rating: No

23—Brazito loamy fine sand, gently sloping

Map Unit Setting

National map unit symbol: 1wsk Elevation: 4,050 to 5,300 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Not prime farmland

Map Unit Composition

Brazito and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Brazito

Setting

Landform: Flood plains

Landform position (three-dimensional): Talf

Down-slope shape: Linear Across-slope shape: Concave Parent material: Mixed alluvium

Typical profile

H1 - 0 to 6 inches: loamy fine sand

H2 - 6 to 60 inches: sand

Properties and qualities

Slope: 0 to 5 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Excessively drained

Runoff class: Negligible

Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum in profile: 4 percent

Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): 4s Land capability classification (nonirrigated): 7s

Hydrologic Soil Group: A

Ecological site: Deep Sand (R042XB011NM)

Hydric soil rating: No

Minor Components

Caliza

Percent of map unit:

Ecological site: Gravelly Sand (R042XB024NM)

Hydric soil rating: No

Glendale

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Arizo

Percent of map unit:

Ecological site: Gravelly Sand (R042XB024NM)

Hydric soil rating: No

Gila

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

24—Brazito very fine sandy loam

Map Unit Setting

National map unit symbol: 1wsl Elevation: 4,050 to 5,200 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Brazito and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Brazito

Setting

Landform: Flood plains

Landform position (three-dimensional): Talf

Down-slope shape: Linear Across-slope shape: Concave Parent material: Mixed alluvium

Typical profile

H1 - 0 to 14 inches: very fine sandy loam

H2 - 14 to 60 inches: sand

Properties and qualities

Slope: 0 to 1 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Excessively drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

high (0.60 to 2.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum in profile: 4 percent

Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Low (about 5.2 inches)

Interpretive groups

Land capability classification (irrigated): 4s Land capability classification (nonirrigated): 7s

Hydrologic Soil Group: B

Ecological site: Deep Sand (R042XB011NM)

Hydric soil rating: No

Minor Components

Vinton

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Agua

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Anthony

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

26—Canutio-Pajarito association, moderately rolling

Map Unit Setting

National map unit symbol: 1wsn Elevation: 4,050 to 5,300 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Not prime farmland

Map Unit Composition

Canutio and similar soils: 45 percent Pajarito and similar soils: 35 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Canutio

Setting

Landform: Ridges on alluvial fans

Landform position (two-dimensional): Summit, shoulder

Landform position (three-dimensional): Head slope, crest, side slope, rise

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Mixed gravelly alluvium

Typical profile

H1 - 0 to 4 inches: very gravelly sandy loam H2 - 4 to 60 inches: very gravelly sandy loam

Properties and qualities

Slope: 1 to 15 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum in profile: 5 percent

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Low (about 4.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7s

Hydrologic Soil Group: A

Ecological site: Gravelly (R042XB010NM)

Hydric soil rating: No

Description of Pajarito

Setting

Landform: Alluvial fans

Landform position (three-dimensional): Rise

Down-slope shape: Linear Across-slope shape: Linear Parent material: Mixed alluvium

Typical profile

H1 - 0 to 4 inches: gravelly sandy loam

H2 - 4 to 60 inches: loam

Properties and qualities

Slope: 1 to 9 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

high (0.57 to 1.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum in profile: 4 percent

Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Moderate (about 8.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7e

Hydrologic Soil Group: B

Ecological site: Loamy (R042XB014NM)

Hydric soil rating: No

Minor Components

Glendale

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Bluepoint

Percent of map unit:

Ecological site: Deep Sand (R042XB011NM)

Hydric soil rating: No

Yturbide

Percent of map unit:

Ecological site: Deep Sand (R042XB011NM)

Hydric soil rating: No

37—Glendale-Gila complex, nearly level

Map Unit Setting

National map unit symbol: 1wt1 Elevation: 4,050 to 5,300 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Not prime farmland

Map Unit Composition

Glendale and similar soils: 40 percent Gila and similar soils: 35 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Glendale

Setting

Landform: Flood plains

Landform position (three-dimensional): Talf

Down-slope shape: Linear Across-slope shape: Concave Parent material: Mixed alluvium

Typical profile

H1 - 0 to 3 inches: silty clay loam H2 - 3 to 60 inches: silty clay loam

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20

to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: Occasional Frequency of ponding: None

Calcium carbonate, maximum in profile: 10 percent

Salinity, maximum in profile: Very slightly saline to slightly saline (2.0 to 4.0

mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: High (about 11.4 inches)

Interpretive groups

Land capability classification (irrigated): 2w Land capability classification (nonirrigated): 7w

Hydrologic Soil Group: C

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Description of Gila

Setting

Landform: Flood plains

Landform position (three-dimensional): Talf

Down-slope shape: Concave Across-slope shape: Linear Parent material: Mixed alluvium

Typical profile

H1 - 0 to 8 inches: very fine sandy loam

H2 - 8 to 60 inches: stratified gravelly sandy loam to silt loam

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20

to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: Occasional Frequency of ponding: None

Calcium carbonate, maximum in profile: 4 percent

Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Moderate (about 6.8 inches)

Interpretive groups

Land capability classification (irrigated): 2w Land capability classification (nonirrigated): 7w

Hydrologic Soil Group: C

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Minor Components

Arizo

Percent of map unit:

Ecological site: Gravelly Sand (R042XB024NM)

Hydric soil rating: No

Brazito

Percent of map unit:

Ecological site: Deep Sand (R042XB011NM)

Hydric soil rating: No

Canutio

Percent of map unit:

Ecological site: Gravelly (R042XB010NM)

Hydric soil rating: No

Vinton

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

41—Harkey loam

Map Unit Setting

National map unit symbol: 1wtb Elevation: 4,100 to 5,000 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Harkey and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Harkey

Setting

Landform: Flood plains

Landform position (three-dimensional): Talf

Down-slope shape: Linear
Across-slope shape: Concave
Parent material: Mixed alluvium

Typical profile

H1 - 0 to 12 inches: loam H2 - 12 to 60 inches: silt loam

Properties and qualities

Slope: 0 to 1 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

high (0.60 to 2.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: Rare Frequency of ponding: None

Calcium carbonate, maximum in profile: 5 percent

Salinity, maximum in profile: Nonsaline to slightly saline (0.0 to 4.0 mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: High (about 9.5 inches)

Interpretive groups

Land capability classification (irrigated): 1 Land capability classification (nonirrigated): 7c

Hydrologic Soil Group: B

Ecological site: Loamy (R042XB014NM)

Hydric soil rating: No

Minor Components

Agua

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Anthony

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

Glendale

Percent of map unit:

Ecological site: Salty Bottomland (R042XB033NM)

Hydric soil rating: No

Vinton

Percent of map unit:

Ecological site: Bottomland (R042XB018NM)

Hydric soil rating: No

62—Nickel very gravelly fine sandy loam, very steep

Map Unit Setting

National map unit symbol: 1wv2 Elevation: 3,000 to 6,000 feet

Mean annual precipitation: 8 to 10 inches

Mean annual air temperature: 58 to 65 degrees F

Frost-free period: 180 to 220 days

Farmland classification: Not prime farmland

Map Unit Composition

Nickel and similar soils: 80 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Nickel

Setting

Landform: Fan piedmonts

Landform position (three-dimensional): Tread

Down-slope shape: Linear Across-slope shape: Convex

Parent material: Mixed gravelly alluvium

Typical profile

H1 - 0 to 12 inches: very gravelly fine sandy loam H2 - 12 to 60 inches: very gravelly fine sandy loam

Properties and qualities

Slope: 10 to 65 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95

in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum in profile: 25 percent

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Sodium adsorption ratio, maximum in profile: 2.0

Available water storage in profile: Low (about 4.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7e

Hydrologic Soil Group: A

Ecological site: Gravelly (R042XB010NM)

Hydric soil rating: No

Minor Components

Badlands

Percent of map unit: Hydric soil rating: No

Chamberino

Percent of map unit:

Ecological site: Gravelly (R042XB010NM)

Hydric soil rating: No

Eba

Percent of map unit:

Ecological site: Gravelly Loam (R042XB035NM)

Hydric soil rating: No

W-Water

Map Unit Composition

Water: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

APPENDIX 2- WATER SYSTEM FINANCIAL INFORMATION

TABLE OF CONTENT:

o WATER SYSTEM FINANCIAL INFORMATION REPORT CITY OF TRUTH OR CONSEQUENCES



CITY OF TRUTH OR CONSEQUENCES BUDGET FOR FISCAL YEAR 7/1/19 TO 6/30/20

CITY OF TRUTH OR CONSEQUENCES BUDGET FOR FISCAL YEAR 7/1/19 TO 6/30/20

38-03 WATER DIVISION	2013-2014	2014-15	2015-16	2016-17	2017-18	2018-19	2019-20	%
EVDENDITIBES	Jernai		Jena	Total at	Jorgan	- Vernal	Lingi	Cilange
504-3803-40305 MII EAGE REIMB	i	,		202	788	131	1 000	70E99
		077	702	252	200	7 7 7	000,-	4000
	1,025	9700	100	000-	7,430	-,-	000,0	479/
	0,790	0,00	0,471	200,0	7,030	0,044	0,000	%/1
504-3803-4331/ DIESEL FUEL	9,459	67,249	7,0,4	4,48/	0,040	9,262	75,000	4400/
	7,170	1,0,10	02,302	00,000	44,000	34,270	000'6'	%61
504-3803-47420 MAINI. VEHICLE	5,704	2,731	25,445	14,501	9,717	4,399	6,000	36%
	1 7	212	9,981	13,192	2,649	3,580	4,000	%ZL
	5,108	2,585	5,6/3	1 0	2,455	2,241	6,000	168%
	ı	ı	4,215	12,950	52,918	30,434	22,350	-27%
			ı	1	630	1 1		#DIV/0!
		100	1	1 7	50 E	3,743	2,000	47%
	493	1,087	9/	2,114	975	878	3,000	242%
	2,711	436	2,268	1,258	2,089	18,476	8,000	-57%
	1	• (• 1	4,564	4,000	-12%
	5,010	1,906	3,426	3,313	3,593	3,880	3,000	-23%
	2,548	1,768	1,330	1,762	1,741	1,351	1,500	11%
	2,046	1,218	715	1,461	1,385	220	4,000	627%
_	1			,	2,098	1	•	#DIV/0i
	8,284	9,084	8,445	8,165	8,538	9,936	10,600	%2
	19,786	19,620	19,392	19,194	19,545	20,488	24,100	18%
	7,358	7,920	8,027	8,411	11,460	8,876	9,958	12%
_	,	661	1,042	11,586	9,126	10,217	10,000	-2%
	962	1,264	1,455	1,531	1,509	1,443	1,000	-31%
	138,833	124,941	107,994	98,141	91,227	131,825	95,000	-28%
	38,717	38,554	43,208	39,673	40,005	39,598	41,000	4%
	1,682	1,694	1,694	2,629	2,900	2,989	3,000	%
	13,078	11,720	12,294	12,676	12,880	13,632	10,000	-27%
504-3803-43/40 PKINIING/PUBLISHING	,		ı	1	83/	651	000	-23%
TOTAL OPERATING EXPENSES	351,536	309,949	331,594	329,683	340,178	365,375	363,508	-1%
CAPITAL OUTLAY								
504-3803-80810 CAPITAL EQUIPMENT/MACHINERY	55,565	23,748	28,500	1	79,000	48,938	103,000	110%
TOTAL CAPITAL OUTLAY	55,565	23,748	28,500	, .	79,000	48,938	103,000	110%
TOTAL EXPENDITURES	743,518	616,813	602,896	537,406	701,038	686,451	743,638	%8
NET INCOME	(96,598)	92,717	(827,772)	160,014	(256,361)	(63,439)	191,387	-402%
101 Capaci Euck	100 000 001	100 000	100 000 001	100 000 001	000000		0000	
101 General Fund 301 W/WW Effluent Fund	(120,000.00)	(100,000.00)	(100,000.00)	(100,000.00)	(100,000.00)	(75,000.00)	(50,000.00)	
502 Jt. Utility Office Support	(27.261.00)	(24.000.00)	(33,000,00)	(44 400 00)	(86 200 00)	(82 130 00)	(59 740 00)	
306 Capital Improvement Jt. Utility Per Code	(115,000.00)	(125,000.00)	(141,159.00)	(141,159.00)	(141,159.00)	(141,974.00)	(23,787.00)	
306 Capital Improvement Jt. Utility Debt Service	(23,276.00)	(21,124.00)	(21,168.00)	(20,714.00)	(20,714.00)	(20,714.00)	(254,984.00)	
313 R&R Water Fund		(2.00)	(2.00)	(2.00)	, , , , , , , , , , , , , , , , , , , ,	•	•	
314 CDBG 316 Emergency Repair Fund	(2,500.00)	(2,500.00)	(2,500.00)	(2,500.00)	(160,000.00)	(2,500,00)	(10,000,00)	
	(288,037)	(272,628)	(297,831)	(308,777)	(510,573)	(322,318)	(398,511)	

CITY OF TRUTH OR CONSEQUENCES BUDGET FOR FISCAL YEAR 7/1/19 TO 6/30/20

Enterprise Funds Recap		iscal Year 2013-14 Actual		Fiscal Year 2014-15 Actual	ļ.	Fiscal Year 2015-16 Actual	正 ``	Fiscal Year 2016-17 Final	E "	Fiscal Year 2016-17 Final	FIE C	Fiscal Year 2018-19 Actual	正	Fiscal Year 2019-20 Final	% Change Last FY
504 Water Division Revenues	₩	934,957	₩	957,153	€9	72,952	↔	1,006,193	€9	955,250	↔	945,330	↔	1,057,195	12%
Total Revenues	4	934,957	69	957,153	₩.	72,952	49	1,006,193	49	955,250	₩.	945,330	₩	1,057,195	12%
Transfers: IN (OUT)	€	(288,037)	\$	(247,624)	↔	(297,827)	↔	(308,773)	69	(510,573)	€9	(322,318)	↔	(122,170)	-62%
Expenditures Personnel Services Operating Expense	⇔ ↔	336,418 351,536		(4 (7)		242,802 331,594	↔ ↔	207,723 329,683	₩ ₩	281,860 340,178	₩ ₩	272,138 365,375	↔ ↔	277,130 363,508	2%
Capital Outlay Total Expenditures	↔ •	55,565 743,519	↔ ↔	23,748 616,813	⇔ •	28,500 602,896	↔ ↔	537,406	()	79,000 701,038	↔ •	48,938 686,451	69 69	103,000 743,638	110% 8%
505 Solid Waste Division Revenues	€	1,249,276	₩	1,241,653	€9	1,177,548	€9	1,394,707	↔	1,453,594	€9	1,525,765	↔	2,147,200	41%
Total Revenues	49	1,249,276	<i>€</i>	1,241,653	₩.	1,177,548	€9	1,394,707	49	1,453,594	69	1,525,765	49	2,147,200	41%
Transfers: IN (OUT)	€	(178,569)	\$	(194,319)	⇔	(179,034)	↔	(189,314)	€9	(213,114)	€9	(218,710)	€>	(400,901)	83%
Expenditures Personnel Services	€\$	575,309		,	-	428,140	₩.	468,949	€9	506,028	€9	510,070	↔	631,930	24%
Operating Expense	69 69	449,998	ა •	504,959		468,366	6 69	555,006 74.021	()	535,784	и и	609,091	69 69	1,098,171	80% 402%
Total Expenditures	•	1,025,307		6	÷ (A	896,506	₩	1,097,976	₩	1,069,044	· 69	1,159,282	69	1,931,310	%29
506 Waste Water Division Revenues	₩	702,329	↔	746,677	↔	882,012	₩	1,064,404	€9	1,035,164	€	1,107,661	€9	1,153,300	4%
Total Revenues	₩	702,329	€9	746,677	49	882,012	€9	1,064,404	₩	1,035,164	₩.	1,107,661	69	1,153,300	4%
Transfers: IN (OUT)	₩	40,027	₩	(14,064)	↔	(146,763)	₩	(187,333)	↔	41,129	↔	(214,801)	↔	(238,902)	11%
Expenditures Personnel Services Operating Expense Capital Outlay	& & & & &	325,599 402,568 65,020 793,187	<i>••••••</i>	340,992 409,812 32,181 782,985	<i>↔ ↔ •</i>	340,747 342,183 - 682,930	\$ \$ \$ \$ \$	333,565 329,368 - 662,933	9 9 9 9	292,170 587,817 115,622 995,609	& & & %	273,938 446,162 110,038 830,138	↔ ↔ ↔	320,628 447,053 135,000 902,681	17% 0% 23% 9%

APPENDIX 3- MUNICIPAL AIRPORT

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- TRUTH OR CONSEQUENCES MUICIPAL AIRPORT ACTIVATION AS A PUBLIC WATER SYSTEM
- o 2019 SANITARY SURVEY REPORT T OR C MUNICIPAL AIRPORT ACTION PLAN
- o SAMPLING TESTING MUNICIPAL AIRPORT
- o 2019 SANITARY SURVEY REPORT





TRUTH OR CONSEQUENCES MUICIPAL AIRPORT ACTIVATION AS A PUBLIC WATER SYSTEM





SUSANA MARTINEZ
Governor
JOHN A. SANCHEZ
Lt. Governor

NEW MEXICO ENVIRONMENT DEPARTMENT

Drinking Water Bureau
2301 Entrada Del Sol
Tel. 575-915-1113• Fax 575-526-6162
Toll Free 1-877-654-8720
www.nmenv.state.nm.us



August 27, 2018

Juan Fuentes-City Manager Truth or Consequences Municipal Airport; NM3501427 505 Sims Truth or Consequences, NM 87901

Subject: Truth or Consequences Municipal Airport Activation as a Public Water System; #NM3501427

Mr. Juan Fuentes,

The New Mexico Environment Department Drinking Water Bureau (NMED-DWB) has determined that the Truth or Consequences Municipal Airport water system located near Truth or Consequences, NM requires monitoring as a Public Water System (PWS). The Truth or Consequences Municipal Airport water system has been classified as a Non-Community water system with a transient population of 40. As a result of this determination of PWS status, the Truth or Consequences Municipal Airport must comply with all relevant rules and regulations for public water systems in 40 CFR 141 and 20.7.10 NMAC. Some of the initial requirements that you should immediately begin working on are as follows:

The Truth or Consequences Municipal Airport water system is required to employ a certified operator with a minimum of a Small Water Certification. You can obtain operator certification information at the following link: https://www.env.nm.gov/drinking_water/dwb-utility-operator-certification-program/

Pursuant to Section 20.7.10.100 NMAC [incorporating 40 CFR Section 141.853(a)(1)], The Truth or Consequences Municipal Airport water system must develop a written sample siting plan that identifies sampling sites and a sample collection schedule that is representative of water throughout the distribution system. You can obtain a sample plan template at https://www.env.nm.gov/dwb/RTCR.htm Microbiological samples are required to be collected by your certified operator according to an approved sample siting plan. Your plan will need to be provided to NMED-DWB for review and approval before samples can be used for compliance determination. Your first microbiological sample to be used for compliance is due in October 2018. The Truth or Consequences Municipal Airport water system is required to collect one microbiological sample per month and provide analytical results to NMED each month.

Pursuant to Section 20.7.10.100 NMAC [incorporating 40 CFR Section 141.403(a)(4)], the Truth or Consequences Municipal Airport water system is required to develop a written Operation and Maintenance (O&M) Plan. You can obtain an O&M Plan template at https://www.env.nm.gov/dwb/forms/index.htm The O&M Plan will need to be provided to NMED-DWB for review.

The Truth or Consequences Municipal Airport water system is required to retain the records associated with the water system for the following periods of time:

- Bacteriological samples 5 years
- Nitrate samples 10 years
- Records of action taken to correct violations 3 years after last action
- Reports, correspondence, communication and sanitary surveys 10 years
- Variance granted to the system -5 years following the expiration of the variance

If you have any questions regarding the activation of your system, please contact your assigned Compliance Officer Aaron Beckworth in the Silver City office at 575-956-1552 or by email at aaron.beckworth@state.nm.us.

Sincerely,

Brandi Garcia, Compliance Supervisor

Drinking Water Bureau Water Protection Division

cc: Joe Martinez-PWSS Manager (electronic)

Aaron Beckworth-Compliance Officer (electronic)

Silver City Field Office

Electronic File System

2019 SANITARY SURVEY REPORT T OR C MUNICIPAL AIRPORT ACTION PLAN



Sandra Whitehead Mayor

Kathy Clark Mayor Pro-Tem

Rolf Hechler Commissioner



505 Sims St.

Truth or Consequences, New Mexico 87901
P: 575-894-6673 ◆ F: 575-894-7767

www.torcnm.org

Paul Baca Commissioner

George Szigeti Commissioner

Morris Madrid City Manager

Notification Sent via Email

March 8, 2019

Aaron Beckworth, Compliance Officer Drinking Water Bureau Water Protection Division 3082 32nd Street Bypass, Suite D Silver City, NM 88061

RE: 2019 Sanitary Survey Report

T or C Municipal Airport Action Plan

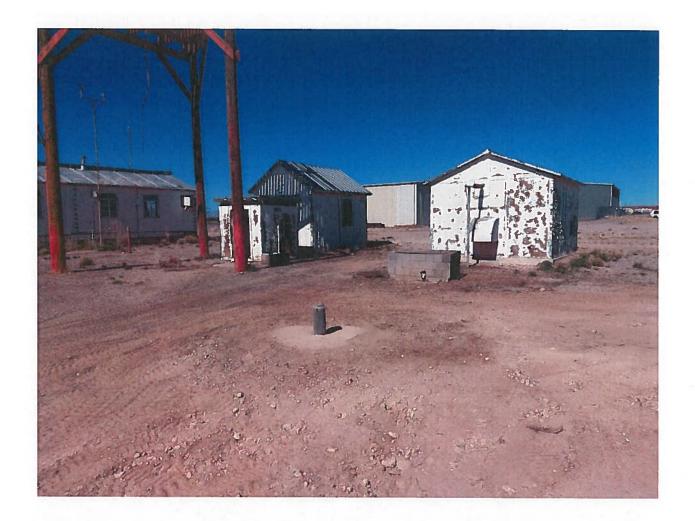
Good Afternoon Mr. Beckworth,

Please accept this letter as a formal action plan as noted and required in the T or C Municipal Airport Sanitary Survey Report 2019. The City is diligently working to correct all violations. Please see references below:

- (004C) System Management Inadequate or lack of an operations and maintenance plan or necessary operational policies. The City is in the process of revising the operations and maintenance plan for the City Water Department to include operations and maintenance of the Airport Water System. This will also include an update to the City Water Department Emergency Response Plan to include the Airport Water System.
- 2. (001E) System Management Poor housekeeping of system facilities.

 The City of T or C Water Department has a work order to disconnect and remove the sand separator and water softener and then Airport personnel will address housekeeping issues within that building. Water Department will install and bury new PVC pipe as directed. Maintenance of this will be part of the O & M Plan.
- 3. (001L) Source Wellhead is not secured from the elements or intrusion or is susceptible to flooding.

As shown in the attached picture, the Airport Personnel has begun the ground maintenance and cleanup around the well casing and exposing the concrete pad. Water will be diverted away from the well head area and a proper containment shelter will be constructed preventing potential contaminants and damage from enter the well and/or aquifer.



If you have any questions or concerns, or need additional information, please contact me at 575-894-6673.

Sincerely,

Morris Madrid - City Manager

Truth or Consequences
505 Sims Street T or C, NM 87901

575-894-6673

mmadrid@torcnm.org

SAMPLING TESTING - MUNICIPAL AIRPORT



From: Beckworth, Aaron, NMENV

 To:
 Navarro, Jesus

 Cc:
 Traci Burnette

 Subject:
 RE: [EXT] Airport

Date: Friday, January 18, 2019 11:20:00 AM

Jesus,

One bac-t sample per month for the airport. The sampling sites have been uploaded to the SDWIS database, so everything is in order. I am expecting to see a sample from the **RT001** location <u>before the end of the month</u>.

No lead and copper sampling required for the airport.

Thank you, Aaron

	Water System Detail Information	1	
Water System No.:	NM3501427	Federal Type:	NC
Water System Name:	TRUTH OR CONSEQUENCES MUNICIPAL AIRPORT	Federal Source:	GW
Principal County Served:	SIERRA	System Status:	A

R	outine TCR Sample Schedu	les
Begin/End Date	Seasonal Period	Requirements
10-01-2018 - Continuous	1/1 - 12/31	1 RT/MN

		Wa	ater System Sampl	ing Points		
		Fac	Smpl Pt ID		Desig	nations
Facility ID	Facility Name	Type Code	Type Code Status	Location	Туре	Begin/End Date
01427000	DIST	DS	RP001D - DS - A	PUBLIC RESTROOMS		
01427000	DIST	DS	RP0010 - DS - A	TERMINAL BUILDING		
01427000	DIST	DS	RP001U - DS - A	MECHANICAL ROOM		
01427000	DIST	DS	RP002D - DS - A	TRAILER HOME		
01427000	DIST	DS	RP002O - DS - A	PUBLIC RESTROOMS		
01427000	DIST	DS	RP002U - DS - A	TERMINAL BUILDING		

01427000	DIST	DS	RP003D - DS - A	LAFONT HANGER	
01427000	DIST	DS	RP003O - DS - A	TRAILER HOME	
01427000	DIST	DS	RP003U - DS - A	PUBLIC RESTROOMS	
01427000	DIST	DS	RP004D - DS - A	AUGE HANGER	
01427000	DIST	DS	RP0040 - DS - A	LAFONT HANGER	
01427000	DIST	DS	RP004U - DS - A	TRAILER HOME	
01427000	DIST	DS	RT001 - DS - A	TERMINAL BUILDING	
01427000	DIST	DS	RT002 - DS - A	PUBLIC RESTROOMS	
01427000	DIST	DS	RT003 - DS - A	TRAILER HOME	
01427000	DIST	DS	RT004 - DS - A	LAFONT HANGER	
01427000	DIST	DS	SP014270001 - DS - A	DIST	
01427001	AIRPORT WELL #1	WL	SP014270011 - EP - A	AIRPORT WELL #1	

From: Navarro, Jesus <jnavarro@torcnm.org> Sent: Friday, January 18, 2019 11:15 AM

To: Beckworth, Aaron, NMENV <Aaron.Beckworth@state.nm.us>

Cc: Traci Burnette <tburnette@torcnm.org>

Subject: [EXT] Airport

Good Morning Aaron Beckworth just so we don't drop the ball on the airport I would like to know as far as the Bac-t samples for the airport its one sample a month and is the airport going to be required to be test for led and copper or its not do to it being none community can you please let me know so we could be able to get everything we need to get thank you have a nice day



2019 SANITARY SURVEY REPORT





MICHELLE LUJAN GRISHAM Governor

> HOWIE C, MORALES Lt. Governor

State of New Mexico ENVIRONMENT DEPARTMENT

Drinking Water Bureau

3082 32nd Street Bypass, Suite D Silver City, NM 88061 Tel. 575-388-1934 • Fax 575-388-3258 www.env.nm.gov/drinking water/



JAMES C. KENNEY
Cabinet Secretary Designate

JENNIFER J. PRUETT Deputy Secretary

Notification Sent via Email

February 14, 2019

Morris Madrid Truth or Consequences Municipal Airport, NM3501427 505 Sims St Truth or Consequences, NM 87901

RE: 2019 Sanitary Survey Report

Dear Mr. Madrid,

Enclosed is a report documenting the recent sanitary survey for the Truth or Consequences Municipal Airport water system, completed on January 15, 2019 by Aaron Beckworth of the New Mexico Environment Department, Drinking Water Bureau (DWB). During the survey, three significant deficiencies were identified.

Upon receipt of this report, the Truth or Consequences Municipal Airport must consult with the DWB within 30 days of the date of this letter for all significant deficiencies (i.e., provide written documentation to DWB within 30 days of receipt of this letter stating how and when each significant deficiency will be corrected). Failure to consult with DWB within 30 days on all significant deficiencies will result in a violation of NMAC 20.7.10.100 incorporating 40 CFR Part 141 Subpart S.

Additionally, the Truth or Consequences Municipal Airport must take corrective action on all significant deficiencies and provide compliance documentation that is acceptable to DWB no later than 120 days of the date of this letter OR be in compliance with a DWB approved schedule and plan for correcting these deficiencies within 120 days of the date of this letter. Failure to correct and provide documentation of significant deficiency corrections no later than 120 days of the date of this letter will result in a treatment technique violation of NMAC 20.7.10.100 incorporating 40 CFR Part 141 Subpart S.

If you have any questions or need additional clarification concerning this report, please contact me in the Silver City office at 575-388-1934 or by e-mail at Aaron.Beckworth@state.nm.us.

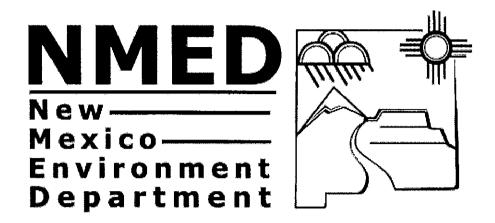
Respectfully,

Aaron Beckworth, Compliance Officer

. Bukworth

Drinking Water Bureau Water Protection Division

cc: Brandi Garcia, Southern Region Supervisor Silver City Area Office File Electronic Central File



SANITARY SURVEY REPORT

For

Truth or Consequences Municipal Airport NM3501427

Este informe contiene información importante acerca de su agua potable. Haga que alguien lo traduzca para usted, o hable con alguien que lo entienda.

Prepared by: Aaron Beckworth

New Mexico Environment Department Drinking Water Bureau Silver City Field Office 3082 32nd Street Bypass, Suite D Silver City, NM 88061

State of New Mexico Environment Department Water Protection Division Drinking Water Bureau



This sanitary survey report fulfills the requirements of New Mexico Administrative Code 20.7.10.100 incorporating 40 Code of Federal Regulations 141.21(d)(2) and 142.16(o)(2) for completing a State approved survey.

	Claron Bukworth		2/14/2019
NMED APPROVING AUTHORITY:		_ Date: _	
	Agron Reckworth, Compliance Of	ficer	

Introduction

A sanitary survey enables the New Mexico Environment Department, Drinking Water Bureau (DWB) to provide a comprehensive and accurate review of the components of a water system, to assess the operating condition and adequacy of the water system, and to determine if past recommendations have been implemented effectively. The sanitary survey encompasses eight specific elements that are evaluated during the survey. Those eight elements are listed below.

- Source (protection, physical components, and condition)
- Treatment
- Distribution system
- Finished water storage
- Pumps, pump facilities, and controls
- Monitoring, reporting, and data verification
- System management, and operation
- Operator compliance with State requirements

Each element may not be specifically mentioned within this report; however, a significant deficiency or recommendation will be noted if any issues are discovered with any of these eight (8) elements.

As part of the sanitary survey a site inspection of the Truth or Consequences Municipal Airport water system was conducted on January 15, 2019 by DWB Compliance Officer Aaron Beckworth, accompanied by DWB Community Services Coordinator Michael Montoya, Truth or Consequences Grant Coordinator Traci Burnett, the water system operator, Jesus Navarro, and Jeff Dornbush, consultant and member of the Public Utility Advisory Board. In addition to the site inspection, a review of various operational and managerial documents, and DWB compliance files for the water system was conducted.

System Description

The Truth or Consequences Municipal Airport water system is classified as a transient non-community water system according to the New Mexico Drinking Water Regulations 20.7.10 NMAC. The water system consists of one well, two pressure tanks, and a distribution system with 5 service connections.

Survey Findings

Sanitary surveys serve as a proactive public health measure and can provide important information on a water system's design and operations, can identify minor and significant deficiencies for correction before they become major problems, and can improve overall system compliance.

Significant Deficiencies

A significant deficiency is defined as any deficiency that is causing or has the potential to cause a threat to public health [New Mexico Administrative Code (NMAC) 20.7.10.100 incorporating 40 Code of Federal Regulations (CFR) §141.403(a)(4)]. Water systems must consult with the DWB within 30 days and take corrective action for any significant deficiencies found during the sanitary survey no later than 120 days after receiving written notification of such deficiencies, or be in compliance with a DWB-approved schedule and plan for correcting these deficiencies within the same 120-day period [NMAC 20.7.10.100 incorporating 40 CFR §141.403(a)(4) and §141.403(a)(5)]. Failure to remedy any significant deficiency will result in a treatment technique violation of NMAC 20.7.10.100 incorporating 40 CFR Part 141 Subpart S.

A total of three significant deficiencies were identified during the survey.

1. Deficiency: (004C) System Management - Inadequate or lack of an operations and

maintenance plan or necessary operational policies.

Regulatory Citation: NMAC 20.7.10.100, incorporating 40 CFR Part 141.403(a)(4)

Concern/Description: An operation and maintenance plan is an essential part of any water

system. The plan should summarize the actions required for cost effective, efficient, safe and reliable operation of the water system. An adequate plan should allow for a flawless transition from one operator to the next. Lacking a written plan could result in insufficient operation and maintenance of the water system as well as prolonged water outages

during emergency situations.

Corrective Action: Truth or Consequences Municipal Airport must prepare and implement

an operation and maintenance plan. A template has been developed to aid in the preparation of a written plan and can be located on the

Technical Assistance page of the DWB website.

2. Deficiency: (001E) System Management - Poor housekeeping of system facilities.

Regulatory Citation: NMAC 20.7.10.400, GENERAL OPERATING REQUIREMENTS,

Paragraph B. Security and protection of a public water system.

"Any part or component of a public water system including but not limited to spring junction boxes, well houses, storage reservoirs, collection devices, pump facilities, and treatment facilities shall be

constructed, operated and maintained to prevent:

(1) unauthorized entry to the water supply;

(2) flooding of the water supply; and (3) contamination of, the water supply."

Concern/Description: Poor housekeeping can result in safety hazards, inability to access

critical facilities, failure of system components, and possible

introduction of contaminants into the water supply.

Corrective Action: Truth or Consequences Municipal Airport must remove unused piping

and equipment, such as the sand separator and water softener; replace deteriorated PVC pipe; properly bury and/or protect newly installed PVC pipe from direct exposure to sunlight; and maintain system

facilities as part of an operation and maintenance plan.

3. Deficiency: (001L) Source - Wellhead is not secured from the elements or intrusion

or is susceptible to flooding.

Regulatory Citation: NMAC 20.7.10.400, GENERAL OPERATING REQUIREMENTS,

Paragraph C. Protection of a public water system well.

"A ground water supply well serving a public water system shall have a sanitary seal installed at the wellhead to protect against entry of storm water and other non-potable fluids or foreign materials and against access by insects, rodents, birds or other vermin. All vents installed in the well casing shall be protected against entrance of foreign material and flooding. If the well is completed in a subsurface vault, the casing shall extend above the potential flooding height. All cracks, joints or other openings at the wellhead and all penetrations to the casing at or near the ground surface shall be tightly sealed with an impermeable material."

Concern/Description:

Properly protected wellheads prevent contaminated water, insects, vermin, or other potential contaminants from entering the well and/or aquifer. Facilities that are susceptible to flooding have an increased potential for contamination by surface water.

Corrective Action:

Truth or Consequences Municipal Airport must remove the dirt mounted up around the well casing and verify the existence of a properly constructed concrete pad surrounding the wellhead as part of the required sanitary seal.

Conclusion

A sanitary survey of the Truth or Consequences Municipal Airport water system was conducted on January 15, 2019. Based upon the onsite inspection and review of various operational and managerial documents, and DWB compliance files, a total of three significant deficiencies were identified. Truth or Consequences Municipal Airport must comply with the each of the following requirements.

- Upon receipt of this report, Truth or Consequences Municipal Airport must consult with the DWB within 30 days for all significant deficiencies (i.e., provide written documentation to the DWB within 30 days of receipt of this report stating how and when each significant deficiency will be addressed).
- Truth or Consequences Municipal Airport must take corrective action on all significant deficiencies and provide compliance documentation that is acceptable to the DWB no later than 120 days after receiving written notification of such deficiencies or be in compliance with an approved schedule and plan for correcting these deficiencies within the same 120-day period.
- In addition, Truth or Consequences Municipal Airport must provide written documentation to the DWB within 30 days of completing corrective action for each significant deficiency.
- Failure to correct any significant deficiency in accordance with the previous bullet will result in a treatment technique violation of NMAC 20.7.10.100 incorporating 40 CFR Part 141 Subpart S.

If you have any questions or need additional clarification concerning this report, please call 575-388-1934 or e-mail <u>Aaron.Beckworth@state.nm.us</u>.



APPENDIX 4- CONSTRUCTION AND NON-CONSTRUCTION DETAIL COST ESTIMATE

TABLE OF CONTENT:

- o ALTERNATIVE II COMPLETE SYSTEM
- ALTERNATIVE III SYSTEM PERFORMANCE UPGRADE
- ALTERNATIVE III A SYSTEM HIGH PRESSURE SOLUTION
- ALTERNATIVE III B SYSTEM REDUNDANCY AND HYDRAULIC ENHANCEMENTS
- o ALTERNATIVE III C ADDITIONAL HYDRAULIC PERFROMANCE ENHACNECMENT
- ALTERNATIVE IV NORTH AREA
- ALTRENATIVE V − EAST AREA
- ALTERNATIVE VI WEST AREA
- ALTRENATIVE VII DOWNTOWN AREA
- o ALTERNATIVE VIII WILLIAMSBURG AREA
- O ALTERNATIVE IX AIRPORT 1 PRESSURE TANK REPLACEMENT
- ALTERNATIVE X AIRPORT 2 WITHOUT FIRE FLOW
- ALTERNATIVE XI AIRPORT 3 WITH FIRE FLOW
- AIRPORT XII AIRPORT 4 VFD WELL PUMP





ALTERNATIVE II - COMPLETE SYSTEM



	Alternative II - Complete System Open Trench Waterline				
	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST
	General	00	ζ	J 555.	
1	Mob/Demob. (5% of General Cost)	LS	1	\$2,602,278.11	\$2,602,278.11
2	Traffic Control (3.43% of General Cost)	LS	1	\$4,163,644.97	\$4,163,644.97
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$1,129,388.70	\$1,129,388.70
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$1,821,594.68	\$1,821,594.68
5	Materials Testing (0.2% of General Cost)	LS	1	\$1,040,911.24	\$1,040,911.24
6	Subsurface Utility Locating	LS	1	\$95,275.00	\$95,275.00
7	Utility Relocation	LS	1	\$95,275.00	\$95,275.00
8	AC Pipe Removal and Disposal	LS	1	\$67,454.70	\$67,454.70
	Waterline		<u>l</u>		
9	6" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	106,343	\$28.78	\$3,060,551.54
10	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	54,966	\$35.70	\$1,962,286.20
11	10" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	23,367	\$36.50	\$852,895.50
12	12" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	18,724	\$37.30	\$698,405.20
13	14"Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	15,949	\$50.77	\$809,730.73
14	Jack and Bore w/ 18-inch Casing pipe, CIP	LF	5,784	\$220.00	\$1,272,480.00
15	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	409	\$3,500.00	\$1,431,563.00
16	6" Gate Valves w/ Valve Can, CIP	EA	881	\$935.00	\$823,893.08
17	8" Gate Valves w/ Valve Can, CIP	EA	1,827	\$1,205.00	\$2,202,026.17
18	10" Gate Valves w/ Valve Can, CIP	EA	294	\$2,500.00	\$734,435.39
19	12" Gate Valves w/ Valve Can, CIP	EA	132	\$3,263.00	\$429,339.84
20	14" Gate Valves w/ Valve Can, CIP	EA	18	\$4,000.00	\$73,838.40
	Furnish and Install 1-inch Single Body Combination Air Valve w/ Traffic Rated Vault on new				
21	waterline, (including all materials, labor, excavation, rock excavation, backfill and site restoration), CIP	EA	5	\$8,000.00	\$40,000.00
22	Furnish and Install 6"x2" PRV Assembly (including PRV, vault, excavation, labor and all required	EA	1	\$60,000.00	\$60,000.00
22	appurtenances for a complete installation) Furnish and Install 10"x4" PRV Assembly (including PRV, vault, excavation, labor and all required			¢20,000,00	¢400.000.00
23	appurtenances for a complete installation)	EA	6	\$80,000.00	\$480,000.00
24	Pressurized waterline connections, CIP	EA	727	\$1,184.22	\$861,378.24
25	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	258,670	\$3.00	\$776,010.14
26	Joint Restraints, CIP	EA	11,703	\$77.75	\$909,912.45
27	1" Water Service, New single connection to existing watermain, cip. SD 2362	EA	3,139	\$1,329.00	\$4,171,731.00
28	Water Meter Box Remove & Replace	EA	3,139	\$1,000.00	\$3,139,000.00
29	Dewatering of Trench, CIP	LF	44,850	\$53.00	\$2,377,062.19
30	Valve/Pipeline abandoment	LS	1	1,093,878.88	\$1,093,878.88
31	Hydrant removal and abandonment	LS	1	173,219.12	\$173,219.12
	Water Well	ı			
32	Furnish and Install 40 HP Pump, duty point of 500 GPM at 110 PSI, CIP. With drop pipe/cable/pit less CIP	EA	1	\$50,000.00	\$50,000.00
33	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 Enclosure, CIP	EA	1	\$30,000.00	\$30,000.00
34	Building 24X30 - Complete w/ Electrical and Plumbing	SQFT	720	\$425.00	\$306,000.00
35	12" Steel Cased Potable Water Well - Drilling Complete	LF	674	\$900.00	\$606,600.00
36	Furnish and install new gas- chlorination disinfection system, CIP.	EA	1	\$165,000.00	\$165,000.00
37	8" Waterline Pipe excl. fitting, (std. spec.sec 801), icl. Trench, & compacted backfill, to 6' depth,	LF	200	\$25.00	\$5,000.00
38	cip. 8" Gate Valve, cip SD 2333	EA	3	\$1,205.00	\$3,615.00
39	6" Service Stub-Out w/ 6" Gate Valve, 100'	EA	2	\$6,000.00	\$12,000.00
	Roadway			\$0,000.00	\$12,000.00
43	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	74,949.67	\$42.00	\$3,147,886.00
	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include	SY	74,949.67	\$62.00	\$4,646,879.33
44	Subgrade Prep. CIP				
44 45	Subgrade Prep, CIP Excavate and Dispose of Unsuitable Material, CIP	CY	449,698	\$15.00	\$6,745,470.00
		CY CY	449,698 449,698	\$15.00 \$15.00	
45	Excavate and Dispose of Unsuitable Material, CIP				\$6,745,470.00
45 46	Excavate and Dispose of Unsuitable Material, CIP Import of Engineered Fill	CY	449,698	\$15.00	

\$63,061,384.5	ion Cost Subtotal:	Construct			
\$70,234,617.0	ion Cost Subtotal:	75% + Construct	R Inflation @ 11.3	5-Y	
\$7,023,462.0	ontingency - 10%:	c			
\$6,566,937.0	NMGRT @8.5%:				
\$4,610,376.0	Finance Interest:	Interin			
\$88,435,392.0	STRUCTION COST:	STIMATED CON	TOTAL		
				ENGINEERING SERVICES	
\$636,481.0	\$636,481.00	1	LS	Bridge Loan @ 5.5%	50
\$927,097.0	\$927,097.00	1	LS	Additional Engineering - Data Collection*	51
\$60,000.0	\$60,000.00	1	LS	Additional Engineering - Computer hydraulic model and calibration*	52
\$35,000.0	\$35,000.00	1	LS	Additional Engineering - Hydrogeology Well siting study *	53
\$7,725,808.0	\$7,725,808.00	1	LS	Engineering Design Services	54
\$177,694.0	\$177,694.00	1	LS	Engineering - Bid Phase	55
\$1,545,162.0	\$1,545,162.00	1	LS	Engineering - Construction Inspection	56
\$20,000.0	\$20,000.00	1	LS	Engineering-Well Construction Oversight	57
\$1,081,613.0	\$1,081,613.00	1	LS	Engineering - Construction Management	58
\$12,208,855.0	Services Subtotal:	Engineering	·		
\$1,037,753.0	NMGRT @ 8.5%:				
\$13,246,608.0	Engineering Total:				
				FINANCING SERVICES	
\$790,139.0	\$790,139.00	1	LS	Loan Origination Fee	59
\$790,139.0	Services Subtotal:	Financing			
\$67,162.0	g NMGRT @8.5%:	Financin			
\$857,301.0	gal Services Total:	Le			
				LEGAL SERVICES	
\$10,000.0	\$10,000.00	1	LS	Legal Fees - Project Attorney	60
\$21,000.0	\$21,000.00	1	LS	Legal Fees - Bond Counsel	61
\$31,000.0	Services Subtotal:	Legal			
\$2,635.0	I NMGRT @ 8.5%:	Lega	· · · · · · · · · · · · · · · · · · ·		
\$33,635.0	gal Services Total:	Le	· · · · · · · · · · · · · · · · · · ·		
\$102,572,93	GRAND TOTAL:	•			



Project No. 19-600-211-00

ALTERNATIVE III – SYSTEM PERFORMANCE UPGRADE



	Alternative III - System Performance L	Jpdate			
	Open Trench Waterline ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST
	General	UNITS	ų i i	UNIT COST	EXTEND COST
1	Mob/Demob. (5% of General Cost)	LS	1	\$680,549.74	\$680,549.74
2	Traffic Control (3.43% of General Cost)	LS	1	\$1,088,879.58	\$1,088,879.58
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$295,358.59	\$295,358.59
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$476,384.82	\$476,384.82
5	Materials Testing (0.2% of General Cost)	LS	1	\$272,219.89	\$272,219.89
6	Subsurface Utility Locating	LS	1	\$22,461.02	\$22,461.02
7	Utility Relocation	LS	1	\$22,461.02	\$22,461.02
8	AC Pipe Removal and Disposal	LS	1	\$15,902.40	\$15,902.40
_	Waterline		l	4	
9	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	11,796	\$35.70	\$421,117.20
10	10" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	22,393	\$36.50	\$817,344.50
	12" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	13,319	\$42.30	\$563,393.70
12	Jack and Bore w/ 18-inch Casing pipe, CIP	LF	947	\$220.00	\$208,340.00
13	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	79	\$3,500.00	\$277,823.00
	6" Gate Valves w/ Valve Can, CIP	EA	45	\$935.00	\$42,075.00
	8" Gate Valves w/ Valve Can, CIP	EA EA	980	\$1,205.00	\$1,181,438.34
16 17	10" Gate Valves w/ Valve Can, CIP 12" Gate Valves w/ Valve Can, CIP	EA	124 5	\$2,500.00 \$3,263.00	\$310,180.07 \$17,383.96
1/	Furnish and Install 6"x2"" PRV Assembly (including PRV, vault, excavation, labor and all required	LA	3	\$3,203.00	\$17,363.50
18	appurtenances for a complete installation)	EA	6	\$80,000.00	\$480,000.00
19	Pressurized waterline connections, CIP	EA	315	\$1,184.22	\$373,502.07
20	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	104,203	\$3.00	\$312,609.15
21	Joint Restraints, CIP	EA	4,430	\$77.75	\$344,443.85
22	1" Water Service, New single connection to existing watermian, cip. SD 2362	EA	285	\$1,329.00	\$378,765.00
23	Water Meter Box Remove & Replace	EA	285	\$1,000.00	\$285,000.00
24	Dewatering of Trench, CIP	LF	12,163	\$53.00	\$644,649.60
25	Valve/Pipeline abandonment	LS	1	\$400,731.69	\$400,731.69
26	Hydrant removal and abandonment	LS	1	\$33,616.58	\$33,616.58
	Water Well		l		
27	Furnish and Install 40 HP Pump, duty point of 500 GPM at 110 PSI, CIP. With drop pipe/cable/pit less CIP	EA	1	\$50,000.00	\$50,000.00
28	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 Enclosure, CIP	EA	1	\$30,000.00	\$30,000.00
29	Building 24X30 - Complete w/ Electrical and Plumbing	SQFT	720	\$425.00	\$306,000.00
30	12" Steel Cased Potable Water Well - Drilling Complete	LF	674	\$900.00	\$606,600.00
31	Furnish and install new gas- chlorination disinfection system, CIP.	EA	1	\$165,000.00	\$165,000.00
32	8" Waterline Pipe excl. fitting, (std. spec.sec 801), icl. Trench, & compacted backfill, to 6' depth, cip.	LF	200	\$25.00	\$5,000.00
33	8" Gate Valve, cip SD 2333	EA	3	\$1,205.00	\$3,615.00
34	6" Service Stub-Out w/ 6" Gate Valve, 100'	EA	2	\$6,000.00	\$12,000.00
	Roadway				
35	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	17,669	\$42.00	\$742,112.00
45	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP	SY	17,669	\$62.00	\$1,095,498.67
46	Excavate and Dispose of Unsuitable Material, CIP	CY	106,016	\$15.00	\$1,590,240.00
47	Import of Engineered Fill	CY	106,016	\$15.00	\$1,590,240.00
48	Geogrid Base Roadway Reinforcement	SY	17,669	\$5.50	\$97,181.33
49	Remove and replace Curb and Gutter @ Services, CIP	LF	1,140	\$25.00	\$28,500.00
50	Remove and replace Sidewalk @ Services, CIP	CY	798	\$48.00	\$38,304.00
				struction Cost Subtotal:	\$16,485,211.76
		2-YR Inflation	n @ 4.55% + Con	struction Cost Subtotal:	\$17,235,289.00
				Contingency - 10%:	\$1,723,529.00
				NMGRT @ 8.5%:	\$1,611,500.00
		7.		nterim Finance Interest:	\$1,131,367.00
		TC	AL ESTIMATE	CONSTRUCTION COST:	\$21,701,685.00

	ENGINEERING SERVICES					
51	Bridge Loan @ 5.5%	LS	1	\$184,945.00	\$184,945.00	
52	Additional Engineering - Data Collection*	LS	1	\$379,176.00	\$379,176.00	
53	Additional Engineering - Computer hydraulic model and calibration*	LS	1	\$60,000.00	\$60,000.00	
54	Additional Engineering - Hydrogeology Well siting study *	LS	1	\$35,000.00	\$35,000.00	
55	Engineering Design Services	LS	1	\$1,895,882.00	\$1,895,882.00	
56	Engineering - Bid Phase	LS	1	\$43,605.00	\$43,605.00	
57	Engineering - Construction Inspection	LS	1	\$663,559.00	\$663,559.00	
58	Engineering-Well Construction Oversight	LS	1	\$20,000.00	\$20,000.00	
59	Engineering - Construction Management	LS	1	\$265,423.00	\$265,423.00	
		•	Engine	ering Services Subtotal:	\$3,547,590.00	
				NMGRT @8.50%:	\$301,545.00	
Engineering Total:						
	FINANCING SERVICES					
60	Loan Origination Fee	LS	1	\$193,897.00	\$193,897.00	
			Fina	ncing Services Subtotal:	\$193,897.00	
			Fin	ancing NMGRT @ 8.5%:	\$16,481.00	
				Legal Services Total:	\$210,378.00	
	LEGAL SERVICES					
61	Legal Fees - Project Attorney	LS	1	\$10,000.00	\$10,000.00	
62	Legal Fees - Bond Counsel	LS	1	\$21,000.00	\$21,000.00	
		•		Legal Services Subtotal:	\$31,000.00	
				Legal NMGRT @ 8.5%:	\$2,635.00	
				Legal Services Total:	\$33,635.00	
				GRAND TOTAL:	\$25,794,833	



ALTERNATIVE III – A SYSTEM HIGH PRESSURE SOLUTION



	Alternative III A- System High Pressure S	Solution			
	Open Trench Waterline ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST
	General	ONITS	ų, i	ONIT COST	EXTEND COST
1	Mob/Demob. (5% of General Cost)	LS	1	\$194,215.79	\$194,215.7
2	Traffic Control (3.43% of General Cost)	LS	1	\$310,745.27	\$310,745.2
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$84,289.65	\$84,289.6
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$135,951.05	\$135,951.0
5	Materials Testing (0.2% of General Cost)	LS	1	\$77,686.32	\$77,686.3
6	Subsurface Utility Locating	LS	1	\$10,670.76	\$10,670.7
7	Utility Relocation	LS	1	\$10,670.76	\$10,670.7
8	AC Pipe Removal and Disposal	LS	1	\$7,554.90	\$7,554.9
	Waterline		-	\$7,55 ii.50	ψ7,55 H.S.
8	6" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	5,500	\$28.78	\$158,290.00
9	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	671	\$35.70	\$23,954.70
10	10" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	9,797	\$36.50	\$357,590.50
11	12" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	9,862	\$42.30	\$417,162.60
12	14"Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	5,552	\$50.77	\$0.00
13	Jack and Bore w/ 18-inch Casing pipe, CIP	LF	647	\$220.00	\$142,340.00
14	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	32	\$3,500.00	\$111,776.00
15	6" Gate Valves w/ Valve Can, CIP	EA	19	\$935.00	\$17,765.00
16	8" Gate Valves w/ Valve Can, CIP	EA	56	\$1,205.00	\$67,204.57
17	10" Gate Valves w/ Valve Can, CIP	EA	54	\$2,500.00	\$135,704.65
18	12" Gate Valves w/ Valve Can, CIP	EA	4	\$3,263.00	\$12,871.88
19	14" Gate Valves w/ Valve Can, CIP	EA		\$4,000.00	\$0.00
20	Furnish and Install 6"x2"" PRV Assembly (including PRV, vault, excavation, labor and all required appurtenances for a complete installation)	EA	6	\$80,000.00	\$480,000.00
21	Pressurized waterline connections, CIP	EA	36	\$1,184.22	\$43,014.43
22	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	12,001	\$3.00	\$36,001.69
23	Joint Restraints, CIP	EA	510	\$77.75	\$39,667.94
	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST
24	1" Water Service, New single connection to existing watermain, cip. SD 2362	EA	95	\$1,329.00	\$126,255.00
25	Water Meter Box Remove & Replace	EA	95	\$1,000.00	\$95,000.00
26	Dewatering of Trench, CIP	LF	4,686	\$53.00	\$248,334.15
25	Valve/Pipeline abandonment	LS	1	\$46,150.34	\$46,150.34
26	Hydrant removal and abandonment	LS	1	\$13,524.90	\$13,524.90
	Roadway				
27	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	8,394	\$42.00	\$352,562.00
28	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP	SY	8,394	\$62.00	\$520,448.6
29	Excavate and Dispose of Unsuitable Material, CIP	CY	12,342	\$15.00	\$185,130.0
30	Import of Engineered Fill	CY	12,342	\$15.00	\$185,130.0
31	Geogrid Base Roadway Reinforcement	SY	8,394	\$5.50	\$46,168.8
32	Remove and replace Curb and Gutter @ Services, CIP	LF	380	\$25.00	\$9,500.0
33	Remove and replace Sidewalk @ Services, CIP	CY	266	\$48.00	\$12,768.0
		•	Construct	tion Cost Subtotal:	\$4,716,100.3
	2-	YR Inflation @	4.55% + Construct	tion Cost Subtotal:	\$4,930,683.0
			C	Contingency - 10%:	\$493,068.0
				NMGRT @ 8.5%:	\$461,019.0
			Interin	n Finance Interest:	\$323,662.0

	ENGINEERING SERVICES				
34	Bridge Loan @ 5.5%	LS	1	\$56,899.00	\$56,899.00
35	Preliminary Engineering Report-PER	LS	1	\$35,000.00	\$35,000.00
36	Environmental w/ Report	LS	1	\$12,000.00	\$12,000.00
37	Additional Engineering - Data Collection*	LS	1	\$108,475.00	\$108,475.00
38	Additional Engineering - Computer hydraulic model and calibration*	LS	1	\$60,000.00	\$60,000.00
39	Engineering Design Services	LS	1	\$542,375.00	\$542,375.00
40	Engineering - Bid Phase	LS	1	\$12,475.00	\$12,475.00
41	Engineering - Construction Inspection	LS	1	\$235,270.00	\$235,270.00
42	Engineering - Construction Management	LS	1	\$75,933.00	\$75,933.00
Engineering Services Subtotal:					\$1,138,427.00
NMGRT @ 7.875%:					\$89,651.00
Engineering Total:					\$1,228,078.00
	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST
	ITEMS LIST FINANCING SERVICES	UNITS	QTY	UNIT COST	EXTEND COST
43		LS	QTY 1	\$55,470.00	
43	FINANCING SERVICES		1		
43	FINANCING SERVICES		1 Financing	\$55,470.00	\$55,470.00
43	FINANCING SERVICES		1 Financing	\$55,470.00 Services Subtotal:	\$55,470.00 \$55,470.00
43	FINANCING SERVICES		1 Financing	\$55,470.00 Services Subtotal: g NMGRT @ 8.5%:	\$55,470.00 \$55,470.00 \$4,715.00
43	FINANCING SERVICES Loan Origination Fee		1 Financing	\$55,470.00 Services Subtotal: g NMGRT @ 8.5%:	\$55,470.00 \$55,470.00 \$4,715.00
	Loan Origination Fee LEGAL SERVICES	LS	1 Financing Financin	\$55,470.00 Services Subtotal: g NMGRT @ 8.5%: egal Services Total:	\$55,470.00 \$55,470.00 \$4,715.00 \$60,185.00
44	Loan Origination Fee Legal Services Legal Fees - Project Attorney	LS	1 Financing Financin Le	\$55,470.00 Services Subtotal: g NMGRT @ 8.5%: egal Services Total: \$10,000.00	\$55,470.00 \$55,470.00 \$4,715.00 \$60,185.00
44	Loan Origination Fee Legal Services Legal Fees - Project Attorney	LS	Financing Financin Le 1 1 Legal	\$55,470.00 Services Subtotal: g NMGRT @ 8.5%: egal Services Total: \$10,000.00 \$21,000.00	\$55,470.00 \$55,470.00 \$4,715.00 \$60,185.00 \$10,000.00 \$21,000.00
44	Loan Origination Fee Legal Services Legal Fees - Project Attorney	LS	Financing Financin Le 1 1 Legal	\$55,470.00 Services Subtotal: g NMGRT @ 8.5%: egal Services Total: \$10,000.00 \$21,000.00 Services Subtotal:	\$55,470.00 \$55,470.00 \$4,715.00 \$60,185.00 \$10,000.00 \$21,000.00 \$31,000.00



ALTERNATIVE III – B SYSTEM REDUNDANCY AND HYDRAULIC ENHANCEMENTS



Open Trench Waterline						
	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND C	
1	General Mach/Domach /55% of Council Cost)	1.0	1	¢350,000,50	ćar	
2	Mob/Demob. (5% of General Cost) Traffic Control (2.4.2% of Conord Cost)	LS LS	1	\$259,088.50 \$414,541.60	\$25	
3	Traffic Control (3.43% of General Cost)	LS	1		\$41	
	Construction Survey/Staking (2.17% of General Cost)		1	\$112,444.41	\$1:	
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$181,361.95	\$18	
5	Materials Testing (0.2% of General Cost)	LS	1	\$103,635.40	\$10	
6	Subsurface Utility Locating	LS	1	\$6,209.75		
7	Utility Relocation	LS	1	· · ·		
8	AC Pipe Removal and Disposal Waterline	LS	1	\$4,396.50	<u> </u>	
0	1	1.5	6191	¢25.70	ຕ່ວງເ	
9	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	6181	\$35.70	\$220	
10	10" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	8774	\$36.50	\$320	
11	Jack and Bore w/ 18-inch Casing pipe, CIP	LF	300	\$220.00	\$66	
12	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	30	\$3,500.00	\$104	
13	6" Gate Valves w/ Valve Can, CIP	EA	13	\$935.00	\$12	
14	8" Gate Valves w/ Valve Can, CIP	EA	514	\$1,205.00	\$619	
15	10" Gate Valves w/ Valve Can, CIP	EA	49	\$2,500.00	\$12:	
16	Pressurized waterline connections, CIP	EA	157	\$1,184.22	\$186	
17	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	51915	\$3.00	\$15	
18	Joint Restraints, CIP	EA	2207	\$77.75	\$17:	
19	1" Water Service, New single connection to existing watermian, cip. SD 2362	EA	70	\$1,329.00	\$93	
20	Water Meter Box Remove & Replace	EA	70	\$1,000.00	\$70	
21	Dewatering of Trench, CIP	LF	4147	\$53.00	\$219	
22	Valve/Pipeline abandonment	LS	1	\$199,649.65	\$199	
23	Hydrant removal and abandonment	LS	1	\$12,666.89	\$12	
	Water Well		T	Г Т		
24	Furnish and Install 40 HP Pump, duty point of 500 GPM at 110 PSI, CIP. With drop pipe/cable/pit less CIP	EA	1	\$50,000.00	\$5	
25	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 Enclosure, CIP	EA	1	\$30,000.00	\$3	
26	Building 24X30 - Complete w/ Electrical and Plumbing	SQFT	720	\$425.00	\$30	
27	12" Steel Cased Potable Water Well - Drilling Complete	LF	674	\$900.00	\$60	
28	Furnish and install new gas- chlorination disinfection system, CIP.	EA	1	\$165,000.00	\$16	
29	8" Waterline Pipe excl. fitting, (std. spec.sec 801), icl. Trench, & compacted backfill, to 6' depth, cip.	LF	200	\$25.00	\$	
30	8" Gate Valve, cip SD 2333	EA	3	\$1,205.00	Ç	
31	6" Service Stub-Out w/ 6" Gate Valve, 100'	EA	2	\$6,000.00	\$1	
	Roadway					
32	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	4885	\$42.00	\$20	
33	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP	SY	4885	\$62.00	\$30	
34	Excavate and Dispose of Unsuitable Material, CIP	CY	29310	\$15.00	\$43	
35	Import of Engineered Fill	CY	29310	\$15.00	\$43	
36	Geogrid Base Roadway Reinforcement	SY	4885	\$5.50	\$2	
37	Remove and replace Curb and Gutter @ Services, CIP	LF	280	\$25.00	Ç	
38	Remove and replace Sidewalk @ Services, CIP	CY	196	\$48.00	\$6,26	
Construction Cost Subtotal:						
2-YR Inflation @ 4.55% + Construction Cost Subtotal:						
Contingency - 10%:					\$65 \$65	
	NMGRT @ 8.5%:					
Interim Finance Interest: TOTAL ESTIMATED CONSTRUCTION COST:				ance interest:	\$4	

	ENGINEERING SERVICES				
39	Bridge Loan @ 5.5%	LS	1	\$70,958.00	\$70,958.00
40	Additional Engineering - Data Collection*	LS	1	\$144,208.00	\$144,208.00
41	Additional Engineering - Hydrogeology Well siting study *	LS	1	\$35,000.00	\$35,000.00
42	Engineering Design Services	LS	1	\$721,042.00	\$721,042.00
43	Engineering - Bid Phase	LS	1	\$16,584.00	\$16,584.00
44	Engineering - Construction Inspection	LS	1	\$252,365.00	\$252,365.00
45	Engineering-Well Construction Oversight	LS	1	\$20,000.00	\$20,000.00
46	Engineering - Construction Management	LS	1	\$100,946.00	\$100,946.00
		En	gineering Serv	vices Subtotal:	\$1,361,103.00
			NN	MGRT @ 8.5%:	\$115,694.00
			Engi	neering Total:	\$1,476,797.00
	FINANCING SERVICES				
47	Loan Origination Fee	LS	1	\$73,743.00	\$73,743.00
			Financing Serv	vices Subtotal:	\$73,743.00
			Financing NN	/IGRT @ 8.5%:	\$6,268.00
			Legal S	Services Total:	\$80,011.00
	LEGAL SERVICES				
48	Legal Fees - Project Attorney	LS	1	\$10,000.00	\$10,000.00
49	Legal Fees - Bond Counsel	LS	1	\$21,000.00	\$21,000.00
			Legal Serv	vices Subtotal:	\$31,000.00
			Legal NN	MGRT @ 8.5%:	\$2,635.00
			Legal S	Services Total:	\$33,635.00
			<u>G</u>	RAND TOTAL:	<u>\$9,844,031</u>



ALTERNATIVE III – C ADDITIONAL HYDRAULIC PERFROMANCE ENHACNECMENT



	Alternative III-C Additional Hydraulic Performance En	hancements			
	Open Trench Waterline				
	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST
	General	ı			
1	Mob/Demob. (5% of General Cost)	LS	1	\$165,640.17	\$165,640.17
2	Traffic Control (3.43% of General Cost)	LS	1	\$265,024.27	\$265,024.27
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$71,887.83	\$71,887.83
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$115,948.12	\$115,948.12
5	Materials Testing (0.2% of General Cost)	LS	1	\$66,256.07	\$66,256.07
6	Subsurface Utility Locating	LS	1	\$5,179.24	\$5,179.24
7	Utility Relocation	LS	1	\$5,179.24	\$5,179.24
8	AC Pipe Removal and Disposal	LS	1	\$3,666.90	\$3,666.90
	Waterline	1			
9	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	4,944	\$35.70	\$176,500.80
10	10" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	3,822	\$36.50	\$139,503.00
11	12" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	3,457	\$42.30	\$146,231.10
15	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	18	\$3,500.00	\$61,362.00
16	6" Gate Valves w/ Valve Can, CIP	EA	13	\$935.00	\$12,155.00
17	8" Gate Valves w/ Valve Can, CIP	EA	411	\$1,205.00	\$495,170.49
18	10" Gate Valves w/ Valve Can, CIP	EA	21	\$2,500.00	\$52,941.02
19	12" Gate Valves w/ Valve Can, CIP	EA	1	\$3,263.00	\$4,512.08
24	Pressurized waterline connections, CIP	EA	122	\$1,184.22	\$144,404.14
25	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	40,287	\$3.00	\$120,861.59
26	Joint Restraints, CIP	EA	1,713	\$77.75	\$133,169.59
27	1" Water Service, New single connection to existing watermian, cip. SD 2362	EA	120	\$1,329.00	\$159,480.00
28	Water Meter Box Remove & Replace	EA	120	\$1,000.00	\$120,000.00
29	Dewatering of Trench, CIP	LF	3,330	\$53.00	\$176,508.55
30	Valve/Pipeline abandonment	LS	1	\$154,931.71	\$154,931.71
31	Hydrant removal and abandonment	LS	1	\$7,424.80	\$7,424.80
	Roadway				
32	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	4,074	\$42.00	\$171,122.00
33	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP	SY	4,074	\$62.00	\$252,608.67
34	Excavate and Dispose of Unsuitable Material, CIP	CY	24,446	\$15.00	\$366,690.00
35	Import of Engineered Fill	CY	24,446	\$15.00	\$366,690.00
36	Geogrid Base Roadway Reinforcement	SY	4,074	\$5.50	\$22,408.83
37	Remove and replace Curb and Gutter @ Services, CIP	LF	480	\$25.00	\$12,000.00
38	Remove and replace Sidewalk @ Services, CIP	CY	336	\$48.00	\$16,128.00
				on Cost Subtotal:	\$4,011,585.20
	2-YR Inf	flation @ 4.55	% + Constructi	on Cost Subtotal:	\$4,194,112.00
			Co	ontingency - 10%:	\$419,411.00
				NMGRT @ 8.5%:	\$392,149.00
Interim Finance Interest:			\$275,312.00		
		TOTAL EST	IMATED CONS	TRUCTION COST:	\$5,280,984.00

	ENGINEE	RING SERVICES			
39	Bridge Loan @ 5.5%	LS	1	\$43,466.00	\$43,466.00
40	Additional Engineering - Data Collection*	LS	1	\$92,270.00	\$92,270.00
41	Engineering Design Services	LS	1	\$461,352.00	\$461,352.00
42	Engineering - Bid Phase	LS	1	\$10,611.00	\$10,611.00
43	Engineering - Construction Inspection	LS	1	\$161,473.00	\$161,473.00
44	Engineering - Construction Management	LS	1	\$64,589.00	\$64,589.00
			Engineering	Services Subtotal:	\$833,761.00
				NMGRT @ 8.5%:	\$70,870.00
				Engineering Total:	\$904,631.00
	FINANC	ING SERVICES			
45	Loan Origination Fee	LS	1	\$47,184.00	\$47,184.00
			Financing	Services Subtotal:	\$47,184.00
			Financing	g NMGRT @ 8.5%:	\$4,011.00
			Le	gal Services Total:	\$51,195.00
	LEGA	L SERVICES			
46	Legal Fees - Project Attorney	LS	1	\$10,000.00	\$10,000.00
47	Legal Fees - Bond Counsel	LS	1	\$21,000.00	\$21,000.00
			Legal	Services Subtotal:	\$31,000.00
			Lega	I NMGRT @ 8.5%:	\$2,635.00
			Le	gal Services Total:	\$33,635.00
				GRAND TOTAL:	\$6,270,445



ALTERNATIVE IV – NORTH AREA



ITEMS LIST		Alternative IV - North Side Open Trench Waterline				
		•	LINITS	OTV	LINIT COST	EXTEND COST
Mode/Demob. 158 of General Cost 15			UNITS	QIT	UNIT COST	EXTEND COST
Traffic Control (BASN of General Cost)	1		LS	1	\$231,489,60	\$231,489,60
S						
SWPPP Preparation, Implementation, and Inspection (15x of General Cost)						
Substrace Utility Locating						
Substrace Utility Locating	5	Materials Testing (0.2% of General Cost)	LS	1	\$92,595.84	\$92,595.84
	6		LS			
Section Proceedings Proceedings Process Proces	7		LS	1		
9 6° Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill 10 8° Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill 11 12° Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill 11 12° Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill 11 12° Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill 11 12° Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill 12 14° Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill 13 Jack and Bore w/ 18-inch Casing pipe, CIP 14 15 555 \$22000 \$117,766.00 15 17 16° Gate Valves w/ Valve Casing pipe, CIP 15 55 \$22000 \$117,766.00 15 5° Gate Valves w/ Valve Can, CIP 16 A 128 \$935.00 \$119,680.00 16 8° Gate Valves w/ Valve Can, CIP 17 10° Gate Valves w/ Valve Can, CIP 18 A 21 \$3,0000 \$521,3279.80 18 12° Gate Valves w/ Valve Can, CIP 18 A 2 \$3,0000 \$50,000.00 18 12° Gate Valves w/ Valve Can, CIP 18 A 2 \$3,0000 \$50,000.00 18 12° Gate Valves w/ Valve Can, CIP 18 A 2 \$3,0000 \$50,000.00 19 12° Gate Valves w/ Valve Can, CIP 18 A 2 \$3,0000 \$50,000.00 19 12° Gate Valves w/ Valve Can, CIP 19 EA 2 \$3,0000 \$50,000.00 19 12° Gate Valves w/ Valve Can, CIP 19 EA 2 \$3,0000 \$50,000.00 19 12° Gate Valves w/ Valve Can, CIP 19 EA 2 \$3,0000 \$50,000.00 19 Presourtied waterline connections, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 16° Gate Valves w/ Valve Can, CIP 19 EA 17,733 \$1,0000 \$3,0000	8	AC Pipe Removal and Disposal	LS	1	\$3,618.90	\$3,618.90
10 8 Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill LF 3,115 \$35.70 \$111,205.50 11 12" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill LF 872 \$37.30 \$32,525.60 12 14"Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill LF 6,149 \$50.77 \$312,184.73 13 12		Waterline				
11 12 Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill LF 872 \$37.30 \$32,525.60 12 14 "Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill LF 6,149 \$50.77 \$312,184.73 13 Jack and Bore w/ 18 Inch Casing pipe, CIP LF 535 \$22.00 \$117,766.00 14 4-1/2 Fire Hydrant w/ piping valves, and connection EA 10 \$3,500.00 \$33,294.00 15 6" Gate Valves w/ Valve Can, CIP EA 128 \$935.00 \$5119,680.00 16 8" Gate Valves w/ Valve Can, CIP EA 438 \$1,205.00 \$52,1329.86 17 10" Gate Valves w/ Valve Can, CIP EA 2 \$2,500.00 \$5,000.00 18 12" Gate Valves w/ Valve Can, CIP EA 2 \$2,500.00 \$5,000.00 18 12" Gate Valves w/ Valve Can, CIP EA 2 \$2,500.00 \$5,000.00 19 14" Gate Valves w/ Valve Can, CIP EA 2 \$4,000.00 \$9,838.40 10 12" Gate Valves w/ Valve Can, CIP EA 2 \$4,000.00 \$9,838.40 10 12" Gate Valves w/ Valve Can, CIP EA 2 \$4,000.00 \$9,838.40 10 12" Gate Valves w/ Valve Can, CIP EA 2 \$4,000.00 \$9,838.40 10 12" Gate Valves w/ Valve Can, CIP EA 2 \$4,000.00 \$9,838.40 12 10 12" Gate Valves w/ Valve Can, CIP EA 167 \$1,184.22 \$197,680.48 12 10 12" Gate Valves w/ Valve Can, CIP EA 167 \$1,184.22 \$197,680.48 12 10 10 10 10 10 10 10	9	6" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	1,927	\$28.78	\$55,459.06
12 14"Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	10	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	3,115	\$35.70	\$111,205.50
13 Jack and Bore w/18-inch Casing pipe, CIP	11	12" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	872	\$37.30	\$32,525.60
4-1/2' Fire Hydrant w/ piping valves, and connection	12	14"Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	6,149	\$50.77	\$312,184.73
14	13	Jack and Bore w/ 18-inch Casing pipe, CIP	LF	535	\$220.00	\$117,766.00
16	14		EA	10	\$3,500.00	\$35,294.00
17	15	6" Gate Valves w/ Valve Can, CIP	EA	128	\$935.00	\$119,680.00
18 12" Gate Valves w/ Valve Can, CIP	16	8" Gate Valves w/ Valve Can, CIP	EA	433	\$1,205.00	\$521,329.86
19	17	10" Gate Valves w/ Valve Can, CIP	EA	2	\$2,500.00	\$5,000.00
Pressurized waterline connections, CIP	18	12" Gate Valves w/ Valve Can, CIP	EA	2	\$3,263.00	\$8,025.67
21 Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	19	14" Gate Valves w/ Valve Can, CIP	EA	2	\$4,000.00	\$9,838.40
22 Joint Restraints, CIP	20	Pressurized waterline connections, CIP	EA	167	\$1,184.22	\$197,680.48
1" Water Service, New single connection to existing watermian, cip. SD 2362 EA 73 \$1,329.00 \$97,017.00 \$24 Water Meter Box Remove & Replace EA 73 \$1,000.00 \$73,000.00 \$73,000.00 \$25 Dewatering of Trench, CIP	21	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	67,272	\$3.00	\$201,816.98
Water Meter Box Remove & Replace	22	Joint Restraints, CIP	EA	1,793	\$77.75	\$139,382.74
25 Dewatering of Trench, CIP	23	1" Water Service, New single connection to existing watermian, cip. SD 2362	EA	73	\$1,329.00	\$97,017.00
26 Valve/Pipeline abandonment	24	Water Meter Box Remove & Replace	EA	73	\$1,000.00	\$73,000.00
Each Section	25	Dewatering of Trench, CIP	LF	603	\$53.00	\$31,966.95
Section Sect	26	Valve/Pipeline abandonment	LS	1	\$196,942.66	\$196,942.66
Furnish and Install 40 HP Pump, duty point of 500 GPM at 110 PSI, CIP. With drop pipe/cable/pit less CIP \$50,000.00	27	Hydrant removal and abandonment	LS	1	\$4,270.57	\$4,270.57
Pipe/cable/pit less CIP Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 EA 1 \$30,000.00 \$30,000.00		Water Well				
Enclosure, CIP	28		EA	1	\$50,000.00	\$50,000.00
12" Steel Cased Potable Water Well - Drilling Complete	29	· · · · · · · · · · · · · · · · · · ·	EA	1	\$30,000.00	\$30,000.00
Sum	30	Building 24X30 - Complete w/ Electrical and Plumbing	SQFT	720	\$425.00	\$306,000.00
S" Waterline Pipe excl. fitting, (std. spec.sec 801), icl. Trench, & compacted backfill, to 6' depth, cip.	31	12" Steel Cased Potable Water Well - Drilling Complete	LF	674	\$900.00	\$606,600.00
Section Sect	32	Furnish and install new gas- chlorination disinfection system, CIP.	EA	1	\$165,000.00	\$165,000.00
Service Stude Color Stude St	33		LF	200	\$25.00	\$5,000.00
Service Stub-Out w/ 6" Gate Valve, 100' EA 2 \$6,000.00 \$12,000.00	34		EA	3	\$1,205.00	\$3,615.00
Roadway Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP SY 4,021 \$42.00 \$168,882.00 \$249,302.00 \$168,882.00 \$249,302.00 \$249,302.00 \$249,302.00 \$249,302.00 \$38 Excavate and Dispose of Unsuitable Material, CIP CY 24,126 \$15.00 \$361,890.00 \$39 Import of Engineered Fill CY 24,126 \$15.00 \$361,890.00 \$40 Geogrid Base Roadway Reinforcement SY 4,021 \$5.50 \$22,115.50 \$41 Remove and replace Curb and Gutter @ Services, CIP LF 292 \$25.00 \$7,300.00		· · · · · · · · · · · · · · · · · · ·		2		. ,
Streets, include Subgrade Prep, CIP Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP Excavate and Dispose of Unsuitable Material, CIP Import of Engineered Fill Geogrid Base Roadway Reinforcement Remove and replace Curb and Gutter @ Services, CIP SY 4,021 \$42.00 \$168,882.00 \$249,302.00 \$249,302.00 \$249,302.00 \$361,890.00 \$361,890.00 \$40 Excavate and Dispose of Unsuitable Material, CIP CY 24,126 \$15.00 \$361,890.00 \$361,890.00 \$40 Excavate and Peplace Curb and Gutter @ Services, CIP LF 292 \$25.00 \$7,300.00					, .,	, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP 38 Excavate and Dispose of Unsuitable Material, CIP CY 24,126 \$15.00 \$361,890.00 39 Import of Engineered Fill CY 24,126 \$15.00 \$361,890.00 40 Geogrid Base Roadway Reinforcement SY 4,021 \$5.50 \$322,115.50 41 Remove and replace Curb and Gutter @ Services, CIP LF 292 \$25.00 \$7,300.00	36		SY	4,021	\$42.00	\$168,882.00
38 Excavate and Dispose of Unsuitable Material, CIP CY 24,126 \$15.00 \$361,890.00 39 Import of Engineered Fill CY 24,126 \$15.00 \$361,890.00 40 Geogrid Base Roadway Reinforcement SY 4,021 \$5.50 \$22,115.50 41 Remove and replace Curb and Gutter @ Services, CIP LF 292 \$25.00 \$7,300.00	37	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets,	SY	4,021	\$62.00	\$249,302.00
40 Geogrid Base Roadway Reinforcement SY 4,021 \$5.50 \$22,115.50 41 Remove and replace Curb and Gutter @ Services, CIP LF 292 \$25.00 \$7,300.00	38	• 1	CY	24,126	\$15.00	\$361,890.00
41 Remove and replace Curb and Gutter @ Services, CIP LF 292 \$25.00 \$7,300.00	39	Import of Engineered Fill	CY	24,126	\$15.00	\$361,890.00
	40	Geogrid Base Roadway Reinforcement	SY	4,021	\$5.50	\$22,115.50
	41	Remove and replace Curb and Gutter @ Services, CIP	LF	292	\$25.00	\$7,300.00
	42	Remove and replace Sidewalk @ Services, CIP	CY	204	\$48.00	\$9,811.20

\$5,600,611.68	tion Cost Subtotal:	Construc					
\$5,855,440.00	tion Cost Subtotal:	2-YR Inflation @ 4.55% + Construction Cost Subtotal:					
\$585,544.00	Contingency - 10%:	Contingency - 10%:					
\$547,484.00	NMGRT @ 8.5%:						
\$384,366.00	m Finance Interest:	Interir					
\$7,372,834.00	NSTRUCTION COST:	ESTIMATED CON	TOTAL E				
			S	ENGINEERING SERVICE			
\$67,009.00	\$67,009.00	1	LS	Bridge Loan @ 5.5%	43		
\$128,820.00	\$128,820.00	1	LS	Additional Engineering - Data Collection*	44		
\$60,000.00	\$60,000.00	1	LS	Additional Engineering - Computer hydraulic model and calibration*	45		
\$35,000.00	\$35,000.00	1	LS	Additional Engineering - Hydrogeology Well siting study *	46		
\$644,098.00	\$644,098.00	1	LS	Engineering Design Services	47		
\$14,814.00	\$14,814.00	1	LS	Engineering - Bid Phase	48		
\$225,434.00	\$225,434.00	1	LS	Engineering - Construction Inspection	49		
\$20,000.00	\$20,000.00	1	LS	Engineering-Well Construction Oversight	50		
\$90,174.00	\$90,174.00	1	LS	Engineering - Construction Management	51		
\$1,285,349.00	Services Subtotal:	Engineering					
\$109,255.00	NMGRT @ 8.5%:						
\$1,394,604.00	Engineering Total:						
				FINANCING SERVICES			
\$65,874.00	\$65,874.00	1	LS	Loan Origination Fee	52		
\$65,874.00	Services Subtotal:	Financing					
\$5,599.00	ng NMGRT @ 8.5%:	Financin					
\$71,473.00	egal Services Total:	Le					
				LEGAL SERVICES			
\$10,000.00	\$10,000.00	1	LS	Legal Fees - Project Attorney	53		
\$21,000.00	\$21,000.00	1	LS	Legal Fees - Bond Counsel	54		
\$31,000.00	l Services Subtotal:	Legal					
\$2,635.00	al NMGRT @ 8.5%:	Leg					
\$33,635.00	egal Services Total:	Le					
\$8,872,546	GRAND TOTAL:						



ALTRENATIVE V – EAST AREA



	Alternative V - East Side				
	Open Trench Waterline				
	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST
	General Services		1 .1	****	4550 ==+ 0=
1	Mob/Demob. (5% of General Cost)	LS	1	\$662,571.27	\$662,571.27
2	Traffic Control (3.43% of General Cost)	LS	1	\$1,060,114.03	\$1,060,114.03
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$287,555.93	\$287,555.93
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$463,799.89	\$463,799.89
5	Materials Testing (0.2% of General Cost)	LS	1	\$265,028.51	\$265,028.51
6	Subsurface Utility Locating	LS	1	\$23,132.20	\$23,132.20
7	Utility Relocation	LS	1	\$23,132.20	\$23,132.20
8	AC Pipe Removal and Disposal	LS	1	\$16,377.60	\$16,377.60
	Waterline	1.5	25 420	620.70	Ć1 010 C7E 10
9	6" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	35,430	\$28.78	\$1,019,675.40
10	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	19,162	\$35.70	\$684,083.40
12	Jack and Bore w/ 18-inch Casing pipe, CIP	LF	804	\$220.00	\$176,792.00
13	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	109	\$3,500.00	\$382,144.00
13	6" Gate Valves w/ Valve Can, CIP	EA	219	\$935.00	\$204,794.43
14	8" Gate Valves w/ Valve Can, CIP	EA	140	\$1,205.00	\$168,367.15
14	10" Gate Valves w/ Valve Can, CIP	EA	33	\$2,500.00	\$82,500.00
15	12" Gate Valves w/ Valve Can, CIP	EA	2	\$3,263.00	\$6,526.00
16	Furnish and Install 6" PRV Assembly (including PRV, vault, excavation, labor and all required appurtenances for a complete installation)	EA	1	\$44,000.00	\$44,000.00
17	Pressurized waterline connections, CIP	EA	71	\$1,184.22	\$83,593.11
18	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	21,965	\$3.00	\$65,896.10
19	Joint Restraints, CIP	EA	1,517	\$77.75	\$117,912.88
20	1" Water Service, New single connection to existing watermian, cip. SD 2362	EA	1,128	\$1,329.00	\$1,499,112.00
21	Water Meter Box Remove & Replace	EA	1,128	\$1,000.00	\$1,128,000.00
22	Dewatering of Trench, CIP	LF	13,102	\$53.00	\$694,410.24
23	Valve/Pipeline abandonment	LS	1	\$136,633.07	\$136,633.07
24	Hydrant removal and abandonment	LS	1	\$46,239.42	\$46,239.42
	Water Well				
25	Furnish and Install 40 HP Pump, duty point of 500 GPM at 110 PSI, CIP. With drop pipe/cable/pit less CIP	EA	1	\$50,000.00	\$50,000.00
26	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 Enclosure, CIP	EA	1	\$30,000.00	\$30,000.00
27	Building 24X30 - Complete w/ Electrical and Plumbing	SQFT	720	\$425.00	\$306,000.00
28	12" Steel Cased Potable Water Well - Drilling Complete	LF	674	\$900.00	\$606,600.00
29	Furnish and install new gas- chlorination disinfection system, CIP.	EA	1	\$165,000.00	\$165,000.00
30	8" Waterline Pipe excl. fitting, (std. spec.sec 801), icl. Trench, & compacted backfill, to 6' depth, CIP.	LF	200	\$25.00	\$5,000.00
31	8" Gate Valve, cip SD 2333	EA	3	\$1,205.00	\$3,615.00
32	6" Service Stub-Out w/ 6" Gate Valve, 100'	EA	2	\$6,000.00	\$12,000.00
	Roadway				
33	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	18,197	\$42.00	\$764,288.00
34	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP	SY	18,197	\$62.00	\$1,128,234.67
35	Excavate and Dispose of Unsuitable Material, CIP	CY	109,184	\$15.00	\$1,637,760.00
36	Import of Engineered Fill	CY	109,184	\$15.00	\$1,637,760.00
37	Geogrid Base Roadway Reinforcement	SY	18,197	\$5.50	\$100,085.33
38	Remove and replace Curb and Gutter @ Services, CIP	LF	4,512	\$25.00	\$112,800.00
39	Remove and replace Sidewalk @ Services, CIP	CY	3,158	\$48.00	\$151,603.20

\$16,053,137.04	ction Cost Subtotal:				
\$16,783,555.00	ction Cost Subtotal:	55% + Constru	R Inflation @ 4.	2-ү	
\$1,678,356.00	Contingency - 10%:				
\$1,569,262.00	NMGRT @ 8.5%:				
\$1,101,715.00	m Finance Interest:	Interi			
\$21,132,888.00	NSTRUCTION COST:	STIMATED COI	TOTAL E		
			ES	ENGINEERING SERVIC	
\$180,264.00	\$180,264.00	1	LS	Bridge Loan @ 5.5%	40
\$369,238.00	\$369,238.00	1	LS	Additional Engineering - Data Collection*	41
\$60,000.00	\$60,000.00	1	LS	Additional Engineering - Computer hydraulic model and calibration*	42
\$35,000.00	\$35,000.00	1	LS	Additional Engineering - Hydrogeology Well siting study *	43
\$1,846,191.00	\$1,846,191.00	1	LS	Engineering Design Services	44
\$42,462.00	\$42,462.00	1	LS	Engineering - Bid Phase	45
\$646,167.00	\$646,167.00	1	LS	Engineering - Construction Inspection	46
\$20,000.00	\$20,000.00	1	LS	Engineering-Well Construction Oversight	47
\$258,467.00	\$258,467.00	1	LS	Engineering - Construction Management	48
\$3,457,789.00	g Services Subtotal:	Engineering			
\$293,912.00	NMGRT @ 8.5%:				
\$3,751,701.00	Engineering Total:				
			S	FINANCING SERVICE	
\$188,815.00	\$188,815.00	1	LS	Loan Origination Fee	49
\$188,815.00	g Services Subtotal:	Financing			
\$16,049.00	ng NMGRT @ 8.5%:	Financii			
\$204,864.00	egal Services Total:	L			
				LEGAL SERVICES	
\$10,000.00	\$10,000.00	1	LS	Legal Fees - Project Attorney	50
\$21,000.00	\$21,000.00	1	LS	Legal Fees - Bond Counsel	51
\$31,000.00	l Services Subtotal:	Lega			
\$2,635.00	al NMGRT @ 8.5%:	Leg			
\$33,635.00	egal Services Total:	L			
<u>\$25,123,088</u>	GRAND TOTAL:				



ALTERNATIVE VI – WEST AREA



	Alternative VI - West Si				
	Open Trench Waterlin				
	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST
	General	I	. 1		
1	Mob/Demob. (5% of General Cost)	LS	1	\$408,264.73	\$408,264.73
2	Traffic Control (3.43% of General Cost)	LS	1	\$653,223.57	\$653,223.57
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$177,186.89	\$177,186.89
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$285,785.31	\$285,785.31
5	Materials Testing (0.2% of General Cost)	LS	1	\$163,305.89	\$163,305.89
6	Subsurface Utility Locating	LS	1	\$14,112.29	\$14,112.29
7	Utility Relocation	LS	1	\$14,112.29	\$14,112.29
8	AC Pipe Removal and Disposal	LS	1	\$9,991.50	\$9,991.50
	Waterline	T			
9	6" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	21,979	\$28.78	\$632,555.62
10	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	11,326	\$35.70	\$404,338.20
11	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	67	\$3,500.00	\$233,135.00
12	6" Gate Valves w/ Valve Can, CIP	EA	192	\$935.00	\$179,699.87
13	8" Gate Valves w/ Valve Can, CIP	EA	206	\$1,205.00	\$248,777.20
14	12" Gate Valves w/ Valve Can, CIP	EA	4	\$3,263.00	\$13,052.00
16	Pressurized waterline connections, CIP	EA	73	\$1,184.22	\$86,456.74
17	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	22,157	\$3.00	\$66,469.86
18	Joint Restraints, CIP	EA	1,571	\$77.75	\$122,126.88
19	1" Water Service, New single connection to existing watermian, cip. SD 2362	EA	597	\$1,329.00	\$793,413.00
20	Water Meter Box Remove & Replace	EA	597	\$1,000.00	\$597,000.00
21	Dewatering of Trench, CIP	LF	1,665	\$53.00	\$88,258.25
22	Valve/Pipeline abandonment	LS	1	\$139,718.33	\$139,718.33
23	Hydrant removal and abandonment	LS	1	\$28,209.34	\$28,209.34
	Water Well				
24	Furnish and Install 40 HP Pump, duty point of 500 GPM at 110 PSI, CIP. With drop pipe/cable/pit less CIP	EA	1	\$50,000.00	\$50,000.00
25	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 Enclosure, CIP	EA	1	\$30,000.00	\$30,000.00
26	Building 24X30 - Complete w/ Electrical and Plumbing	SQFT	720	\$425.00	\$306,000.00
27	12" Steel Cased Potable Water Well - Drilling Complete	LF	674	\$900.00	\$606,600.00
28	Furnish and install new gas- chlorination disinfection system, CIP.	EA	1	\$165,000.00	\$165,000.00
29	8" Waterline Pipe excl. fitting, (std. spec.sec 801), icl. Trench, & compacted	LF	200	\$25.00	\$5,000.00
	backfill, to 6' depth, cip.			·	
30	8" Gate Valve, cip SD 2333	EA	3	\$1,205.00	\$3,615.00
31	6" Service Stub-Out w/ 6" Gate Valve, 100'	EA	2	\$6,000.00	\$12,000.00
	Roadway				
32	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	11,102	\$42.00	\$466,270.00
33	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP	SY	11,102	\$62.00	\$688,303.33
34	Excavate and Dispose of Unsuitable Material, CIP	CY	66,610	\$15.00	\$999,150.00
35	Import of Engineered Fill	CY	66,610	\$15.00	\$999,150.00
36	Geogrid Base Roadway Reinforcement	SY	11,102	\$5.50	\$61,059.17
37	Remove and replace Curb and Gutter @ Services, CIP	LF	2,388	\$25.00	\$59,700.00
38	Remove and replace Sidewalk @ Services, CIP	CY	1672	\$48.00	\$80,236.80
		1		ion Cost Subtotal:	\$9,891,277.05
	2-YR I	nflation @ 4.5		ion Cost Subtotal:	\$10,341,330.00
			C	ontingency - 10%:	\$1,034,133.00
				NMGRT @ 8.5%:	\$966,914.00
			Interim	Finance Interest:	\$678,831.00
		TOTAL E	STIMATED CON	STRUCTION COST:	\$13,021,208.00

	ENGINEERING SERV	ICES			
39	Bridge Loan @ 5.5%	LS	1	\$113,499.00	\$113,499.
40	Additional Engineering - Data Collection*	LS	1	\$227,509.00	\$227,509.
41	Additional Engineering - Computer hydraulic model and calibration*	LS	1	\$60,000.00	\$60,000.
42	Additional Engineering - Hydrogeology Well siting study *	LS	1	\$35,000.00	\$35,000
43	Engineering Design Services	LS	1	\$1,137,546.00	\$1,137,546
44	Engineering - Bid Phase	LS	1	\$26,164.00	\$26,164
45	Engineering - Construction Inspection	LS	1	\$398,141.00	\$398,141
46	Engineering-Well Construction Oversight	LS	1	\$20,000.00	\$20,000
47	Engineering - Construction Management	LS	1	\$159,256.00	\$159,256
			Engineering	Services Subtotal:	\$2,177,115
				NMGRT @ 8.5%:	\$185,055
				Engineering Total:	\$2,362,170
	FINANCING SERVI	CES			
48	Loan Origination Fee	LS	1	\$116,340.00	\$116,340
			Financing	Services Subtotal:	\$116,340
			Financin	g NMGRT @ 8.5%:	\$9,889
			Le	gal Services Total:	\$126,229
	LEGAL SERVICES	;			
49	Legal Fees - Project Attorney	LS	1	\$10,000.00	\$10,000
50	Legal Fees - Bond Counsel	LS	1	\$21,000.00	\$21,000
Legal Services Subtotal:					\$31,000
Legal NMGRT @ 8.5%:					\$2,635
Legal Services Total:					\$33,635
GRAND TOTAL:				\$15,543,2	



ALTRENATIVE VII – DOWNTOWN AREA



	Alternative VII - Downtown				
	Open Trench Waterline ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST
	General	OMITS	Q.I.	ONIT COST	EXTEND COST
1	Mob/Demob. (5% of General Cost)	LS	1	\$381,999.53	\$381,999.53
2	Traffic Control (3.43% of General Cost)	LS	1	\$611,199.25	\$611,199.25
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$152,799.81	\$152,799.81
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$267,399.67	\$267,399.67
5	Materials Testing (0.2% of General Cost)	LS	1	\$152,799.81	\$152,799.81
6	Subsurface Utility Locating	LS	1	\$10,572.03	\$10,572.03
7	Utility Relocation	LS	1	\$10,572.03	\$10,572.03
8	AC Pipe Removal and Disposal	LS	1	\$7,485.00	\$7,485.00
	Waterline			Ţ.,	+1,155.15
9	6" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	15,694	\$28.78	\$451,673.32
10	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	3,749	\$35.70	\$133,839.30
11	10" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	974	\$36.50	\$35,551.00
12	12" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	4,533	\$37.30	\$169,080.90
13	lack and Boro w/ 19 inch Casing nino. CIP	LF	431	\$220.00	\$94,886.00
14	Jack and Bore w/ 18-inch Casing pipe, CIP 4-1/2' Fire Hydrant w/ piping valves, and connection	EA	50	\$3,500.00	\$174,650.00
15	6" Gate Valves w/ Valve Can, CIP	EA	99	\$935.00	\$92,471.59
16	8" Gate Valves w/ Valve Can, CIP	EA	37	\$1,205.00	\$44,299.76
17	10" Gate Valves w/ Valve Can, CIP	EA	135	\$2,500.00	\$336,755.32
18	12" Gate Valves w/ Valve Can, CIP	EA	90	\$3,263.00	\$292,988.21
19	14" Gate Valves w/ Valve Can, CIP	EA	2	\$4,000.00	\$8,000.00
20	Pressurized waterline connections, CIP	EA	63	\$1,184.22	\$74,691.37
21	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	24,151	\$3.00	\$72,451.66
22	Joint Restraints, CIP	EA	1,280	\$77.75	\$99,500.99
23	1" Water Service, New single connection to existing watermian, cip. SD 2362	EA	403	\$1,329.00	\$535,587.00
24	Water Meter Box Remove & Replace	EA	403	\$1,000.00	\$403,000.00
25	Dewatering of Trench, CIP	LF	14,970	\$53.00	\$793,410.00
26	Valve/Pipeline abandonment	LS	1	\$125,668.33	\$125,668.33
27	Hydrant removal and abandonment	LS	1	\$21,132.65	\$21,132.65
	Water Well				
28	Furnish and Install 40 HP Pump, duty point of 500 GPM at 110 PSI, CIP. With drop	EA	1	\$50,000.00	\$50,000.00
29	pipe/cable/pit less CIP Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12	EA	1	\$30,000.00	\$30,000.00
	Enclosure, CIP				
30	Building 24X30 - Complete w/ Electrical and Plumbing	SQFT	720	\$425.00	\$306,000.00
31	12" Steel Cased Potable Water Well - Drilling Complete	LF FA	674	\$900.00	\$606,600.00
32	Furnish and install new gas- chlorination disinfection system, CIP. 8" Waterline Pipe excl. fitting, (std. spec.sec 801), icl. Trench, & compacted backfill,	EA	1	\$165,000.00	\$165,000.00
33	to 6' depth, cip.	LF	200	\$25.00	\$5,000.00
34	8" Gate Valve, cip SD 2333	EA	3	\$1,205.00	\$3,615.00
35	6" Service Stub-Out w/ 6" Gate Valve, 100'	EA	2	\$6,000.00	\$12,000.00
	Roadway Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential	I	l 1	T	
36	Streets, include Subgrade Prep, CIP	SY	8,317	\$42.00	\$349,300.00
	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP	SY	8,317	\$62.00	\$515,633.33
37	Excavate and Dispose of Unsuitable Material, CIP	CY	49,900	\$15.00	\$748,500.00
38	Import of Engineered Fill	CY	49,900	\$15.00	\$748,500.00
39	Geogrid Base Roadway Reinforcement	SY	8,317	\$5.50	\$45,741.63
40	Remove and replace Curb and Gutter @ Services, CIP	LF	1,612	\$25.00	\$40,300.00
41	Remove and replace Sidewalk @ Services, CIP	CY	1,128	\$48.00	\$54,163.2

\$9,234,817.7	ion Cost Subtotal:	Construct			
\$9,655,002.0	2-YR Inflation @ 4.55% + Construction Cost Subtotal:				
\$965,500.0	Contingency - 10%:				
\$902,743.0	NMGRT @ 8.5%:				
\$633,778.0	Finance Interest:	Interim			
\$12,157,023.0	STRUCTION COST:	MATED CONS	TOTAL EST		
				ENGINEERING SERVICES	
\$106,386.0	\$106,386.00	1	LS	Bridge Loan @ 5.5%	42
\$212,410.0	\$212,410.00	1	LS	Additional Engineering - Data Collection*	43
\$60,000.0	\$60,000.00	1	LS	Additional Engineering - Computer hydraulic model and calibration*	44
\$35,000.0	\$35,000.00	1	LS	Additional Engineering - Hydrogeology Well siting study *	45
\$1,062,050.0	\$1,062,050.00	1	LS	Engineering Design Services	46
\$24,427.0	\$24,427.00	1	LS	Engineering - Bid Phase	47
\$371,718.0	\$371,718.00	1	LS	Engineering - Construction Inspection	48
\$20,000.0	\$20,000.00	1	LS	Engineering-Well Construction Oversight	49
\$148,687.0	\$148,687.00	1	LS	Engineering - Construction Management	50
\$2,040,678.0	Services Subtotal:	Engineering		·	
\$173,458.0	NMGRT @ 8.5%:				
\$2,214,136.0	Engineering Total:				
				FINANCING SERVICES	
\$108,619.0	\$108,619.00	1	LS	Loan Origination Fee	51
\$108,619.0	Services Subtotal:	Financing			
\$9,233.0	NMGRT @ 8.5%:	Financing			
\$117,852.0	gal Services Total:	Le			
				LEGAL SERVICES	
\$10,000.0	\$10,000.00	1	LS	Legal Fees - Project Attorney	52
\$21,000.0	\$21,000.00	1	LS	Legal Fees - Bond Counsel	53
\$31,000.0	Services Subtotal:	Legal			
\$2,635.0	Legal NMGRT @ 8.5%:				
\$33,635.0	gal Services Total:	Le			
\$14,522,64	GRAND TOTAL:				



ALTERNATIVE VIII – WILLIAMSBURG AREA



Alternative VIII - Williamsburg									
Open Trench Waterline									
	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST				
General (F9) of Convert Costs									
2	Mob/Demob. (5% of General Cost)	LS LS	1	\$582,097.11	\$582,097.11 \$931.355.37				
3	Traffic Control (3.43% of General Cost)	LS	1	\$931,355.37	\$931,355.37				
4	Construction Survey/Staking (2.17% of General Cost) SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$232,838.84 \$407,467.98	\$232,838.84				
5	Materials Testing (0.2% of General Cost)	LS	1	\$232,838.84	\$232,838.84				
6	Subsurface Utility Locating	LS	1	\$19,886.02	\$19,886.02				
7	Utility Relocation	LS	1	\$19,886.02	\$19,886.02				
8	AC Pipe Removal and Disposal	LS	1	\$14,079.30	\$14,079.30				
0	Waterline		1	314,075.30	\$14,075.30				
9	6" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	31,313	\$28.78	\$901,188.14				
10	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	5,818	\$35.70	\$207,702.60				
11	14"Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	9,800	\$50.77	\$497,546.00				
12	Jack and Bore w/ 18-inch Casing pipe, CIP	LF	3,067	\$220.00	\$674,696.00				
13	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	94	\$3,500.00	\$328,517.00				
14	6" Gate Valves w/ Valve Can, CIP	EA	198	\$935.00	\$185,172.19				
15	8" Gate Valves w/ Valve Can, CIP	EA	31	\$1,205.00	\$37,813.86				
17	12" Gate Valves w/ Valve Can, CIP	EA	28	\$3,263.00	\$91,364.00				
18	14" Gate Valves w/ Valve can, CIP	EA	14	\$4,000.00	\$56,000.00				
19	Pressurized waterline connections, CIP	EA	38	\$1,184.22	\$45,454.47				
20	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	18,922	\$3.00	\$56,766.40				
21	Joint Restraints, CIP	EA	1,113	\$77.75	\$86,545.12				
22	1" Water Service, New single connection to existing watermian, cip. SD 2362	EA	653	\$1,329.00	\$867,837.00				
23	Water Meter Box Remove & Replace	EA	653	\$1,000.00	\$653,000.00				
24	Dewatering of Trench, CIP	LF	2,347	\$53.00	\$124,367.15				
25	Valve/Pipeline abandonment	LS	1	\$94,184.80	\$94,184.80				
26	Hydrant removal and abandonment	LS	1	\$39,750.56	\$39,750.56				
20	Water Well		_	Ç33,730.30	ψ33), 30.30				
27	Furnish and Install 40 HP Pump, duty point of 500 GPM at 110 PSI, CIP. With drop pipe/cable/pit less CIP	EA	1	\$50.00	\$50.00				
28	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 Enclosure, CIP	EA	1	\$30,000.00	\$30,000.00				
29	Building 24X30 - Complete w/ Electrical and Plumbing	SQFT	720	\$425.00	\$306,000.00				
30	12" Steel Cased Potable Water Well - Drilling Complete	LF	674	\$900.00	\$606,600.00				
31	Furnish and install new gas- chlorination disinfection system, CIP.	EA	1	\$165,000.00	\$165,000.00				
32	8" Waterline Pipe excl. fitting, (std. spec.sec 801), icl. Trench, & compacted backfill, to 6' depth, cip.	LF	200	\$25.00	\$5,000.00				
33	8" Gate Valve, cip SD 2333	EA	3	\$1,205.00	\$3,615.00				
34	6" Service Stub-Out w/ 6" Gate Valve, 100'	EA	2	\$6,000.00	\$12,000.00				
	Roadway								
35	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	20,858	\$42.00	\$876,045.33				
36	Asphalt Roadway, Remove, Dispose and Replace with SP III, 6" Thick for Arerial Streets, include Subgrade Prep, CIP	SY	20,858	\$62.00	\$1,293,209.78				
36	Excavate and Dispose of Unsuitable Material, CIP	CY	104,291	\$15.00	\$1,564,366.67				
37	Import of Engineered Fill	CY	104,291	\$15.00	\$1,564,366.67				
38	Geogrid Base Roadway Reinforcement	SY	20,858	\$5.50	\$114,720.22				
39	Remove and replace Curb and Gutter @ Services, CIP	LF	2,612	\$25.00	\$65,300.00				
40	Remove and replace Sidewalk @ Services, CIP	CY	1,828	\$48.00	\$87,763.20				
Construction Cost Subtotal:									
		2-YR Inflation	n @ 4.55% + Constru	ction Cost Subtotal:	\$14,723,140.00				
				Contingency - 10%:	\$1,472,314.00				
				NMGRT @ 8.5%:	\$1,376,614.00				
			Inter	im Finance Interest:	\$966,464.00				
		TC	OTAL ESTIMATED CO	NSTRUCTION COST:	\$18,538,532.00				

ENGINEERING SERVICES						
41	Bridge Loan @ 5.5%	LS	1	\$158,910.00	\$158,910.00	
42	Additional Engineering - Data Collection*	LS	1	\$323,909.00	\$323,909.00	
43	Additional Engineering - Computer hydraulic model and calibration*	LS	1	\$60,000.00	\$60,000.00	
44	Additional Engineering - Hydrogeology Well siting study *	LS	1	\$35,000.00	\$35,000.00	
45	Engineering Design Services	LS	1	\$1,619,545.00	\$1,619,545.00	
46	Engineering - Bid Phase	LS	1	\$37,250.00	\$37,250.00	
47	Engineering - Construction Inspection	LS	1	\$566,841.00	\$566,841.00	
48	Engineering-Well Construction Oversight	LS	1	\$20,000.00	\$20,000.00	
49	Engineering - Construction Management	LS	1	\$226,736.00	\$226,736.00	
			Engineerin	g Services Subtotal:	\$3,048,191.00	
NMGRT @ 8.5%:						
				Engineering Total:	\$3,307,287.00	
	FINANCING SERVICES					
50	Loan Origination Fee	LS	1	\$165,635.00	\$165,635.00	
			Financir	g Services Subtotal:	\$165,635.00	
			Financi	ng NMGRT @ 8.5%:	\$14,079.00	
			l	egal Services Total:	\$179,714.00	
	LEGAL SERVICES					
51	Legal Fees - Project Attorney	LS	1	\$10,000.00	\$10,000.00	
52	Legal Fees - Bond Counsel	LS	1	\$21,000.00	\$21,000.00	
Legal Services Subtotal:						
Legal NMGRT @ 8.5%:						
Legal Services Total:						
GRAND TOTAL:						



ALTERNATIVE IX – AIRPORT 1 PRESSURE TANK REPLACEMENT



	Alternative IX - Airport 1 Pessure Tank Replacement							
Item #	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST			
General								
1	Mob/Demob. (5% of General Cost)	LS	1	\$10,392.00	\$10,392.00			
2	Traffic Control (3.43% of General Cost)	LS	1	\$16,627.20	\$16,627.20			
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$4,156.80	\$4,156.80			
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$7,274.40	\$7,274.40			
5	Materials Testing (0.2% of General Cost)	LS	1	\$4,156.80	\$4,156.80			
6	Subsurface Utility Locating	LS	1	\$5,000.00	\$5,000.00			
7	Utility Relocation	LS	1	\$5,000.00	\$5,000.00			
	Airport Water system							
8	Site Grading/Excavation	CY	60	\$100.00	\$6,000.00			
9	6-Inch Gravel Pad, Including Subgrade Prep, Installed	SY	12	\$120.00	\$1,440.00			
10	Furnish and Install 200 Gallon Pressure Tank	LS	2	\$8,000.00	\$16,000.00			
11	Furnish and Install 4 inch DIP, Including Trenching and Compacted Backfill, per APWA Standard Spec.801 CIP.	LF	100	\$90.00	\$9,000.00			
12	Furnish and Install new chlorinaiton disinfection systemr CIP	EA	1	\$8,000.00	\$8,000.00			
13	Connect to existing well , CIP.	EA	1	\$6,000.00	\$6,000.00			
14	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 Enclosure, CIP	EA	1	\$30,000.00	\$30,000.00			
15	Chain Link Fence, incl. All attachments, hwardware & anchor posts. CIP	LF	200	\$35.00	\$7,000.00			
16	Ductile Iron MJ fittings, class 250,8" forceman, INCL. Joining Material	LB	2,000	\$4.20	\$8,400.00			
17	Furnish and install Building 12' by 30'	SQ-FT	360	\$300.00	\$108,000.00			
18	well sanitary seal pitless unit 8-inch W/ concrete slab	EA	1	\$8,000.00	\$8,000.00			
			Construct	ion Cost Subtotal:	\$260,447.20			
2-YR Inflation @ 4.55% + Construction Cost Subtotal:								
			С	ontingency - 10%:	\$27,230.00			
				NMGRT @ 8.5%:	\$25,460.00			
			Interim	Finance Interest:	\$17,874.00			
		TOTAL ESTIM	ATED CONS	STRUCTION COST:	\$342,862.00			
	ENGINEERING SERVICES	1						
19	Bridge Loan @ 5.5%	LS	1	\$4,340.00	\$4,340.00			
20	Additional Engineering - Data Collection*	LS	1	\$3,594.00	\$3,594.00			
21	Additional Engineering - Geotech	LS	1	\$10,000.00	\$10,000.00			
22	Engineering Design Services	LS	1	\$29,953.00	\$29,953.00			
23	Engineering - Bid Phase	LS	1	\$689.00	\$689.00			
24	Engineering - Construction Inspection	LS	1	\$10,483.00	\$10,483.00			
25	Engineering-Well Construction Oversight	LS	1	\$20,000.00	\$20,000.00			
26	Saw cut, remove, and dispose of existing asphalt, CIP	LS	1	\$4,193.00	\$4,193.00			
Engineering Services Subtotal:								
NMGRT @ 8.5%:								
				Engineering Total:	\$90,328.00			
			TOTAL	ESTIMATED COST:	\$433,190.00			

ALTERNATIVE X – AIRPORT 2 WITHOUT FIRE FLOW



Alternative X - Airport 2 Without Fire Flow						
Item #	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST	
	General					
1	Mob/Demob. (5% of General Cost)	LS	1	\$13,795.00	\$13,795	
2	Traffic Control (3.43% of General Cost)	LS	1	\$22,072.00	\$22,072	
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$5,518.00	\$5,518	
4	SWPPP Preparation, Implementation, and Inspection(1% of General Cost)	LS	1	\$9,656.50	\$9,657	
5	Materials Testing (0.2% of General Cost)	LS	1	\$3,198.00	\$3,198	
6	Subsurface Utility Locating	LS	1	\$5,000.00	\$5,000	
7	Utility Relocation	LS	1	\$5,000.00	\$5,000	
	Airport Water system					
8	Site Grading/Excavation	CY	60	\$100.00	\$6,000.00	
9	Engineered Fill/Subgrade Prep for Tank Foundation, Including Compaction/Testing	CY	8	\$120.00	\$960.00	
10	6-Inch Gravel Pad, Including Subgrade Prep, Installed	SY	12	\$200.00	\$2,400.00	
11	Furnish and Install 7200 Gallon Welded Steel Tank, AWWA D100-11 CIP.	GAL	7,200	\$3.20	\$23,040.00	
12	Tank Foundation Installed	LS	1	\$6,000.00	\$6,000.00	
13	Furnish and Install Cathodic Protection/Level Monitor for Tank, CIP	LS	1	\$12,000.00	\$12,000.00	
14	Furnish and Install 4 inch DIP, Including Trenching and Compacted Backfill, per APWA Standard Spec.801 CIP.	LF	100	\$90.00	\$9,000.00	
15	Furnish and Install new chlorinaiton disinfection systemr CIP	EA	1	\$8,000.00	\$8,000.00	
16	Chain Link Fence, incl. All attachments, hwardware & anchor posts. CIP	LF	200	\$35.00	\$7,000.00	
17	2-50 GPM Variable speed Booster pack	EA	1	\$45,000.00	\$45,000.00	
18	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 Enclosure, CIP	EA	1	\$30,000.00	\$30,000.00	
19	Ductile Iron MJ fittings, class 250,8" forceman, INCL. Joining Material	LB	2,500	\$4.20	\$10,500.00	
20	Furnish and install Building 12' by 30'	SQ-FT	360	\$300.00	\$108,000.00	
21	well sanitary seal pitless unit 8-inch W/ concrete slab	EA	1	\$8,000.00	\$8,000.00	
		Co	onstruction	Cost Subtotal:	\$340,139.50	
2-YR Inflation @ 4.55% + Construction Cost Subtotal:						
			Con	tingency - 10%:	\$35,562.00	
			N	MGRT @ 8.5%:	\$33,250.00	
			Interim Fi	nance Interest:	\$23,344.00	
	TO'	TAL ESTIMAT	ED CONSTE	RUCTION COST:	\$447,772	
	ENGINEERING SERVICES					
22	Bridge Loan @ 5.5%	LS	1	\$5,163.00	\$5,163.00	
23	Additional Engineering- Data Collection*	LS	1	\$4,694.00	\$4,694.00	
24	Additional Engineering - Geotech	LS	1	\$10,000.00	\$10,000.00	
25	Engineering Design Services	LS	1	\$39,118.00	\$39,118.00	
26	Saw cut, remove, and dispose of existing asphalt, CIP	LS	1	\$900.00	\$900.00	
27	Engineering - Construction Inspection	LS	1	\$13,691.00	\$13,691.00	
28	Engineering-Well Construction Oversight	LS	1	\$20,000.00	\$20,000.00	
29	Engineering - Construction Management	LS	1	\$5,476.00	\$5,476.00	
Engineering Services Subtotal:					\$99,042.00	
			N	MGRT @ 8.5%:	\$8,419.00	
			Eng	gineering Total:	\$107,461.00	
			TOTAL EST	TIMATED COST:	\$555,233.00	

ALTERNATIVE XI – AIRPORT 3 WITH FIRE FLOW



Alternative XI - Airport 3 With Fire Flow						
Item #	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST	
	General			4	4	
1	Mob/Demob. (5% of General Cost)	LS	1	\$57,914.12	\$57,914.12	
2	Traffic Control (3.43% of General Cost)	LS	1	\$92,662.59	\$92,662.59	
3	Construction Survey/Staking (2.17% of General Cost)	LS LS	1	\$23,165.65	\$23,165.65	
5	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$40,539.88	\$40,539.88 \$23,165.65	
6	Materials Testing (0.2% of General Cost) Subsurface Utility Locating	LS	1	\$23,165.65 \$5,000.00	\$5,000.00	
7	Utility Relocation	LS	1	\$5,000.00	\$5,000.00	
	Airport Water system	LJ		\$3,000.00	\$3,000.00	
8	Site Grading/Excavation	CY	181	\$10.00	\$1,810.00	
9	Engineered Fill/Subgrade Prep for Tank Foundation, Including Compction and Testing	CY	84	\$12.00	\$1,008.00	
10	6-Inch Gravel Pad, Including Subgrade Prep, Installed	SY	125	\$20.00	\$2,500.00	
11	Furnish and Install 200.000 Gallon Welded Steel Tank, AWWA D100-11 CIP.	GAL	200,000	\$1.75	\$350,000.00	
12	Tank Foundation Installed	LS	200,000	\$45,000.00	\$45,000.00	
13	Furnish and Install Cathodic Protection/Level Monitor for Tank, CIP	LS	1	\$45,000.00	\$45,000.00	
- 15	Furnish and Install 8 inch DIP, Including Trenching and Compacted Backfill, per APWA Standard Spec.801	L3	1	\$24,000.00	324,000.00	
14	CIP.	LF	140	\$120.00	\$16,800.00	
15	Furnish and Install new chlorinaiton disinfection systemr CIP	EA	1	\$8,000.00	\$8,000.00	
16	Chain Link Fence, incl. All attachments, hwardware & anchor posts. CIP	LF	720	\$35.00	\$25,200.00	
17	2-50 GPM Variable Speed Booster pack	EA	1	\$45,000.00	\$45,000.00	
18	Furnish and Install Electrical/Control Panel for Booster/Well Pumps and NEMA 12 Enclosure, Complete in Place	EA	1	\$30,000.00	\$30,000.00	
19	Ductile Iron MJ fittings, class 250,8" forceman, INCL. Joining Material	LB	3,500	\$4.20	\$14,700.00	
20	Fire Booster Pump 1500GPM	EA	1	\$60,000.00	\$60,000.00	
21	Furnish and install Building 12' by 30'	SQ-FT	360	\$300.00	\$108,000.00	
22	8" Waterline C-900 DR-18 PVC Pipe Installed With Trenching and Backfill	LF	2,549	\$35.70	\$90,999.30	
23	4-1/2' Fire Hydrant w/ piping valves, and connection	EA	5	\$3,500.00	\$17,500.00	
24	8" Gate Valves w/ Valve Can, CIP	EA	15	\$1,205.00	\$18,075.00	
25	Ductile Iron MJ Fittings, All Sizes, Class 25, CIP	LB	2,270	\$3.00	\$6,810.00	
	Roadway					
26	Asphalt Roadway, Remove, Dispose and Replace with SP III, 3" Thick for Residential Streets, include Subgrade Prep, CIP	SY	1,699	\$50.00	\$84,966.67	
27	Excavate and Dispose of Unsuitable Material, CIP	CY	5,098	\$15.00	\$76,470.00	
28	Import of Engineered Fill	CY	5,098	\$15.00	\$76,470.00	
29	Geogrid Base Roadway Reinforcement	SY	850	\$5.50	\$4,673.17	
32	Saw cut, remove, and dispose of existing asphalt, CIP	SY	1,699	\$4.10	\$6,967.27	
33	Subgrade Prep,	SY	1,699	\$2.50	\$4,248.33	
34	6" Aggregate Base Course, CIP	SY	1,699	\$7.00	\$11,895.33	
35	Asphalt Paving, 2-3" Lifts, w/ machine laydown, CIP	SY	1,699	\$16.00	\$27,189.33	
			Construction	Cost Subtotal:	\$1,405,730.29	
	2-YR Inflatio	on @ 4.55% +	Construction	Cost Subtotal:	\$1,469,691.00	
			Con	tingency - 10%:	\$146,969.00	
			N	MGRT @ 8.5%:	\$137,416.00	
			Interim Fi	nance Interest:	\$96,474.00	
		OTAL ESTIMA	TED CONST	RUCTION COST:	\$1,850,550.00	
	ENGINEERING SERVICES					
36	Bridge Loan @ 5.5%	LS	1	\$16,170.00	\$16,170.00	
37	Additional Engineering - Data Collection*	LS	1	\$19,400.00	\$19,400.00	
38	Additional Engineering - Geotech	LS	1	\$10,000.00	\$10,000.00	
39	Engineering Design Services	LS	1	\$161,666.00	\$161,666.00	
40	Engineering - Bid Phase	LS	1	\$3,718.00	\$3,718.00	
41	Engineering - Construction Inspection	LS	1	\$56,583.00	\$56,583.00	
42	Engineering-Well Construction Oversight	LS	1	\$20,000.00	\$20,000.00	
43	Engineering - Construction Management	LS	1	\$22,633.00	\$22,633.00	
		Eng		vices Subtotal:	\$310,170.00	
				MGRT @ 8.5%:	\$26,364.00	
				gineering Total:	\$336,534.00	
			TOTAL EST	IMATED COST:	\$2,187,084.00	

AIRPORT XII - AIRPORT 4 VFD WELL PUMP



Item #	ITEMS LIST	UNITS	QTY	UNIT COST	EXTEND COST			
General								
1	Mob/Demob. (5% of General Cost)	LS	1	\$11,992.00	\$11,992.0			
2	Traffic Control (3.43% of General Cost)	LS	1	\$19,187.20	\$19,187.2			
3	Construction Survey/Staking (2.17% of General Cost)	LS	1	\$4,796.80	\$4,796.8			
4	SWPPP Preparation, Implementation, and Inspection (1% of General Cost)	LS	1	\$8,394.40	\$8,394.4			
5	Materials Testing (0.2% of General Cost)	LS	1	\$4,796.80	\$4,796.8			
6	Subsurface Utility Locating	LS	1	\$5,000.00	\$5,000.0			
7	Utility Relocation	LS	1	\$5,000.00	\$5,000.0			
	Airport Water system							
8	Site Grading/Excavation	CY	60	\$100.00	\$6,000.00			
9	6-Inch Gravel Pad, Including Subgrade Prep, Installed	SY	12	\$120.00	\$1,440.00			
10	Furnish and Install 30 Gallon Pressure Tank	LS	1	\$2,000.00	\$2,000.00			
11	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, and NEMA 12 Enclosure, CIP	EA	1	\$18,000.00	\$18,000.0			
12	2-50 GPM Variable Speed Booster pack W/VFD	EA	1	\$28,000.00	\$28,000.00			
13	Furnish and Install new chlorinaiton disinfection systemr CIP	EA	1	\$8,000.00	\$8,000.00			
14	Furnish and Install Electrical/Control Panel for Booster/Well Pumps, Including Well VFD and NEMA 12 Enclosure, CIP	EA	1	\$45,000.00	\$45,000.00			
15	Chain Link Fence, incl. All attachments, hwardware & anchor posts. CIP	LF	200	\$35.00	\$7,000.00			
16	Ductile Iron MJ fittings, class 250,8" forceman, INCL. Joining Material	LB	2,000	\$4.20	\$8,400.00			
17	Furnish and install Building 12' by 30'	SQ-FT	360	\$300.00	\$108,000.00			
18	well sanitary seal pitless unit 8-inch W/ concrete slab	EA	1	\$8,000.00	\$8,000.00			
			Constructi	ion Cost Subtotal:	\$299,007.2			
	2-YR Infla	tion @ 4.55% +	- Constructi	on Cost Subtotal:	\$312,612.0			
			Co	ontingency - 10%:	\$31,261.0			
				NMGRT @ 8.5%:	\$29,229.0			
			Interim	Finance Interest:	\$20,521.0			
		TOTAL ESTIM	ATED CONS	TRUCTION COST:	\$393,623.0			
	ENGINEERING SERVICES	1						
19	Bridge Loan @ 5.5%	LS	1	\$4,738.00	\$4,738.00			
20	Additional Engineering - Data Collection*	LS	1	\$4,126.00	\$4,126.0			
21	Additional Engineering - Geotech	LS	1	\$10,000.00	\$10,000.0			
22	Engineering Design Services	LS	1	\$34,387.00	\$34,387.0			
23	Engineering - Bid Phase	LS	1	\$791.00	\$791.0			
24	Engineering - Construction Inspection	LS	1	\$12,036.00	\$12,036.0			
25	Engineering-Well Construction Oversight	LS	1	\$20,000.00	\$20,000.0			
26	Saw cut, remove, and dispose of existing asphalt, CIP	LS	1	\$4,814.00	\$4,814.0			
Engineering Services Subtotal:								
				NMGRT @ 8.5%:	\$7,726.00			
				ngineering Total:	\$98,618.00			
			TOTAL E	STIMATED COST:	\$492,241.0			

APPENDIX 5- OPERATION AND MAINTENANACE

TABLE OF CONTENT:

- o ALTERNATIVE II COMPLETE SYSTEM
- ALTERNATIVE III SYSTEM PERFORMANCE UPGRADE
- o ALTERNATIVE III A SYSTEM HIGH PRESSURE SOLUTION
- ALTERNATIVE III B SYSTEM REDUNDANCY AND HYDRAULIC ENHANCEMENTS
- o ALTERNATIVE III C ADDITIONAL HYDRAULIC PERFROMANCE ENHACNECMENT
- ALTERNATIVE IV NORTH AREA
- ALTRENATIVE V − EAST AREA
- ALTERNATIVE VI WEST AREA
- o ALTRENATIVE VII DOWNTOWN AREA
- O ALTERNATIVE VIII WILLIAMSBURG AREA
- ALTERNATIVE IX AIRPORT 1 PRESSURE TANK REPLACEMENT
- O ALTERNATIVE X AIRPORT 2 WITHOUT FIRE FLOW
- ALTERNATIVE XI AIRPORT 3 WITH FIRE FLOW
- AIRPORT XII AIRPORT 4 VFD WELL PUMP





O&M Alternative I - Do Nothing WATERLINES

Input Variables											
Discount Rate:		2.25%									
Repair Costs:	\$	137,800									
Power cost due to Water Losses	S .	39,434									
O&M	\$	566,404									

Year:	1	2	3	4	5
Repair Costs:	\$ 140,900.50	\$ 144,070.76	\$ 147,312.35	\$ 150,626.88	\$ 154,015.99
Water Loss:	\$ 40,321.04	\$ 41,228.27	\$ 42,155.90	\$ 43,104.41	\$ 44,074.26
O&M	\$ 579,148.31	\$ 592,179.15	\$ 605,503.18	\$ 619,127.00	\$ 633,057.36
Future Value	\$ 760,369.86	\$ 777,478.18	\$ 794,971.44	\$ 812,858.29	\$ 831,147.60
Net Present Value:	\$ 743,638.00	\$ 743,638.00	\$ 743,638.00	\$ 743,638.00	\$ 743,638.00

Year:	6	7	8	9	10
Repair Costs:	\$ 157,481.35	\$ 161,024.68	\$ 164,647.73	\$ 168,352.31	\$ 172,140.23
Water Loss:	\$ 45,065.93	\$ 46,079.91	\$ 47,116.71	\$ 48,176.84	\$ 49,260.82
O&M	\$ 647,301.15	\$ 661,865.43	\$ 676,757.40	\$ 691,984.44	\$ 707,554.09
Future Value	\$ 849,848.43	\$ 868,970.02	\$ 888,521.84	\$ 908,513.58	\$ 928,955.14
Net Present Value:	\$ 743,638.00	\$ 743,638.00	\$ 743,638.00	\$ 743,638.00	\$ 743,638.00

Year:	11	12	13	14	15
Repair Costs:	\$ 176,013.39	\$ 179,973.69	\$ 184,023.10	\$ 188,163.62	\$ 192,397.30
Water Loss:	\$ 50,369.19	\$ 51,502.49	\$ 52,661.30	\$ 53,846.18	\$ 55,057.72
O&M	\$ 723,474.06	\$ 739,752.22	\$ 756,396.65	\$ 773,415.57	\$ 790,817.42
Future Value	\$ 949,856.63	\$ 971,228.40	\$ 993,081.04	\$ 1,015,425.36	\$ 1,038,272.44
Net Present Value:	\$ 743,638.00	\$ 743,638.00	\$ 743,638.00	\$ 743,638.00	\$ 743,638.00

Year:		16		17		18		19		20
Repair Costs:	\$	196,726.24	\$	201,152.58	\$	205,678.51	\$	210,306.28	\$	215,038.17
Water Loss:	\$	56,296.52	\$	57,563.19	\$	58,858.36	\$	60,182.67	\$	61,536.78
O&M	\$	808,610.81	\$	826,804.56	\$	845,407.66	\$	864,429.33	\$	883,878.99
Future Value	\$	1,061,633.57	\$	1,085,520.32	\$	1,109,944.53	\$	1,134,918.28	\$	1,160,453.94
Net Present Value:	\$	743,638.00	\$	743,638.00	\$	743,638.00	\$	743,638.00	\$	743,638.00
				T	ota	l Lifetime Mainten	anc	ce Cost (20 years):	\$	18,941,969
	Total Lifetime Maintenance Cost (present value):									
	ANNUAL TOTAL O&M ALT I									



O&M Alternative II - Complete System WATERLINES

Input Variabl	es	
Discount Rate:		2.25%
Repair Costs:	\$	4,685
Water Losses	\$	1,341
O&M	\$	566,404
Well Equipment	\$	11,758

Year:	1	2	3	4	5
Repair Costs:	\$ 4,790.62	\$ 4,898.41	\$ 5,008.62	\$ 5,121.31	\$ 5,236.54
Water Loss:	\$ 1,370.92	\$ 1,401.76	\$ 1,433.30	\$ 1,465.55	\$ 1,498.52
O&M	\$ 579,148.31	\$ 592,179.15	\$ 605,503.18	\$ 619,127.00	\$ 633,057.36
Well Equipment	\$ 12,022.56	\$ 12,293.06	\$ 12,569.66	\$ 12,852.47	\$ 13,141.65
Future Value	\$ 597,332.40	\$ 610,772.38	\$ 624,514.76	\$ 638,566.34	\$ 652,934.08
Net Present Value:	\$ 584,188.17	\$ 584,188.17	\$ 584,188.17	\$ 584,188.17	\$ 584,188.17

Year:	6	7	8	9	10
Repair Costs:	\$ 5,354.37	\$ 5,474.84	\$ 5,598.02	\$ 5,723.98	\$ 5,852.77
Water Loss:	\$ 1,532.24	\$ 1,566.72	\$ 1,601.97	\$ 1,638.01	\$ 1,674.87
O&M	\$ 647,301.15	\$ 661,865.43	\$ 676,757.40	\$ 691,984.44	\$ 707,554.09
Well Equipment	\$ 13,437.34	\$ 13,739.68	\$ 14,048.82	\$ 14,364.92	\$ 14,688.13
Future Value	\$ 667,625.10	\$ 682,646.66	\$ 698,006.21	\$ 713,711.35	\$ 729,769.86
Net Present Value:	\$ 584,188.17	\$ 584,188.17	\$ 584,188.17	\$ 584,188.17	\$ 584,188.17

Year:	11	12	13	14	15
Repair Costs:	\$ 5,984.46	\$ 6,119.11	\$ 6,256.79	\$ 6,397.56	\$ 6,541.51
Water Loss:	\$ 1,712.55	\$ 1,751.08	\$ 1,790.48	\$ 1,830.77	\$ 1,871.96
O&M	\$ 723,474.06	\$ 739,752.22	\$ 756,396.65	\$ 773,415.57	\$ 790,817.42
Well Equipment	\$ 15,018.62	\$ 15,356.54	\$ 15,702.06	\$ 16,055.35	\$ 16,416.60
Future Value	\$ 746,189.68	\$ 762,978.95	\$ 780,145.97	\$ 797,699.26	\$ 815,647.49
Net Present Value:	\$ 584,188.17	\$ 584,188.17	\$ 584,188.17	\$ 584,188.17	\$ 584,188.17

Year:		16		17		18		19		20
Repair Costs:	\$	6,688.69	\$	6,839.19	\$	6,993.07	\$	7,150.41	\$	7,311.30
Water Loss:	\$	1,914.08	\$	1,957.15	\$	2,001.18	\$	2,046.21	\$	2,092.25
O&M	\$	808,610.81	\$	826,804.56	\$	845,407.66	\$	864,429.33	\$	883,878.99
Well Equipment	\$	16,785.97	\$	17,163.66	\$	17,549.84	\$	17,944.71	\$	18,348.47
Future Value	\$	833,999.56	\$	852,764.55	\$	871,951.75	\$	891,570.67	\$	911,631.01
Net Present Value:	\$	584,188.17	\$	584,188.17	\$	584,188.17	\$	584,188.17	\$	584,188.17
	Total Lifetime Maintenance Cost (20 years):									
	Total Lifetime Maintenance Cost (present value):									

ANNUAL TOTAL O&M ALT II \$

584,188



O&M Alternative III - System Performance Upgrade WATERLINES

Input Variables										
Discount Rate:		2.25%								
Repair Costs:	\$	85,712								
Water Losses	\$	24,528								
O&M	\$	566,404								
Well Equipment	\$	11,758								

Well Equipment	\$ 11,758				
Year:	1	2	3	4	5
Repair Costs:	\$87,640.11	\$89,612.01	\$91,628.28	\$93,689.92	\$95,797.94
Water Loss:	\$25,079.69	\$25,643.98	\$26,220.97	\$26,810.94	\$27,414.19
O&M	\$579,148.31	\$592,179.15	\$605,503.18	\$619,127.00	\$633,057.36
Well Equipment	\$12,022.56	\$12,293.06	\$12,569.66	\$12,852.47	\$13,141.65
Future Value	\$703,890.67	\$719,728.21	\$735,922.09	\$752,480.34	\$769,411.15
Net Present Value:	\$688,401.63	\$688,401.63	\$688,401.63	\$688,401.63	\$688,401.63
<u>.</u>					
Year:	6	7	8	9	10
Repair Costs:	\$97,953.40	\$100,157.35	\$102,410.89	\$104,715.13	\$107,071.22
Water Loss:	\$28,031.01	\$28,661.71	\$29,306.59	\$29,965.99	\$30,640.23
O&M	\$647,301.15	\$661,865.43	\$676,757.40	\$691,984.44	\$707,554.09
Well Equipment	\$13,437.34	\$13,739.68	\$14,048.82	\$14,364.92	\$14,688.13
Future Value	\$786,722.90	\$804,424.16	\$822,523.71	\$841,030.49	\$859,953.67
Net Present Value:	\$688,401.63	\$110,239.41	\$110,239.41	\$110,239.41	\$110,239.41
Year:	11	12	13	14	15
Repair Costs:	\$109,480.33	\$111,943.63	\$114,462.37	\$117,037.77	\$119,671.12
Water Loss:	\$31,329.63	\$32,034.55	\$32,755.33	\$33,492.32	\$34,245.90
O&M	\$723,474.06	\$739,752.22	\$756,396.65	\$773,415.57	\$790,817.42
Well Equipment	\$15,018.62	\$15,356.54	\$15,702.06	\$16,055.35	\$16,416.60
Future Value	\$879,302.63	\$899,086.94	\$919,316.40	\$940,001.02	\$961,151.04
Net Present Value:	\$688,401.63	\$688,401.63	\$688,401.63	\$688,401.63	\$688,401.63
· ·	4.5	4=	40	10	20
Year:	16	17	18	19	20
Repair Costs:	\$122,363.72	\$125,116.90	\$127,932.03	\$130,810.50	\$133,753.74
Water Loss:	\$35,016.43	\$35,804.30	\$36,609.90	\$37,433.62	\$38,275.88
O&M	\$808,610.81	\$826,804.56	\$845,407.66	\$864,429.33	\$883,878.99
Well Equipment	\$16,785.97	\$17,163.66	\$17,549.84	\$17,944.71	\$18,348.47
Future Value	\$982,776.94	\$1,004,889.42	\$1,027,499.43	\$1,050,618.17	\$1,074,257.08
Net Present Value:	\$688,401.63	\$688,401.63	\$688,401.63	\$688,401.63	\$688,401.63
				ance Cost (20 years):	
_		Total Life		Cost (present value):	\$ 10,989,446
			ANNUAL	TOTAL O&M ALT III	\$ 688,402



O&M Alternative III A- System High Pressure Solutions WATERLINES

		WATERLINES			
Input Variable	es				
Discount Rate:	2.25%				
Repair Costs:	\$101,007				
Water Losses	\$28,905				
O&M	\$566,404				
-					
Year:	1	2	3	4	5
Repair Costs:	\$103,280.07	\$105,603.87	\$107,979.96	\$110,409.50	\$112,893.72
Water Loss:	\$29,555.32	\$30,220.32	\$30,900.28	\$31,595.53	\$32,306.43
O&M	\$579,148.31	\$592,179.15	\$605,503.18	\$619,127.00	\$633,057.36
Future Value	\$711,983.70	\$728,003.34	\$744,383.41	\$761,132.04	\$778,257.51
Net Present Value:	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58
	\$15,667.12	· ·			. ,
Year:	6	7	8	9	10
Repair Costs:	\$115,433.83	\$118,031.09	\$120,686.79	\$123,402.24	\$126,178.79
Water Loss:	\$33,033.33	\$33,776.58	\$34,536.55	\$35,313.62	\$36,108.18
O&M	\$647,301.15	\$661,865.43	\$676,757.40	\$691,984.44	\$707,554.09
Future Value	\$795,768.30	\$813,673.09	\$831,980.73	\$850,700.30	\$869,841.06
Net Present Value:	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58
Year:	11	12	13	14	15
Repair Costs:	\$129,017.81	\$131,920.71	\$134,888.93	\$137,923.93	\$141,027.22
Water Loss:	\$36,920.61	\$37,751.33	\$38,600.73	\$39,469.25	\$40,357.31
O&M	\$723,474.06	\$739,752.22	\$756,396.65	\$773,415.57	\$790,817.42
Future Value	\$889,412.48	\$909,424.26	\$929,886.31	\$950,808.75	\$972,201.95
Net Present Value:	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58
Year:	16	17	18	19	20
Repair Costs:	\$144,200.33	\$147,444.84	\$150,762.35	\$154,154.50	\$157,622.98
Water Loss:	\$41,265.35	\$42,193.82	\$43,143.18	\$44,113.90	\$45,106.46
0&M	\$808,610.81	\$826,804.56	\$845,407.66	\$864,429.33	\$883,878.99
Future Value	\$994,076.49	\$1,016,443.21	\$1,039,313.18	\$1,062,697.73	\$1,086,608.43
Net Present Value:	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58	\$696,316.58
				ance Cost (20 years):	\$ 17,736,596
		Total Life		Cost (present value):	\$ 11,115,798
			ANNUAL T	OTAL O&M ALT III-A	\$696,317



O&M Alternative III B- System Redundancy and Hydraulic Performance Enhancements

		WATERLINES			
Input Variable	es				
Discount Rate:	2.25%				
Repair Costs:	\$129,119				
Water Losses	\$36,949				
O&M	\$566,404				
Well equipment	\$11,758				
Year:	1	2	3	4	5
Repair Costs:	\$132,023.77	\$134,994.30	\$138,031.68	\$141,137.39	\$144,312.98
Water Loss:	\$37,780.82	\$38,630.89	\$39,500.08	\$40,388.83	\$41,297.58
O&M	\$579,148.31	\$592,179.15	\$605,503.18	\$619,127.00	\$633,057.36
Well Equipment	\$12,022.56	\$12,293.06	\$12,569.66	\$12,852.47	\$13,141.65
Future Value	\$760,975.45	\$778,097.40	\$795,604.59	\$813,505.70	\$831,809.57
Net Present Value:	\$744,230.27	\$744,230.27	\$744,230.27	\$744,230.27	\$744,230.27
<u> </u>					
Year:	6	7	8	9	10
Repair Costs:	\$147,560.02	\$150,880.12	\$154,274.92	\$157,746.11	\$161,295.40
Water Loss:	\$42,226.78	\$43,176.88	\$44,148.36	\$45,141.70	\$46,157.39
O&M	\$647,301.15	\$661,865.43	\$676,757.40	\$691,984.44	\$707,554.09
Well Equipment	\$13,437.34	\$13,739.68	\$14,048.82	\$14,364.92	\$14,688.13
Future Value	\$850,525.29	\$869,662.11	\$889,229.51	\$909,237.17	\$929,695.01
Net Present Value:	\$744,230.27	\$744,230.27	\$744,230.27	\$744,230.27	\$744,230.27
Year:	11	12	13	14	15
Repair Costs:	\$164,924.54	\$168,635.35	\$172,429.64	\$176,309.31	\$180,276.27
Water Loss:	\$47,195.93	\$48,257.84	\$49,343.64	\$50,453.87	\$51,589.08
O&M	\$723,474.06	\$739,752.22	\$756,396.65	\$773,415.57	\$790,817.42
Well Equipment	\$15,018.62	\$15,356.54	\$15,702.06	\$16,055.35	\$16,416.60
Future Value	\$950,613.14	\$972,001.94	\$993,871.98	\$1,016,234.10	\$1,039,099.37
Net Present Value:	\$744,230.27	\$744,230.27	\$744,230.27	\$744,230.27	\$744,230.27
v	4.6	4=	40	40	20
Year:	16	17	18	19	20
Repair Costs:	\$184,332.48	\$188,479.96	\$192,720.76	\$197,056.98	\$201,490.76
Water Loss:	\$52,749.83	\$53,936.71	\$55,150.28	\$56,391.16	\$57,659.96
O&M	\$808,610.81	\$826,804.56	\$845,407.66	\$864,429.33	\$883,878.99
Well Equipment	\$16,785.97	\$17,163.66	\$17,549.84	\$17,944.71	\$18,348.47
Future Value	\$1,062,479.11	\$1,086,384.88	\$1,110,828.54	\$1,135,822.19	\$1,161,378.19
Net Present Value:	\$744,230.27	\$744,230.27	\$744,230.27	\$744,230.27	\$744,230.27

ANNUAL TOTAL O&M ALT III-B

Total Lifetime Maintenance Cost (20 years): \$

Total Lifetime Maintenance Cost (present value): \$

\$744,230

18,957,055

11,880,678



O&M Alternative III C- Additional Hydraulic Performance Enhancements WATERLINES

Input Variable	es				
Discount Rate:	2.25%				
Repair Costs:	\$130,634				
Water Losses	\$37,383				
O&M	\$566,404				
Year:	1	2	3	4	5
Repair Costs:	\$133,573.67	\$136,579.08	\$139,652.11	\$142,794.28	\$146,007.15
Water Loss:	\$38,224.35	\$39,084.40	\$39,963.80	\$40,862.98	\$41,782.40
O&M	\$579,148.31	\$592,179.15	\$605,503.18	\$619,127.00	\$633,057.36
Future Value	\$750,946.33	\$767,842.63	\$785,119.09	\$802,784.27	\$820,846.91
Net Present Value:	\$734,421.84	\$734,421.84	\$734,421.84	\$734,421.84	\$734,421.84
Year:	6	7	8	9	10
Repair Costs:	\$149,292.32	\$152,651.39	\$156,086.05	\$159,597.99	\$163,188.94
Water Loss:	\$42,722.50	\$43,683.76	\$44,666.64	\$45,671.64	\$46,699.25
O&M	\$647,301.15	\$661,865.43	\$676,757.40	\$691,984.44	\$707,554.09
Future Value	\$839,315.97	\$858,200.58	\$877,510.09	\$897,254.07	\$917,442.28
Net Present Value:	\$734,421.84	\$734,421.84	\$734,421.84	\$734,421.84	\$734,421.84
Year:	11	12	13	14	15
Repair Costs:	\$166,860.69	\$170,615.06	\$174,453.90	\$178,379.11	\$182,392.64
Water Loss:	\$47,749.99	\$48,824.36	\$49,922.91	\$51,046.18	\$52,194.72
O&M	\$723,474.06	\$739,752.22	\$756,396.65	\$773,415.57	\$790,817.42
Future Value	\$938,084.73	\$959,191.64	\$980,773.45	\$1,002,840.86	\$1,025,404.77
Net Present Value:	\$734,421.84	\$168,017.63	\$168,017.63	\$168,017.63	\$168,017.63
Year:	16	17	18	19	20
Repair Costs:	\$186,496.47	\$190,692.64	\$194,983.23	\$199,370.35	
Water Loss:	·				\$203,856.18
O&M	\$53,369.10 \$808,610.81	\$54,569.90 \$826,804.56	\$55,797.72 \$845,407.66	\$57,053.17 \$864,429.33	\$58,336.87
Future Value	\$1,048,476.38	\$1,072,067.10	\$1,096,188.61	\$1,120,852.85	\$883,878.99 \$1,146,072.04
Net Present Value:	\$734,421.84	\$734,421.84	\$734,421.84	\$1,120,832.83	\$1,146,072.04
ivet Flesent Value.	3/34,421.04			ance Cost (20 years):	\$ 18,707,215
				Cost (present value):	\$ 11,724,099
		TOTAL EITE	time manitenance	cost (present value).	7 11,727,033

ANNUAL TOTAL O&M ALT III-C

\$734,422



O&M Alternative IV - North Side

				WATE						
Input Vai	riah	Nac	ı	WAIL	INLI	IIVES				
Discount Rate:	_	2.25%								
Repair Costs:		130,772								
Water Losses		· · · · · · · · · · · · · · · · · · ·								
O&M	Ş	37,423								
Well equipment		\$566,404								
weii equipment		\$11,758								
Year:		1		2		3		4		5
Repair Costs:	\$	133,714.57	\$	136,723.15	\$	139,799.42	\$	142,944.91	\$	146,161.17
Water Loss:	_	38,264.67	\$	39,125.63	\$		\$	40,906.09	\$	41,826.47
O&M	\$	579,148.31	\$	592,179.15	\$		\$	619,127.00	\$	633,057.36
Well equipment		12,022.56	\$	12,293.06	\$	12,569.66	\$	12,852.47	\$	13,141.65
Future Value	\$	763,150.11	_	780,320.99	\$		\$	815,830.47	\$	834,186.66
Net Present Value:		746,357.08	\$	746,357.08	\$		\$	746,357.08	\$	746,357.08
Weet reseme value.	7	740,337.00	7	740,337.00	7	740,337.00	7	740,337.00	7	740,337.00
Year:		6		7		8		9		10
Repair Costs:	Ś	149,449.80	\$	152,812.42	\$	156,250.70	\$	159,766.34	\$	163,361.08
Water Loss:		42,767.57		43,729.84	\$	44,713.76		45,719.82	\$	46,748.52
O&M	\$	647,301.15	\$	661,865.43	\$	676,757.40	\$	691,984.44	\$	707,554.09
Well equipment	\$	13,437.34	\$	13,739.68	\$		\$	14,364.92	\$	14,688.13
Future Value	\$	852,955.86	\$	872,147.36	\$		\$	911,835.52	\$	932,351.82
Net Present Value:	\$	746,357.08	\$	746,357.08	\$	746,357.08	\$	746,357.08	\$	746,357.08
Year:		11		12		13		14		15
Repair Costs:	\$	167,036.70	\$	170,795.03	\$	174,637.92	\$	178,567.27	\$	182,585.04
Water Loss:	\$	47,800.36	\$	48,875.87	\$	49,975.57	\$	51,100.02	\$	52,249.77
O&M	\$	723,474.06	\$	739,752.22	\$	756,396.65	\$	773,415.57	\$	790,817.42
Well equipment	\$	15,018.62	\$	15,356.54	\$	15,702.06		16,055.35	\$	16,416.60
Future Value	\$	953,329.73	\$	974,779.65	\$	996,712.20	\$	1,019,138.22	\$	1,042,068.83
Net Present Value:	\$	746,357.08	\$	746,357.08	\$	746,357.08	\$	746,357.08	\$	746,357.08
Year:		16		17		18		19		20
Renair Costs	¢	186 693 20	¢	190 893 80	¢	195 188 91	¢	199 580 66	¢	204 071 22

Year:		16		17		18		19		20	
Repair Costs:	\$	186,693.20	\$	190,893.80	\$	195,188.91	\$	199,580.66	\$	204,071.22	
Water Loss:	\$	53,425.39	\$	54,627.46	\$	55,856.58	\$	57,113.36	\$	58,398.41	
O&M	\$	808,610.81	\$	826,804.56	\$	845,407.66	\$	864,429.33	\$	883,878.99	
Well equipment	\$	16,785.97	\$	17,163.66	\$	17,549.84	\$	17,944.71	\$	18,348.47	
Future Value	\$	1,065,515.38	\$	1,089,489.47	\$	1,114,002.99	\$	1,139,068.05	\$	1,164,697.09	
Net Present Value:	\$	746,357.08	\$	168,194.86	\$	168,194.86	\$	168,194.86	\$	168,194.86	
	Total Lifetime Maintenance Cost (20 years):									19,011,229	
	Total Lifetime Maintenance Cost (present value): \$										

ANNUAL TOTAL O&M ALT IV \$

746,357



O&M Alternative V - East Side WATERLINES

Input Var	iables	
Discount Rate:		2.25%
Repair Costs:	\$	105,830
Water Losses	\$	30,301
O&M		\$566,404
Well equipment		\$11,758

Year:	1	2	3	4	5
Repair Costs:	\$ 108,211.58	\$ 110,646.34	\$ 113,135.89	\$ 115,681.44	\$ 118,284.28
Water Loss:	\$ 30,982.69	\$ 31,679.80	\$ 32,392.60	\$ 33,121.43	\$ 33,866.66
O&M	\$ 579,148.31	\$ 592,179.15	\$ 605,503.18	\$ 619,127.00	\$ 633,057.36
Well equipment	\$ 12,022.56	\$ 12,293.06	\$ 12,569.66	\$ 12,852.47	\$ 13,141.65
Future Value	\$ 730,365.14	\$ 746,798.36	\$ 763,601.32	\$ 780,782.35	\$ 798,349.95
Net Present Value:	\$ 714,293.54	\$ 714,293.54	\$ 714,293.54	\$ 714,293.54	\$ 714,293.54

Year:	6	7	8	9	10
Repair Costs:	\$ 120,945.67	\$ 123,666.95	\$ 126,449.46	\$ 129,294.57	\$ 132,203.70
Water Loss:	\$ 34,628.66	\$ 35,407.81	\$ 36,204.48	\$ 37,019.08	\$ 37,852.01
O&M	\$ 647,301.15	\$ 661,865.43	\$ 676,757.40	\$ 691,984.44	\$ 707,554.09
Well equipment	\$ 13,437.34	\$ 13,739.68	\$ 14,048.82	\$ 14,364.92	\$ 14,688.13
Future Value	\$ 816,312.83	\$ 834,679.86	\$ 853,460.16	\$ 872,663.01	\$ 892,297.93
Net Present Value:	\$ 714,293.54	\$ 714,293.54	\$ 714,293.54	\$ 714,293.54	\$ 714,293.54

Year:	11	12	13	14	15
Repair Costs:	\$ 135,178.28	\$ 138,219.79	\$ 141,329.74	\$ 144,509.66	\$ 147,761.12
Water Loss:	\$ 38,703.68	\$ 39,574.51	\$ 40,464.94	\$ 41,375.40	\$ 42,306.35
O&M	\$ 723,474.06	\$ 739,752.22	\$ 756,396.65	\$ 773,415.57	\$ 790,817.42
Well equipment	\$ 15,018.62	\$ 15,356.54	\$ 15,702.06	\$ 16,055.35	\$ 16,416.60
Future Value	\$ 912,374.64	\$ 932,903.07	\$ 953,893.38	\$ 975,355.99	\$ 997,301.50
Net Present Value:	\$ 714,293.54	\$ 136,131.32	\$ 136,131.32	\$ 136,131.32	\$ 136,131.32

Year:		16		17		18		19		20		
Repair Costs:	\$	151,085.75	\$	154,485.18	\$	157,961.10	\$	161,515.22	\$	165,149.31		
Water Loss:	\$	43,258.24	\$	44,231.55	\$	45,226.76	\$	46,244.36	\$	47,284.86		
O&M	\$	808,610.81	\$	826,804.56	\$	845,407.66	\$	864,429.33	\$	883,878.99		
Well equipment	\$	16,785.97	\$	17,163.66	\$	17,549.84	\$	17,944.71	\$	18,348.47		
Future Value	\$	1,019,740.78	\$	1,042,684.95	\$	1,066,145.36	\$	1,090,133.63	\$	1,114,661.63		
Net Present Value:	\$	714,293.54	\$	136,131.32	\$	136,131.32	\$	136,131.32	\$	136,131.32		
				To	tal L	ifetime Mainter	nanc	e Cost (20 years):	\$	18,194,506		
	Total Lifetime Maintenance Cost (present value):											
						ANNUA	L TO	OTAL O&M ALT V	\$	714,294		



O&M Alternative VI - West Side WATERLINES

Input Variable	es	
Discount Rate:		2.25%
Repair Costs:	\$	118,370
Water Losses	\$	33,882
O&M	\$	566,404
Well equipment	\$	11,758

Well equipment	\$	11,758								
Year:		1		2		3		4		5
Repair Costs:	¢	121,033.53	\$	123,756.78	\$	126,541.31		129,388.49	\$	132,299.73
Water Loss:	\$	34,643.84	\$	35,423.33	\$	36,220.35	\$	37,035.31	\$	37,868.60
0&M	\$	579,148.31	\$	592,179.15	\$	605,503.18	\$	619,127.00	\$	633,057.36
Well equipment		12,022.56	\$	12,293.06	\$	12,569.66	\$	12,852.47	\$	13,141.65
Future Value	\$	746,848.24	\$	763,652.32	\$	780,834.50	\$	798,403.28	\$	816,367.35
Net Present Value:		730,413.92	\$	730,413.92	\$	730,413.92	\$	730,413.92	\$	730,413.92
Net Fleselit value.	Ą	730,413.32	Ą	730,413.32	Ą	730,413.32	Ą	730,413.32	Ą	730,413.32
Year:		6		7		8		9		10
Repair Costs:	\$	135,276.48	\$	138,320.20	\$	141,432.40	\$	144,614.63	\$	147,868.46
Water Loss:	\$	38,720.65	\$	39,591.86	\$	40,482.68	\$	41,393.54	\$	42,324.89
O&M	\$	647,301.15	\$	661,865.43	\$	676,757.40	\$	691,984.44	\$	707,554.09
Well equipment	\$	13,437.34	\$	13,739.68	\$	14,048.82	\$	14,364.92	\$	14,688.13
Future Value	\$	834,735.61	\$	853,517.17	\$	872,721.30	\$	892,357.53	\$	912,435.58
Net Present Value:	\$	730,413.92	\$	730,413.92	\$	730,413.92	\$	730,413.92	\$	730,413.92
Year:		11		12		13		14		15
Repair Costs:	\$	151,195.50	\$	154,597.40	\$	158,075.84	\$	161,632.55	\$	165,269.28
Water Loss:	\$	43,277.20	\$	44,250.94	\$	45,246.59	\$	46,264.64	\$	47,305.59
O&M	\$	723,474.06	\$	739,752.22	\$	756,396.65	\$	773,415.57	\$	790,817.42
Well equipment		15,018.62	\$	15,356.54	\$	15,702.06	\$	16,055.35	\$	16,416.60
Future Value	\$	932,965.38	\$	953,957.10	\$	975,421.13	\$	997,368.11	\$	1,019,808.89
Net Present Value:	\$	730,413.92	\$	730,413.92	\$	730,413.92	\$	730,413.92	\$	730,413.92
Year:		16		17		18		19		20
Repair Costs:		168,987.84	\$	172,790.06	\$	176,677.84	\$	180,653.09	\$	184,717.79
Water Loss:	\$	48,369.97	\$	49,458.29	\$	50,571.10	\$	51,708.95	\$	52,872.40
O&M	\$	808,610.81	\$	826,804.56	\$	845,407.66	\$	864,429.33	\$	883,878.99
Well equipment		16,785.97	\$	17,163.66	\$	17,549.84	\$	17,944.71	\$	18,348.47
Future Value	\$	1,042,754.59	\$	1,066,216.57	\$	1,090,206.44	\$	1,114,736.09	\$	1,139,817.65
Net Present Value:	\$	730,413.92	\$	730,413.92	\$	730,413.92		730,413.92	\$	730,413.92
							_	ce Cost (20 years):		18,605,125
_				Total L	.ifet			st (present value):		11,660,118 730,414
	ANNUAL TOTAL 0&M ALT VI									



O&M Alternative VII - Downtown WATERLINES

Input Variables											
Discount Rate:		2.25%									
Repair Costs:	\$	121,264									
Water Losses	\$	34,694									
O&M	\$	566,404									
Well equipment	\$	11,758									

Year:	1	2	3	4	5
Repair Costs:	\$ 123,992.44	\$ 126,782.27	\$ 129,634.87	\$ 132,551.66	\$ 135,534.07
Water Loss:	\$ 35,474.45	\$ 36,272.63	\$ 37,088.76	\$ 37,923.26	\$ 38,776.53
O&M	\$ 579,148.31	\$ 592,179.15	\$ 605,503.18	\$ 619,127.00	\$ 633,057.36
Well equipment	\$ 12,022.56	\$ 12,293.06	\$ 12,569.66	\$ 12,852.47	\$ 13,141.65
Future Value	\$ 750,637.76	\$ 767,527.11	\$ 784,796.47	\$ 802,454.39	\$ 820,509.61
Net Present Value:	\$ 734,120.06	\$ 734,120.06	\$ 734,120.06	\$ 734,120.06	\$ 734,120.06

Year:	6	7	8	9	10
Repair Costs:	\$ 138,583.58	\$ 141,701.71	\$ 144,890.00	\$ 148,150.03	\$ 151,483.40
Water Loss:	\$ 39,649.01	\$ 40,541.11	\$ 41,453.28	\$ 42,385.98	\$ 43,339.67
O&M	\$ 647,301.15	\$ 661,865.43	\$ 676,757.40	\$ 691,984.44	\$ 707,554.09
Well equipment	\$ 13,437.34	\$ 13,739.68	\$ 14,048.82	\$ 14,364.92	\$ 14,688.13
Future Value	\$ 838,971.08	\$ 857,847.93	\$ 877,149.51	\$ 896,885.37	\$ 917,065.29
Net Present Value:	\$ 734,120.06	\$ 734,120.06	\$ 734,120.06	\$ 734,120.06	\$ 734,120.06

Year:	11		12		13		14	15
Repair Costs:	\$ 154,891.78	\$	158,376.85	\$	161,940.33	\$	165,583.98	\$ 169,309.62
Water Loss:	\$ 44,314.81	\$	45,311.89	\$	46,331.41	\$	47,373.87	\$ 48,439.78
O&M	\$ 723,474.06	\$	739,752.22	\$	756,396.65	\$	773,415.57	\$ 790,817.42
Well equipment	\$ 15,018.62	\$	15,356.54	\$	15,702.06	\$	16,055.35	\$ 16,416.60
Future Value	\$ 937,699.26	\$	958,797.50	\$	980,370.44	\$	1,002,428.77	\$ 1,024,983.42
Net Present Value:	\$ 734.120.06	Ś	734.120.06	Ś	734.120.06	Ś	734.120.06	\$ 734.120.06

Year:		16		17		18		19		20	
Repair Costs:	\$	173,119.09	\$	177,014.27	\$	180,997.09	\$	185,069.52	\$	189,233.59	
Water Loss:	\$	49,529.67	\$	50,644.09	\$	51,783.58	\$	52,948.71	\$	54,140.06	
O&M	\$	808,610.81	\$	826,804.56	\$	845,407.66	\$	864,429.33	\$	883,878.99	
Well equipment	\$	16,785.97	\$	17,163.66	\$	17,549.84	\$	17,944.71	\$	18,348.47	
Future Value	\$	1,048,045.55	\$	1,071,626.57	\$	1,095,738.17	\$	1,120,392.28	\$	1,145,601.11	
Net Present Value:	\$	734,120.06	\$	734,120.06	\$	734,120.06	\$	734,120.06	\$	734,120.06	
	Total Lifetime Maintenance Cost (20 years):										
	Total Lifetime Maintenance Cost (present value):										

ANNUAL TOTAL O&M ALT VII \$

734,120



O&M Alternative VIII - Wiliamsburg WATERLINES

Input Variables											
Discount Rate:		2.25%									
Repair Costs:	\$	107,208									
Water Losses	\$	30,679									
O&M		\$566,404									
Well equipment		\$11,758									

Year:	1	2	3	4	5
Repair Costs:	\$ 109,620.59	\$ 112,087.05	\$ 114,609.01	\$ 117,187.71	\$ 119,824.44
Water Loss:	\$ 31,369.77	\$ 32,075.59	\$ 32,797.29	\$ 33,535.23	\$ 34,289.77
O&M	\$ 579,148.31	\$ 592,179.15	\$ 605,503.18	\$ 619,127.00	\$ 633,057.36
Well equipment	\$ 12,022.56	\$ 12,293.06	\$ 12,569.66	\$ 12,852.47	\$ 13,141.65
Future Value	\$ 732,161.23	\$ 748,634.86	\$ 765,479.14	\$ 782,702.42	\$ 800,313.22
Net Present Value:	\$ 716,050.10	\$ 716,050.10	\$ 716,050.10	\$ 716,050.10	\$ 716,050.10

Year:	6	7	8	9	10
Repair Costs:	\$ 122,520.49	\$ 125,277.20	\$ 128,095.93	\$ 130,978.09	\$ 133,925.10
Water Loss:	\$ 35,061.29	\$ 35,850.17	\$ 36,656.80	\$ 37,481.58	\$ 38,324.92
O&M	\$ 647,301.15	\$ 661,865.43	\$ 676,757.40	\$ 691,984.44	\$ 707,554.09
Well equipment	\$ 13,437.34	\$ 13,739.68	\$ 14,048.82	\$ 14,364.92	\$ 14,688.13
Future Value	\$ 695,799.78	\$ 711,455.28	\$ 727,463.02	\$ 743,830.94	\$ 760,567.14
Net Present Value:	\$ 716,050.10	\$ 716,050.10	\$ 716,050.10	\$ 716,050.10	\$ 716,050.10

Year:	11	12	13	14	15
Repair Costs:	\$ 136,938.42	\$ 140,019.53	\$ 143,169.97	\$ 146,391.29	\$ 149,685.10
Water Loss:	\$ 39,187.23	\$ 40,068.94	\$ 40,970.49	\$ 41,892.33	\$ 42,834.90
O&M	\$ 723,474.06	\$ 739,752.22	\$ 756,396.65	\$ 773,415.57	\$ 790,817.42
Well equipment	\$ 15,018.62	\$ 15,356.54	\$ 15,702.06	\$ 16,055.35	\$ 16,416.60
Future Value	\$ 914,618.31	\$ 935,197.23	\$ 956,239.16	\$ 977,754.54	\$ 999,754.02
Net Present Value:	\$ 716,050.10	\$ 716,050.10	\$ 716,050.10	\$ 716,050.10	\$ 716,050.10

Year:		16		17		18		19		20		
Repair Costs:	\$	153,053.01	\$	156,496.71	\$	160,017.88	\$	163,618.28	\$	167,299.69		
Water Loss:	\$	43,798.69	\$	44,784.16	\$	45,791.80	\$	46,822.12	\$	47,875.62		
O&M	\$	808,610.81	\$	826,804.56	\$	845,407.66	\$	864,429.33	\$	883,878.99		
Well equipment	\$	16,785.97	\$	17,163.66	\$	17,549.84	\$	17,944.71	\$	18,348.47		
Future Value	\$	1,022,248.49	\$	1,045,249.08	\$	1,068,767.18	\$	1,092,814.44	\$	1,117,402.77		
Net Present Value:	\$	716,050.10	\$	716,050.10	\$	716,050.10	\$	716,050.10	\$	716,050.10		
	Total Lifetime Maintenance Cost (20 years):											
	\$	11,430,818										

ANNUAL TOTAL O&M ALT VIII \$ 716,050



O&M Alternative IX - Airport 1 Pressure Tank

Input Variables											
Discount Rate:		2.25%									
Repair Costs:	\$	1,200									
Annual Running cost	\$	886									
O&M	\$	-									

Odivi	7							
					-			
Year:		1	2		3	4		5
Repair Costs:	\$	1,227.00	\$ 1,254.61	\$	1,282.84	\$ 1,311.70	\$	1,341.21
Running cost	\$	906.22	\$ 926.61	\$	947.45	\$ 968.77	\$	990.57
O&M	\$	-	\$ -	\$	-	\$ -	\$	1
Future Value	\$	2,133.22	\$ 2,181.21	\$	2,230.29	\$ 2,280.47	\$	2,331.78
Net Present Value:	\$	2,086.27	\$ 2,086.27	\$	2,086.27	\$ 2,086.27	\$	2,086.27
Year:		6	7		8	9		10
Repair Costs:	\$	1,371.39	\$ 1,402.25	\$	1,433.80	\$ 1,466.06	\$	1,499.04
Running cost	\$	1,012.86	\$ 1,035.65	\$	1,058.95	\$ 1,082.77	\$	1,107.14
O&M	\$	-	\$ -	\$	-	\$ -	\$	-
Future Value	\$	2,384.25	\$ 2,437.89	\$	2,492.75	\$ 2,548.83	\$	2,606.18
Net Present Value:	\$	2,086.27	\$ 2,086.27	\$	2,086.27	\$ 2,086.27	\$	2,086.27
Year:		11	12		13	14		15
Repair Costs:		1,532.77	\$ 1,567.26	\$	1,602.52	\$ 1,638.58	\$	1,675.45
Running cost	\$	1,132.05	\$ 1,157.52	\$	1,183.56	\$ 1,210.19	\$	1,237.42
O&M	\$	-	\$ -	\$	-	\$ -	\$	-
Future Value	\$	2,664.82	\$ 2,724.78	\$	2,786.09	\$ 2,848.77	\$	2,912.87
Net Present Value:	\$	2,086.27	\$ 2,086.27	\$	2,086.27	\$ 2,086.27	\$	2,086.27
							_	
Year:		16	17		18	19		20
Repair Costs:		1,713.15	\$ 1,751.69	\$	1,791.10	\$ 1,831.40	\$	1,872.61
Running cost		1,265.26	\$ 1,293.73	\$	1,322.84	\$ 1,352.61	\$	1,383.04
O&M	\$	-	\$ -	\$	-	\$ -	\$	-
Future Value	\$	2,978.41	\$ 3,045.42	\$	3,113.95	\$ 3,184.01	\$	3,255.65
Net Present Value:	\$	2,086.27	\$ 2,086.27	\$	2,086.27	\$ 2,086.27	\$	2,086.27
						e Cost (20 years):	\$	53,142
			Total Lif	etin		t (present value):		33,305
								2.000

2,086

ANNUAL TOTAL O&M ALT IX \$



O&M Alternative X - Airport 2 Without Fire Flow

Input Variables											
Discount Rate:		2.25%									
Repair Costs:	\$	1,200.00									
Annual Running cost	\$	886									
O&M	\$	-									

-								
	1	2		3		4		5
\$ 1,227.00	\$	1,254.61	\$	1,282.84	\$	1,311.70	\$	1,341.21
\$ 906.22	2 \$	926.61	\$	947.45	\$	968.77	\$	990.57
\$ -	\$	-	\$	-	\$	-	\$	-
\$ 2,133.22	2 \$	2,181.21	\$	2,230.29	\$	2,280.47	\$	2,331.78
\$ 2,086.27	7 \$	2,086.27	\$	2,086.27	\$	2,086.27	\$	2,086.27
	6	7		8		9		10
\$ 1,371.39	\$	1,402.25	\$	1,433.80	\$	1,466.06	\$	1,499.04
\$ 1,012.86	5 \$	1,035.65	\$	1,058.95	\$	1,082.77	\$	1,107.14
\$ -	\$	-	\$	-	\$	-	\$	-
	_	•	\$	2,492.75	\$	2,548.83	\$	2,606.18
\$ 2,086.27	7 \$	2,086.27	\$	2,086.27	\$	2,086.27	\$	2,086.27
					_			
		12		13		14		15
	_		\$	1,602.52	\$	1,638.58	\$	1,675.45
	_		-	1,183.56		1,210.19	_	1,237.42
\$ -	\$		\$	-	\$	-	\$	-
			\$	2,786.09	_	2,848.77	\$	2,912.87
\$ 2,086.27	7 \$	2,086.27	\$	2,086.27	\$	2,086.27	\$	2,086.27
	-		_	-	_	-	_	
	_					-		20
		·	<u> </u>				<u> </u>	1,872.61
	_		-	1,322.84		1,352.61	<u> </u>	1,383.04
			_	-	_	-	<u> </u>	
\$ 2,978.41	. \$		-	•			_	3,255.65
								2 22 2 2 =
\$ 2,086.27	\$,	\$	2,086.27	\$	2,086.27 ce Cost (20 years):	\$	2,086.27 53,142
	\$ 1,227.00 \$ 906.22 \$ - \$ 2,133.22 \$ 2,086.27 \$ 1,371.39 \$ 1,012.86 \$ 2,384.25 \$ 2,086.27 \$ 1,532.77 \$ 1,132.05 \$ - \$ 2,664.82 \$ 2,086.27	\$ 1,227.00 \$ 906.22 \$ \$ - \$ \$ 2,384.25 \$ \$ 1,132.05 \$ \$ 1,713.15 \$ \$ 1,265.26 \$ \$ - \$ \$	\$ 1,227.00 \$ 1,254.61 \$ 906.22 \$ 926.61 \$ - \$ - \$ - \$ \$ 2,181.21 \$ 2,086.27 \$ 2,086.27 \$ 1,371.39 \$ 1,402.25 \$ 1,012.86 \$ 1,035.65 \$ - \$ - \$ 2,384.25 \$ 2,086.27 \$ 2,086.27 \$ 1,567.26 \$ 1,132.05 \$ 1,157.52 \$ - \$ - \$ - \$ 2,664.82 \$ 2,724.78 \$ 2,086.27 \$ 2,086.27 \$ 1,751.69 \$ 1,265.26 \$ 1,293.73 \$ - \$ - \$ - \$ - \$ - \$ - \$ - \$ - \$ - \$	1	1 2 3 3 4 4 4 5 4 5 4 5 4 5 5	Table Tabl	1 2 3 4 4 5 1,227.00 5 1,254.61 5 1,282.84 5 1,311.70 5 906.22 5 926.61 5 947.45 5 968.77 5 - 5 - 5 - 5 5 - 5 5	1 2 3 4

Total Lifetime Maintenance Cost (present value): \$

ANNUAL TOTAL O&M ALT X \$

33,305

2,086.27



O&M Alternative XI - Airport 3 With Fire Flow

Input Variables											
Discount Rate:		2.25%									
Repair Costs:	\$	2,791									
Annual Running cost	\$	37,795									
O&M	\$	-									

OQIVI	Ç					
Year:		1	2	3	4	5
Repair Costs:	\$	2,853.41	\$ 2,917.61	\$ 2,983.26	\$ 3,050.38	\$ 3,119.02
Running cost	\$	38,645.09	\$ 39,514.61	\$ 40,403.69	\$ 41,312.77	\$ 42,242.31
O&M	\$	-	\$ -	\$ -	\$ -	\$ -
Future Value	\$	41,498.51	\$ 42,432.22	\$ 43,386.95	\$ 44,363.16	\$ 45,361.33
Net Present Value:	\$	40,585.34	\$ 40,585.34	\$ 40,585.34	\$ 40,585.34	\$ 40,585.34
Year:		6	7	8	9	10
Repair Costs:	\$	3,189.20	\$ 3,260.95	\$ 3,334.32	\$ 3,409.35	\$ 3,486.06
Running cost	\$	43,192.76	\$ 44,164.60	\$ 45,158.30	\$ 46,174.36	\$ 47,213.29
O&M	\$	-	\$ -	\$ -	\$ -	\$ -
Future Value	\$	46,381.96	\$ 47,425.55	\$ 48,492.63	\$ 49,583.71	\$ 50,699.34
Net Present Value:	\$	40,585.34	\$ 40,585.34	\$ 40,585.34	\$ 40,585.34	\$ 40,585.34
Year:		11	12	13	14	15
Repair Costs:	\$	3,564.49	\$ 3,644.69	\$ 3,726.70	\$ 3,810.55	\$ 3,896.29
Running cost	\$	48,275.58	\$ 49,361.79	\$ 50,472.43	\$ 51,608.05	\$ 52,769.24
O&M	\$	-	\$ 	\$ -	\$ -	\$ -
Future Value	\$	51,840.08	\$ 53,006.48	\$ 54,199.13	\$ 55,418.61	\$ 56,665.52
Net Present Value:	\$	40,585.34	\$ 40,585.34	\$ 40,585.34	\$ 40,585.34	\$ 40,585.34
Year:		16	17	18	19	20
Repair Costs:		3,983.95	\$ 4,073.59	\$ 4,165.25	\$ 4,258.97	\$ 4,354.79
Running cost	\$	53,956.54	\$ 55,170.57	\$ 56,411.90	\$ 57,681.17	\$ 58,979.00
O&M	\$	-	\$ -	\$ -	\$ -	\$ -
Future Value	\$	57,940.50	\$ 59,244.16	\$ 60,577.15	\$ 61,940.14	\$ 63,333.79
Net Present Value:	\$	40,585.34	\$ 40,585.34	\$ 40,585.34	\$ 40,585.34	\$ 40,585.34

Total Lifetime Maintenance Cost (present value): \$

ANNUAL TOTAL O&M ALT XI \$

Total Lifetime Maintenance Cost (20 years): \$

647,893 40,585

1,033,791



O&M Alternative XII- Airport 4 VFD Well Pump

Input Variables											
Discount Rate:	2.25%										
Repair Costs:	\$ 1,200										
Annual Running cost	\$ 1,182										
O&M	\$ -										

Year:	1	2	3	4	5
Repair Costs:	\$ 1,227.00	\$ 1,254.61	\$ 1,282.84	\$ 1,311.70	\$ 1,341.21
Running cost	\$ 1,208.29	\$ 1,235.47	\$ 1,263.27	\$ 1,291.70	\$ 1,320.76
O&M	\$ -	\$ -	\$ -	\$ -	\$ -
Future Value	\$ 2,435.29	\$ 2,490.08	\$ 2,546.11	\$ 2,603.40	\$ 2,661.97
Net Present Value:	\$ 2,381.70	\$ 2,381.70	\$ 2,381.70	\$ 2,381.70	\$ 2,381.70

Year:	6	7	8	9	10
Repair Costs:	\$ 1,371.39	\$ 1,402.25	\$ 1,433.80	\$ 1,466.06	\$ 1,499.04
Running cost	\$ 1,350.48	\$ 1,380.86	\$ 1,411.93	\$ 1,443.70	\$ 1,476.18
O&M	\$ -	\$ 1	\$ -	\$ -	\$ -
Future Value	\$ 2,721.87	\$ 2,783.11	\$ 2,845.73	\$ 2,909.76	\$ 2,975.23
Net Present Value:	\$ 2,381.70	\$ 2,381.70	\$ 2,381.70	\$ 2,381.70	\$ 2,381.70

Year:	11	12	13	14	15
Repair Costs:	\$ 1,532.77	\$ 1,567.26	\$ 1,602.52	\$ 1,638.58	\$ 1,675.45
Running cost	\$ 1,509.40	\$ 1,543.36	\$ 1,578.08	\$ 1,613.59	\$ 1,649.90
O&M	\$ -	\$ -	\$ -	\$ -	\$ -
Future Value	\$ 3,042.17	\$ 3,110.62	\$ 3,180.61	\$ 3,252.17	\$ 3,325.34
Net Present Value:	\$ 2,381.70	\$ 2,381.70	\$ 2,381.70	\$ 2,381.70	\$ 2,381.70

Year:		16		17		18		19		20		
Repair Costs:	\$	1,713.15	\$	1,751.69	\$	1,791.10	\$	1,831.40	\$	1,872.61		
Running cost	\$	1,687.02	\$	1,724.98	\$	1,763.79	\$	1,803.47	\$	1,844.05		
O&M	\$	-	\$	-	\$	-	\$	-	\$	-		
Future Value	\$	3,400.16	\$	3,476.67	\$	3,554.89	\$	3,634.88	\$	3,716.66		
Net Present Value:	\$	2,381.70	\$	2,381.70	\$	2,381.70	\$	2,381.70	\$	2,381.70		
	Total Lifetime Maintenance Cost (20 years):											
	Total Lifetime Maintenance Cost (present value):											
						ANNUAL	TO	TAL O&M ALT XII	\$	2,382		



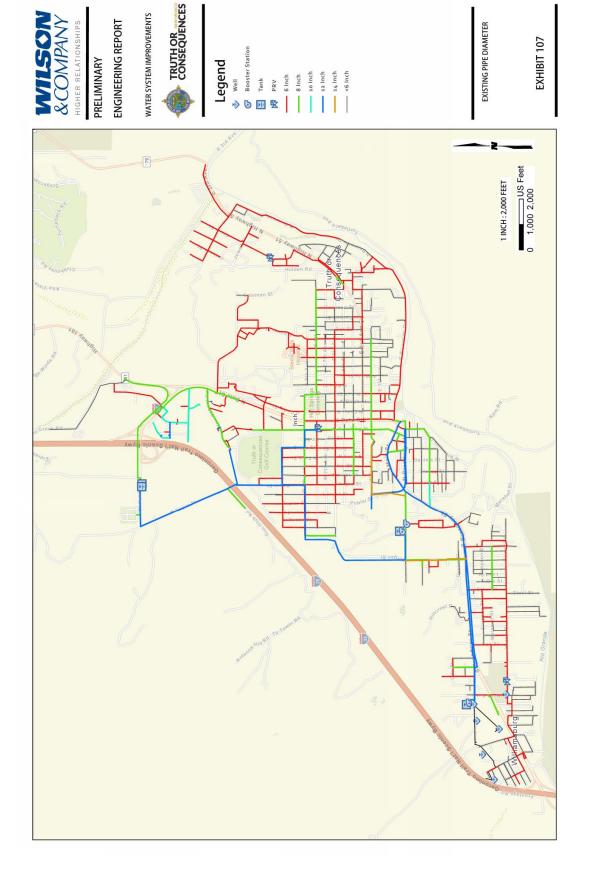
APPENDIX 6- EXHIBITS

TABLE OF CONTENT:

- O EXHIBIT 107: EXISTING PIPE DIAMETER
- o EXHIBIT 108: AIRPORT OVERVIEW
- o EXHIBIT 109: AIRPORT OVERVIEW 2
- o EXHIBIT 110: SYSTEM COMPLETION OVERVIEW
- o EXHIBIT 111: EXISTING SYSTEM OVERVIEW USDA TOPOGRAPHIC MAP





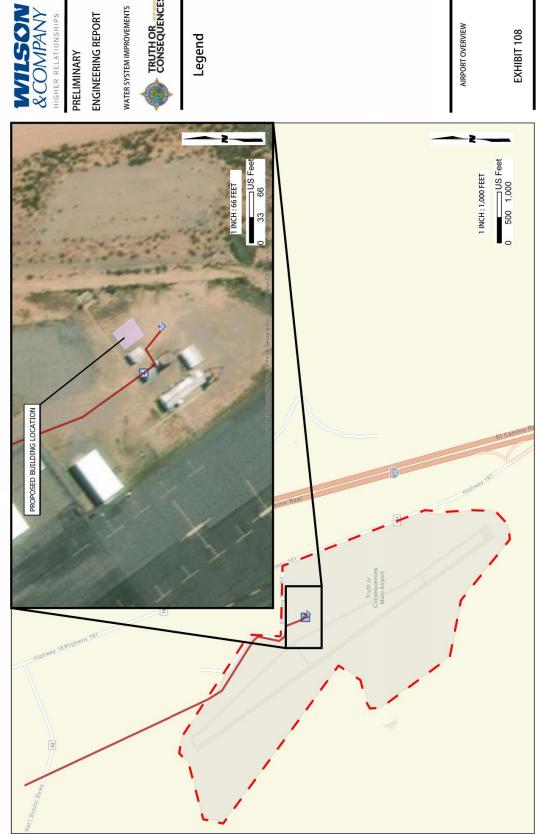






City – Wide Water System Improvements Preliminary Engineering Report









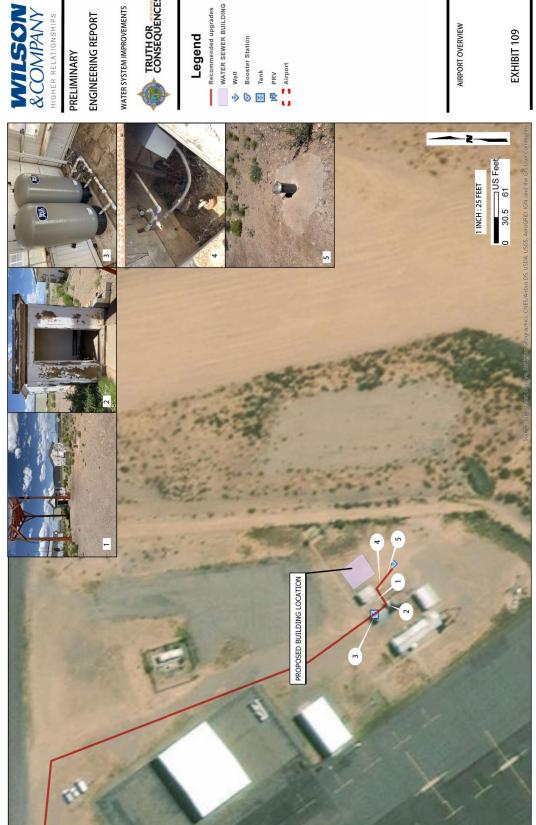






Legend











































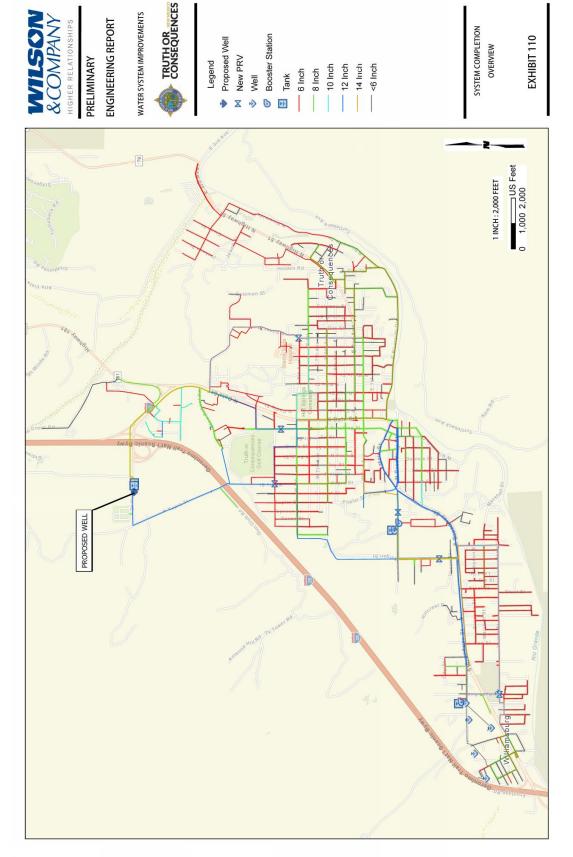


















PRELIMINARY

ENGINEERING REPORT

WATER SYSTEM IMPROVEMENTS



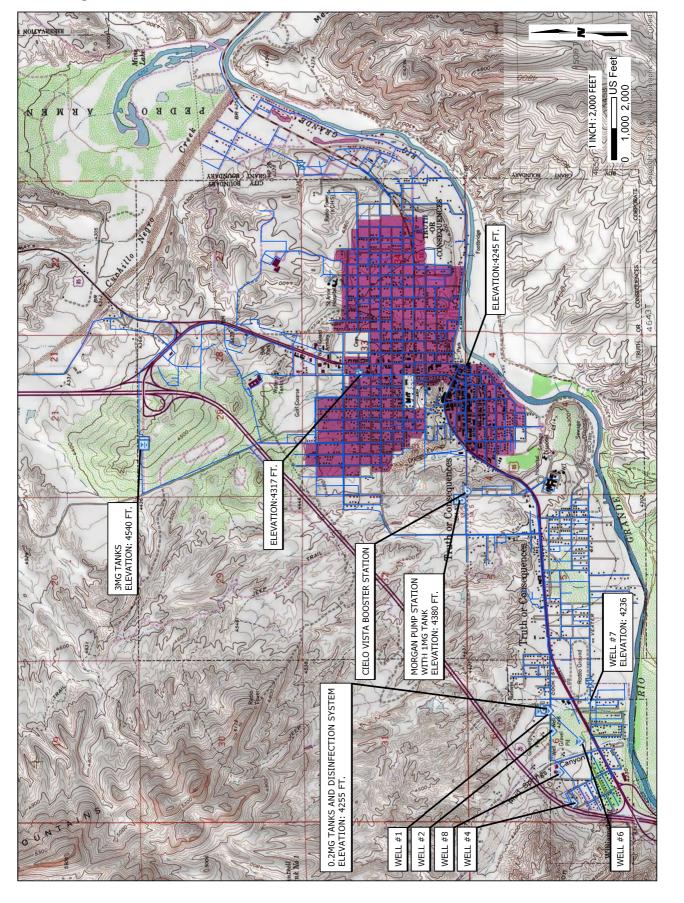
EXISTING SYSTEM Legend

G Booster Station M

Waterline

EXISTING SYSTEM OVERVIEW USDA TOPOGRAPHIC MAP

EXHIBIT 111





APPENDIX 7- SHORT LIVE ASSET RESERVE

TABLE OF CONTENT:

o SHORT LIVE ASSET TABLE CITY OF TRUTH OR CONSEQUENCES





Asset Section Section Section Section Section Section Section Sect	Description	Estimate	d Life Cycle
Cook Street TreatmentFacility Pump #2 Motor \$ 16,070.00	Description	1-5 years	6-10 years
Well #7 Pump Motor			
Wolf life Pump Motice	· ·		
Well ## Pump Motor	'		
Well ## Pump Motor			
Mell #4 Pump Motor	<u> </u>		
Ciclo Vista Pump Station Pump No. 2 Motor \$ 700.00			
Dielo Vista Pump Station Pump No. 1 Motor \$ 7,000.00			
Booster Pump Station No. 2 Pump No. 1 Motor \$ 16,070.00			
Cook St. Treatment Facility Pump No. 1 Motor \$ 16,070.00		•	
\$31,200.00	I I	, , , , , , , ,	
Well #2 Pump S 3,310. Well #3 Pump S 57,590. Well #4 Pump S 59,510. Booster Pump Station #2 Pump S 39,510. Booster Pump Station #2 Pump S 7,690. Booster Pump Station #2 Pump S 39,510. Well #4 Pump S 31,190. Cielo Vista Pump Station Pump No. 1 S 31,190. Cielo Vista Pump Station Pump No. 3 S 10,400. Pershing Booster Pump Station No. 2, Pump No. 1* S 31,190. Cielo Vista Pump Station Pump No. 3 S 10,400. Pershing Booster Pump Station No. 2, Pump No. 2* S - Cook St. Treatment Facility Flow Meter S 5,200. Well No. 2 Flow Meter S 5,200. Well No. 1 Flow Meter S 5,200. Well No. 1 Flow Meter S 5,200. Well No. 8 Flow Meter S 5,200. Well No. 6 Flow Meter S 5,200. Well No. 1 Flow Meter S 5,200. Well No. 2 Flow Meter S 5,200. Well No. 3 Flow Meter S 5,200. Well No. 3 Flow Meter S 5,200. Well No. 3 Flow Meter S 5,200. Well			
Mell #8 Pump		Ψ 31,200.00	\$ 3,310.00
Mell #7 Pump			
Mell #6 Pump \$ 3,95101.			
Sooster Pump Station #2 Pump \$ 7,690. Well #4 Pump \$ 12,420. Well #4 Pump \$ 3,3910. Solido Vista Pump Station Pump No. 1 \$ 3,1910. Solido Vista Pump Station Pump No. 3 \$ 10,400. Pershing Booster Pump Station No. 2, Pump No. 1* \$ 1,420. Pershing Booster Pump Station No. 2, Pump No. 1* \$ 1,400. Pershing Booster Pump Station No. 2, Pump No. 2* \$ 2,200. Solido Vista Pump Station No. 2, Pump No. 2* \$ 3,200. Solido Vista Pump Station No. 2, Pump No. 2* \$ 3,200. Well No. 2 Flow Meter \$ 3,200. Well No. 2 Flow Meter \$ 5,200. Well No. 7 Flow Meter \$ 5,200. Well No. 1 Flow Meter \$ 5,200. Well No. 4 Flow Meter \$ 5,200. Well No. 1 Flow Meter \$ 5,200. Well No. 1 Pump Electrical System* \$ 5,200. Well No. 2 Pump Electrical System* \$ 7,820. Well No. 2 Pump Electrical System* \$ 9,960. Well No. 4 pump Electrical System \$ 6,960. Well No. 5 pump Electrical System \$ 6,960. Well No. 8 pump Electrical System \$ 6,960. Well No. 8 pump Electrical System \$ 6,960. Well No. 8 pump Electrical System \$ 6,960. Well No. 9 pump Electrical System \$ 6,960. Well No. 9 pump Electrical System \$ 6,960. Well No. 9 pump Electrical System \$ 6,960. Solo No. 9 pump Electrical Syst			
Well #1 Pump			,
Mell #A Pump	Well #1 Pump		
Cielo Vista Pump Station Pump No. 1 \$ 31,190.			, ,
Signature Station Pump No. 3 Signature Signatu			
Pershing Booster Pump Station No. 2, Pump No. 1* Pershing Booster Pump Station No. 2, Pump No. 2* Cook St. Treatment Facility Flow Meter* Well #2 Pump Well 1No. 2 Flow Meter* Well No. 2 Flow Meter Well No. 3 Flow Meter Well No. 5 Flow Meter Well No. 6 Flow Meter Well No. 6 Flow Meter S. 5,200.0 Well No. 6 Flow Meter S. 5,200.0 Well No. 6 Flow Meter S. 5,200.0 Well No. 1 Flow Meter S. 6,200.0 Well No. 1 Pump Electrical System S. 4,480.0 Well No. 1 pump Electrical System S. 4,480.0 Well No. 1 pump Electrical System S. 6,9860.0 Well No. 7 pump Electrical System S. 6,9860.0 Well No. 8 pump Electrical System S. 6,9860.0 Well No. 8 pump Electrical System S. 6,9860.0 Well No. 8 pump Station No. 2 Electrical System S. 6,9860.0 Well No. 8 pump Station Electrical System S. 6,9860.0 Serbing Booster pump Station No. 2 Electrical System S. 5,200.0 S. 5,200.0 Well No. 8 pump Electrical System S. 6,9860.0 S. 5,200.0	Cielo Vista Pump Station Pump No. 3		
Pershing Booster Pump StationNo. 2, Pump No. 2* Well War Wel	Pershing Booster Pump Station No. 2, Pump No. 1*		
Scok St. Treatment Facility Flow Meter* \$ 16,550. Well No. 2 Flow Meter \$ 5,200. Well No. 2 Flow Meter \$ 5,200. Well No. 1 Flow Meter \$ 5,200. Well No. 6 Flow Meter \$ 5,200. Well No. 1 Flow Meter \$ 5,200. Well No. 2 pump Electrical System \$ 47,480. Well No. 1 pump Electrical System \$ 47,480. Well No. 1 pump Electrical System \$ 6,980.0 Well No. 2 pump Electrical System \$ 6,980.0 Well No. 3 pump Electrical System \$ 6,980.0 Well No. 7 pump Electrical System \$ 6,980.0 Well No. 9 pump Electrical System \$ 6,980.0 Selectrical System \$ 6,980	Pershing Booster Pump StationNo. 2, Pump No. 2*		•
Well No. 2 Flow Meter	Cook St. Treatment Facility Flow Meter*		\$ -
Well No. 7 Flow Meter	Well #2 Pump		\$ 16,550.00
Well No. 8 Flow Meter	Well No. 2 Flow Meter*		\$ -
Well No. 4 Flow Meter \$ 5,200.0	Well No. 7 Flow Meter		\$ 5,200.00
Well No. 6 Flow Meter	Well No. 8 Flow Meter		\$ 5,200.00
Secoster Pump Station No. 2 Flow Meter \$ 5,200.0	Well No. 4 Flow Meter		\$ 5,200.00
Well No. 1 Flow Meter* \$	Well No. 6 Flow Meter		
Well No. 2 pump Electrical System* \$ 47,480.0			
Well No. 4 pump Electrical System			
Well No. 1 pump Electrical System* \$ 69,860.0 \$ 69,			
Well No. 6 pump Electrical System			
Well No. 7 pump Electrical System \$ 67,820.0 \$ 69,860.0 \$ 69,8			'
Well No. 8 pump Electrical System \$ 69,860.0	•		. ,
Science Station Stat			
Pershing Booster pump Station No. 2 Electrical System* Pershing Booster Pump Station No. 2 Back-up Generator* Cook St Treatment Facility Electrical System* SCADA System RTUs* SCADA System RTUs* SCADA System Software Gas-Chlorination System* S250 HP Vertical Turbine Booster Pump x2 Pump Control Panel/Soft Starter* S260 HP Vertical Turbine Booster Pump x2 Pump Control Panel/Soft Starter* S270 MG Electrical/Control System* S280 MG Steel Storage Tank on Morgan St. S281 MG Steel Storage Tank on Cemetery Rd. S281 MG Steel Storage Tank on Cemetery Rd. S281 MG Steel Storage Tank on Cemetery Rd. S282 MG Steel Storage Tank S284 MS Steel Storage Tank S285 MS Subtotal of Short-Lived Assets (per period) S286 MS Steel Storage Tank S286 MS Subtotal of Short-Lived Assets (per year) S286 MS Steel Storage Tank S286 MS Subtotal of Short-Lived Assets (per month) S286 MS Steel Storage Tank S286 MS Subtotal of Short-Lived Assets (per month) S286 MS Steel Storage Tank S286 MS Subtotal of Short-Lived Assets (per month) S286 MS Subtotal of Short-Lived Assets (per month) S286 MS S486 MS			
Pershing Booster Pump Station No. 2 Back-up Generator* Cook St Treatment Facility Electrical System* SCADA System RTUs* SCADA System Software Gas-Chlorination System* S250 HP Vertical Turbine Booster Pump x2 Pump Control Panel/Soft Starter* D.2 MG Electrical/Control System* S.3 MG Electrical/Control System* S.4 Gas-Chlorination System* D.3 MG Steel Storage Tank on Morgan St. 1.2 MG Steel Storage Tank on Cemetery Rd. S.3.0 MG Steel Storage Tank on Steel Storage Tank S.3.0 MG Steel Storage Tank S.3.0 MG Steel Storage Tank S.3.0 MG Steel Storage Tank on Steel Storage Tank S.3.0 MG Steel Storage Tank on Steel Storage Tank S.3.0 MG Steel Storage Tank Steel S			. ,
Scand System RTUs* \$ \$ \$ \$ \$ \$ \$ \$ \$	0 1 1		
SCADA System RTUs* \$ 38,600.06			· · · · · · · · · · · · · · · · · · ·
\$38,600.06 \$38			•
Sas-Chlorination System* \$ 249,500.0 Pump Control Panel/Soft Starter* \$ 249,500.0 Sum G Electrical/Control System* \$ 261,486.0 Sum G Steel Storage Tank on Morgan St. \$ 261,486.0 Sum G Steel Storage Tank on Cemetery Rd. \$ 261,486.0 Sum G Steel Storage Tank on Cemetery Rd. \$ 261,486.0 Sum G Steel Storage Tank \$ 39,223.0 Sum G Steel Stora			
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Subtotal of Short-Lived Assets (per period) \$ 97,530.00 \$ 1,628,407.0 Subtotal of Short-Lived Assets (per year) \$ 19,506.00 \$ 162,840.0 Subtotal of Short-Lived Assets (per month) \$ 1,625.50 \$ 13,570.0 Total of Short-Lived Assets (1-10 years) \$ 1,725,93 Total Annual Reserve Deposit, Short-Lived Assets (1-10 years, per year) \$ 182,34	New Well Pump and Motor		
Subtotal of Short-Lived Assets (per year) \$ 19,506.00 \$ 162,840.7 Subtotal of Short-Lived Assets (per month) \$ 1,625.50 \$ 13,570.0 Total of Short-Lived Assets (1-10 years) \$ 1,725,93 Total Annual Reserve Deposit, Short-Lived Assets (1-10 years, per year) \$ 182,34	Subtotal of Short-Lived Assets (per period)	\$ 97,530.00	
Subtotal of Short-Lived Assets (per month) \$ 1,625.50 \$ 13,570.0 Total of Short-Lived Assets (1-10 years) \$ 1,725,93 Total Annual Reserve Deposit, Short-Lived Assets (1-10 years, per year) \$ 182,34	Subtotal of Short-Lived Assets (per year)		
Total of Short-Lived Assets (1-10 years) \$ 1,725,93 Total Annual Reserve Deposit, Short-Lived Assets (1-10 years, per year) \$ 182,34	Subtotal of Short-Lived Assets (per month)	\$ 1,625.50	\$ 13,570.06
			1,725,937
	Total Annual Reserve Deposit, Short-Lived Assets (1-10 years, per year	\$	182,347
i otal Monthly Reserve Deposit, Short-Lived Assets (1-10 years, per month) \$ 15,19	Total Monthly Reserve Deposit, Short-Lived Assets (1-10 years, per month)		15,196

APPENDIX 8- WATER LOSSES

TABLE OF CONTENT:

o WATER LOSSES SUMMARY TABLE CITY OF TRUTH OR CONSEQUENCES



Summary of Water Losses

	Full System						Al	ternative II - Co	mp	lete System)	
Alternative %	-							96.60	0%			
Year	2016	2017	2018		2019		2016	2017		2018		2019
Losses (GAL)	105,123,718	108,683,000	141,018,000	12	27,534,000		101,549,512	104,987,778	1	36,223,388	1	23,197,844
Annual Losses	\$ 183,967	\$ 190,195	\$ 246,782	\$	223,185	\$	177,712	\$ 183,729	\$	238,391	\$	215,596
•	Alterna	ative III - Syste	m Performance Up	odgra	ade		Alternativ	e III A - System	Hig	h pressure	Sou	ltion
Alternative %		3	7.80%					26.70	0%			
Year	2016	2017			2019		2016	2017		2018		2019
Losses (GAL)	39,736,765	41,082,174			48,207,852		28,068,033	29,018,361		37,651,806		34,051,578
Annual Losses	\$ 69,539	· · ·	' '		84,364	\$	49,119	\$ 50,782		65,891	\$	59,590
	Alternativ	•	Redundancy and	Нус	draulic		Alternative	IIIC -Adiditona			ror	nance
			e Enhancements					Enhance	_	nts		
Alternative %			5.30%					5.20	1%			
Year	2016	_	2018		2019		2016	2017		2018		2019
Losses (GAL)	6,622,794	, ,			8,034,642		5,466,433	5,651,516		7,332,936		6,631,768
Annual Losses	\$ 11,590	. ,		\$	14,061	\$	9,566	\$ 9,890		12,833	\$	11,606
			IV - North Side			Alternative V - East Side						
Alternative %			5.10%					23.10	6%			
Year	2016		2018		2019	_	2016	2017		2018		2019
Losses (GAL)	5,361,310	, ,			6,504,234		24,346,653	25,170,983		32,659,769		29,536,874
Annual Losses	\$ 9,382			\$	11,382	\$	42,607	\$ 44,049	_ '	57,155	\$	51,690
			VI - West Side					Alternative VII		owntown		
Alternative %			4.08%	1				12.02	2%	1		
Year	2016	2017	2018		2019		2016	2017		2018		2019
Losses (GAL)	14,801,419		, ,		17,956,787		12,635,871	13,063,697	_	16,950,364		15,329,587
Annual Losses	\$ 25,902	\$ 26,779	. ,	Ş	31,424	\$	22,113	\$ 22,861	\$	29,663	\$	26,827
			'III - Williamsburg			ı.						
Alternative %			2.20%									
Year	2016		2018		2019							
Losses (GAL)	23,337,465	, ,	, ,		28,312,548							
Annual Losses	\$ 40,841	\$ 42,223	\$ 54,785	\$	49,547							

APPENDIX 9- FIRE HYDRANT BREAKDOWN

CONTENT:

- o FIRE HYDRANT SUMMARY TABLE AGE/PRESSURE CAPACITY BREAKDOWN
- FIRE FLOW HYDRANT TESTING REPORT (THIS PAGE REPORT IS NOT INCLUDED IN THIS APPENDIX, AVALIABLE PER REQUEST)



FIRE HYDRANT SUMMARY TABLE AGE/PRESSURE CAPACITY BREAKDOWN



Fire Hydrant Age Breakdown

Age (Years)	Number of Hydrants	Percentage of Total
> 50	25	7.8%
> 40	42	13.1%
> 30	45	14.0%
> 3	209	65.1%

Fire Hydrant Pressure Capacity Breakdown

•	•	
Pressure Capacity	Number of	Percentage of
(GPM)	Hydrants	Total
< 1500	16	5.3%
< 2500	123	40.5%
< 3500	152	50.0%
< 5700	13	4.3%

APPENDIX 10- NMED WATER SYSTEM VIOLATIONS

CONTENT:

NMED WATER SYSTEM VIOLATIONS



New Mexico Environment Department		UOCP Operator Lookup	Drinking Wate	er Program	
County Map	of NM	Water System Search	Helj		
		Vater System Detail Informatio	n		
Water System No.:	NM3514327	A COMPANY OF THE PARTY OF THE P	Federal Type:	C	
	TRUTH OR O	CONSEQUENCES	Federal Source:	GW	
Principal County Served:	SIERRA		System Status:	A	
Principal City Served:	TRUTH OR O	CONSEQUENCES	Activity Date:	06-01-1977	

Group Violations								
Fed Fiscal Year	Determ. Date	Violation Type	Violation Name	Analyte Group	Analyte Group Name			
2017	11-18-2016	27	MONITORING, ROUTINE (DBP), MAJOR	DBP2	DBP STAGE 2			
2016	11-03-2015	27	MONITORING, ROUTINE (DBP), MAJOR	DBP2	DBP STAGE 2			
2006	11-30-2005	03	MONITORING, ROUTINE MAJOR	socs	OLD SOCS			

		الرورات	Individual Violation	S				
Violation No.	Determ. Date	Violation Type	Violation Name	Analyte Code	Analyte Name	RTC Exp.	RTC Imp.	RTC Other
2018-36616	02-09- 2018	75	PUBLIC NOTICE RULE LINKED TO VIOLATION	7500	PUBLIC NOTICE	Y		
2017-36615	02-23- 2017	52	FOLLOW-UP OR ROUTINE TAP M/R (LCR)	5000	LEAD & COPPER RULE	Y		
2017-36614	12-19- 2016	72	CCR ADEQUACY/AVAILABILITY/CONTENT	7000	CONSUMER CONFIDENCE RULE	Y		
2017-36611	11-23- 2016	3A	MONITORING, ROUTINE, MINOR (RTCR)	3014	E. COLI	Y		
2016-36610	08-03- 2016	71	CCR REPORT	7000	CONSUMER CONFIDENCE RULE	Y		
2016-36609	07-20- 2016	52	FOLLOW-UP OR ROUTINE TAP M/R (LCR)	5000	LEAD & COPPER RULE	Y		
2006-36606	06-12- 2006	22	MCL (TCR), MONTHLY	3100	COLIFORM (TCR)	Y	Y	
2004-304	08-03- 2004	51	INITIAL TAP SAMPLING (LCR)	5000	LEAD & COPPER RULE	Y		
2000-33400	10-10- 2000	24	MONITORING (TCR), ROUTINE MINOR	3100	COLIFORM (TCR)	Y	Y	

^{*} Denotes violation began in last 6 months but is currently eligible for implicit RTC.

RTC EXP denotes violation has any of the following enforcement actions: SOX, EOX.

RTC IMP denotes SWTR or TCR violation does not have a violation in the following 6 months.



New Mexico Environment Department		UOCP Operator Lookup	Drinking Wate	Drinking Water Program		
County Map	of NM	Water System Search	Help)		
		Vater System Detail Information)H			
Water System No.:	NM3501427		Federal Type:	NC		
Water System Name:	TRUTH OR C	CONSEQUENCES MUNICIPAL	Federal Source:	GW		
Principal County Served:	SIERRA		System Status:	A		
Principal City Served:	TRUTH OR C	CONSEQUENCES	Activity Date:	08-27-2018		

			Group Violation	1S	
Fed Fiscal Year	Determ. Date	Violation Type	Violation Name	Analyte Group	Analyte Group Name

Individual Violations									
Violation No.	Determ. Date	Violation Type	Violation Name	Analyte Code	Analyte Name	RTC Exp.	RTC lmp.	RTC Other	
2019-3	01-16-2019	3.4	MONITORING, ROUTINE, MAJOR (RTCR)	3014	E. COLI	Y			
2019-2	12-18-2018	3.4	MONITORING, ROUTINE, MAJOR (RTCR)	3014	E. COLI	Y			
2019-1	11-21-2018	3 4	MONITORING, ROUTINE, MAJOR (RTCR)	3014	E. COLI	Y			

^{*} Denotes violation began in last 6 months but is currently eligible for implicit RTC.

RTC EXP denotes violation has any of the following enforcement actions: SOX, EOX.

RTC IMP denotes SWTR or TCR violation does not have a violation in the following 6 months.

RTC Other denotes violation has any of the following enforcement actions:

EF&, EF/, EF9, EFK, EFL, EFQ, EFV, EO0, EO6, SF&, SF9, SFK, SFL, SFO, SFQ, SFV, SF0, SF6



APPENDIX 11- WATER SYSTEM ASSET MANAGEMENT PLAN

CONTENT:

o AMP-SMITH ENGINEERING





Water System Asset Management Plan

City of Truth or Consequences



PREPARED BY:

Smith Engineering Company 2201 San Pedro NE, Building 4, Suite 200 Albuquerque, NM 87110 (505)884-0700 www.smithengineering.pro

November 2014 April 2015 (rev 1) March 2017 (rev 2)

Smith Project No. 116102

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Revision History

Version	DATE	DETAILS OF CHANGE
1	November 2014	INITIAL
2	APRIL 2015	Addressed NMED Comments
3	MARCH 2017	UPDATE

THIS MANUAL IS A LIVING DOCUMENT, AND AS SUCH IT SHOULD BE ENHANCED THROUGHOUT THE LIFE OF THE WATER SYSTEM. THEREFORE, CHANGES IN ASSETS, OPERATIONAL CHARACTERISTICS, MAINTENANCE PROCEDURES, AND O&M DATA FOR BUILDINGS, EQUIPMENT, AND TREATMENT SYSTEMS SHOULD BE UPDATED AS EQUIPMENT/SYSTEMS ARE DEVELOPED, DELETED, MODIFIED AND/OR UPGRADED. THE MANUAL SHOULD ALSO BE ANNUALLY AND MODIFIED AS NECESSARY.



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ASSET MANAGEMENT PLAN

WATER SYSTEM CITY OF T OR C

The technical material and data contained in this document were prepared under the supervision and direction of the undersigned, whose seal as a professional engineer licensed to practice in the state of New Mexico, is affixed below.



Allena Fernandez, PE



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Q:\SEC---PROJECTS\116102 Tor C - Water System Improvements, Phase 1\ENGINEERING_CIVIL\Reports\2017 AMP Update\T or C Water System AMP 2017.docx





March 15, 2017

Juan Fuentes

City Manager

505 Sims St.

City of T or C, New Mexico 87901

Re:

City of Truth or Consequences Water System Asset Management Plan

Dear Mr. Fuentes:

The Water System Asset Management Plan is a living document and requires periodic updating. This update includes reviewing the plan issued in 2015 for the following items:

1. Required metering of all diversions and users,

A section indicating that the City's current municipal code requires all water usage (except for fire suppression usage) to be metered has been included. A copy of ordinance is included in Appendix D.

2. Asset inventory, noting current condition, mapped locations, anticipated useful life and value;

All asset inventories, conditions, mapping, and useful life and value have been updated. The asset tables in the plan and the appendices have been updated.

3. Defined level of service required to meet state and federal regulations, customer demands, and long-term goals as applicable;

The level of service for each asset category was reviewed and found meet current state and federal requirements. It also is meets current customer demands and the City's long term goals. There has been no change in the plan regarding this item.

 Risk analysis of asset performance based on likelihood of failure and level of consequence, as applicable;

The Probability analysis, the Consequence analysis and the Risk calculations have been updated.

5. Operations and maintenance strategic plan and a Capital Improvement Plan that addresses all costs for managing the asset, project or system over time; and



The CIP has been updated to include costs for managing the assets for the next 10 years. TABLE 25A and B have been updated which provides a financial management plan for addressing the costs related to 10 year CIP.

6. Funding strategy for inclusion in Financial Plan.

Funding for the CIP includes loans, grants and increasing user rates. Two options have been reviewed and presented in this plan. These options include:

- a. an increase in water user rates paired with a 100% loans and,
- b. an increase in water user rates paired with a 50% loans and a 50% grants scenario.

If you have any questions regarding the data presented in this AMP update, please feel free to contact me.

Sincerely,

Smith Engineering Company

Allena Fernandez, PE, VP

Engineer



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BACKGROUND AND SCOPE

The City of Truth or Consequences (the City) hired Smith Engineering Company (Smith) to prepare an Asset Management Plan (AMP) for their Municipal Potable Water System (Water System). The AMP addresses five fundamental questions regarding the management of the City Water System:

- What is the current state of the Water System's assets?
- What is the required level of service?
- Which assets are critical for performance?
- What are the best Capital Improvement Project (CIP) and Operation and maintenance (O&M) Strategies (Life-cycle Costs) for the Water System?
- What is the City's best long-term financing strategy for the Water System?

Determining the answers to these questions allows the City to make more informed planning decisions for the Water System and, in the long term, save money and provide a higher level of service.

The AMP addresses the issues by creating an inventory of the water system assets, with corresponding Geographic Information System (GIS) model, and developing a ranking system to prioritize the rehabilitation and replacement needs for the entire Water System. The ranking system is mathematically calculated based on condition and risk assessments of the assets.

The report describes how Smith and the City developed the AMP using asset management principles that include gathering data, preparing an asset inventory, performing condition and risk assessments on the assets, and preparing a 10-year CIP for the Municipal Potable Water System. Lastly, the report gives the City recommendations that can improve the water system asset management.

Asset management is growing in popularity as a tool for utilities to maximize capital value as well as O&M expenses. The process Smith developed for the AMP of the City Water System is straightforward and can (and should) be modified, enhanced, and expanded as additional or improved data become available.

REQUIRED METERING OF ALL DIVERSIONS AND USERS

The City's current municipal code requires all water consumption to be metered except for fire suppression. Appendix D has a letter indicating the water metering requirements and the current 2017 water rates.



CURRENT REHABILITATION AND REPLACEMENT PROGRAM FOR THE WATER SYSTEM

There is currently very limited long term planning or replacement scheduling in regards to the existing components of the municipal potable Water System. Instead, the City bases the current Water System replacement and rehabilitation process on prioritization that comes from reacting to portions or components of the system that are in a state of failure.

At present, maintenance and replacement of the distribution system waterlines of the Water System is on an immediate-need basis, due to breaks or problems with pressure and/or flow. The current City practice is to replace or repair waterlines in the same general alignment as the existing pipe. In order to install new pipe or repair existing pipe located in paved streets, in most cases, the existing pavement is removed and then replaced with new pavement at the same thickness. At the time of waterline pipe replacement, the contractor or City Staff will typically replace all valves and service lines (to the meters) associated with the pipe, and replace and/or relocate meter boxes if needed. As of now, the City has not used the relatively new technology of trenchless waterline rehabilitation.

As is the case with the distribution waterlines, current repair and replacement of Water System equipment components such as wells, pumps, pump motors, check valves associated with pumps, flow meters, electrical and control systems, and pump system housing structures is on an immediate-need basis. For example, a pump motor is typically sent out for repairs after it has failed in the field and the associated pump will be inoperable until the motor repair is complete. In rare circumstances, replacement parts are available in storage at the time a component fails. However, in general, if a component of the Water System equipment fails, the equipment will be out of service until repair or replacement is completed.



WATER SYSTEM ASSET MANAGEMENT PLAN

To be an effective management and planning tool for the City Water System, the AMP must provide a clear understanding of the entire system based on a detailed analysis of all the individual components (assets) in the system. The primary intent of the AMP is to provide effective prioritization of replacement or rehabilitation of system assets, which requires the determination of the existing condition, functionality, and criticality of each asset. Smith, with the assistance of City staff, developed the Water System AMP using the following three steps:

- The first step in developing the AMP was the creation of an inventory of the Water System assets. Smith created the inventory of the City Water System assets in the form of a Geographical Information System (GIS) model. The Water System GIS Model contains digital files that record all the information collected on the assets of the system.
- 2. The second step in developing the AMP was assessing the condition and risk of all the inventoried Water System assets to prioritize replacement or rehabilitation of the assets. Smith performed both condition assessments and risk assessments on each asset, resulting in the assignment of a numerical value, or Risk Score, to each asset. Smith used the Risk Score, or rank, assigned to each asset to determine the prioritization of replacement/rehabilitation for the assets.
- 3. The last step in developing the AMP was scheduling the replacement or rehabilitation of Water System assets based on the Risk Score ranking assigned to them (the higher the score, the higher the priority). Smith used the replacement/rehabilitation prioritization of the assets to produce a 10-year CIP for the Water System.

The following sections of this report discuss the three steps of the AMP in detail for all the Water System assets. To provide clarity and organization of asset details, the Inventory of Assets and Condition and Risk Assessments of Assets sections of this report are subdivided by Water System asset category (Waterlines, Wells, Storage Tanks, etc.). In some cases, the descriptions of condition scoring and risk scoring for certain asset categories may be somewhat redundant as the intent was to have the descriptions of each asset category stand independently and for consistency.

This AMP does not include evaluation of any of the water system owned by the Village of Williamsburg. The City of Truth of Consequences operates and maintains the water system of the Village of Williamsburg, but does not own any of those water system components. Therefore, the components of the water system serving the Village of



Williamsburg are not assets of the City of Truth of Consequences and, as such, were not evaluated or included in the asset inventory in this report.

INVENTORY OF ASSETS

DATA COLLECTION

Smith collected data for the Water System AMP from various information sources in the possession of the City with the assistance of City staff. The City provided a large portion of the data regarding water distribution pipes, valves, and hydrants to Smith in digital format as AutoDesk AutoCAD and ESRI ArcGIS files. Smith did not evaluate the accuracy of the data already provided in digital format and digital information on some of the Water System components, particularly the distribution system, is currently limited.

The data used in the AMP for the remainder of the Water System components (pumps, controls, electrical systems, wells, buildings, etc.) were collected by reviewing engineering plan sets, as-builts, past planning reports, and operation and maintenance manuals provided by the City and by performing field visits. All of the data collected to develop the AMP report was also used to create a companion Water System GIS Model.

ESRI Geodula base WATER SYSTEM GIS MODEL

The Water System GIS Model consists of Shapefiles, a vector data storage format, each → file representing an individual feature (component) of the system, such as a waterline pipe, tank, or well. Associated with the Shapefiles are database files that store the geographic locations and attributes of the Water System features that are represented by the Shapefiles as point, polylines, or polygons. Attributes related to each feature, such as pipe diameter or material, are displayed in the GIS Model as Attribute Tables. The Shapefiles of pipes, hydrants, and valves in the Water System GIS Model are based on existing Shapefiles provided by the Citylthat were updated by comparing them to existing CAD files, old As-builts, the mapping used by the water utility, and conversations/interviews with City staff. Shapefiles for the Components of the Water System (i.e. wells, pumps systems, electrical systems, storage tanks, and buildings) were created using the previously mentioned information sources and added to the GIS Model.

General information regarding the City is also included in the GIS Model. Shapefiles of interstates and streets and locations of schools and hospitals are examples. Incorporating these data into the GIS Model is useful for giving a sense of location as well as determining consequences associated with the failure of a given Water System component.

The Water System GIS Model is an interactive digital inventory of the assets that the City can modify and update using ArcMap software and is the first of its kind for the City. Smith recommends that the City update their current GIS software to ArcMap 10.3 so as

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the field with a mobile application

to be able to use the GIS Model, which was created using ArcMap 10.3, as effectively and easily as possible.

The operators/staff of the Water System will be able to keep all the mapping and asset information of the GIS Model current and accurate by frequently using the GIS software (on whichever computer it is installed) to access and edit the model. Once an asset of the Water System has been modified (i.e. replaced or rehabilitated), or if information on the asset has inaccuracies that need to be corrected, the operators of the system can update the GIS Model accordingly by using the ArcMap 10.3 software. The Attribute Table associated with the asset has information fields that can be edited to input the new information on the asset. If necessary, the Shapefile that geographically represents the asset in the GIS Model can also be edited and updated using the ArcMap 10.3 software. Hardcopies of the tables and maps showing the updates can be printed out from the model, if desired. Some step-by-step instructions for editing the Water System GIS Model are provided with the CD that contains the model. In addition, comprehensive guidance for using the ArcMap 10.3 software is in the user manual that is provide as part of the purchase of the software.

Development of the Water System asset inventory will continue to be an on-going process for the City beyond completion of the initial AMP and GIS Model. The Water System GIS Model is intended to be dynamic tool for the City and should be updated frequently with new data on the Water System assets to prioritize system management as accurately as possible.

Water System Assets Categories

The Water System Inventory of Assets contained in this report is divided up into the following five categories:

- 1. Distribution System (Waterline Pipes, Valves and Hydrants)
- 2. Wells
- 3. Pump Systems and Disinfection System
- 4. Buildings and Structures
- 5. Storage Tanks

The five categories of assets are discussed in detail on the following pages of the report. Information for all the assets in the five asset categories, including the estimated current values and replacement values for the assets, are contained in **APPENDIX A** and **APPENDIX B**. The estimates of the current value of the assets were based on remaining useful life of the assets. The estimates of the replacement value of the assets were based on the costs of replacing the assets using expected, current technology. In addition, the Attribute Tables in the Water System GIS Model for all assets have been modified to contain the estimated replacement values of the assets.



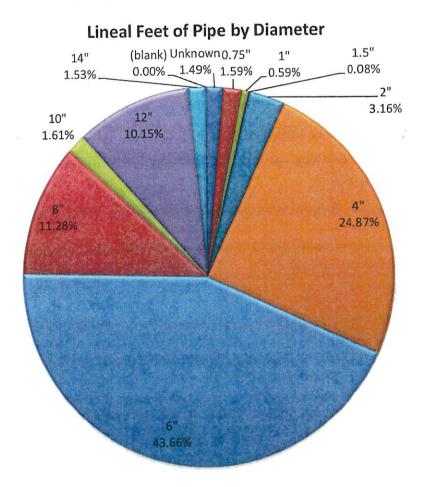
Distribution System (Waterline Pipes, Valves, and Hydrants)

At present the distribution system provides water to a service area greater than five square miles. The Water System GIS Model Inventory database contains 2065 records related to the distribution system network, summarized as follows:

- 1. 1070 records for the Waterline Pipes (total), representing 417,146 Lineal Feet (LF), or 79.0 miles, of distribution pipe.
- 2. 686 records for the Valves, representing 686 known valves.
- 3. 309 records for the Fire Hydrants, represent 309 known hydrants.

Majority of the distribution pipe is PVC (C900 and Schedule 40) (56%), followed by asbestos cement (AC) (29%), and cast iron (11%). Copper, poly, and ductile iron pipe make up the remaining 1% of distribution system. **FIGURE 1** and **FIGURE 2** are charts that show further detail on the size and material of the waterline pipes.

FIGURE 1: Waterline Pipe Sizes of Distribution System





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FIGURE 2: Waterline Pipe Material of Distribution System

Lineal Feet of Pipe by Material

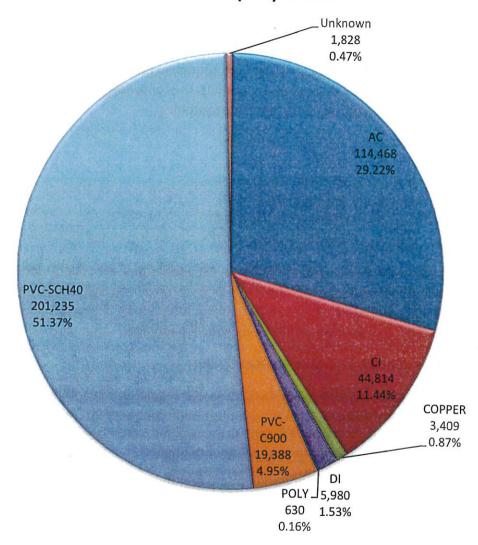


FIGURE 1 and FIGURE 2 reflect the data on the waterline assets compiled in the Water System GIS Model. Waterlines are represented in the GIS Model as Shapefiles made up of polylines and there is a database related to all the waterlines in the GIS Model, displayed in the model as an Attribute Table, which contains information on the characteristics of the pipes (size, material, age, etc).

The distribution system valves and fire hydrants are represented in the GIS Model inventory as Shapefiles that consist of nodes (points) and there are a databases and associated Attribute Tables in the model for the valve and hydrants. Information available on the valves of the distribution system is very limited with no direct



information on the size or age for the vast majority of the valves. For this report, it is assumed that the valve diameter is the same as the diameter pipe to which it is connected and associated with in the GIS Model. There is information on the types and ages of the hydrants on the GIS Model. Hydrant installation dates range from 1962 to 2014, with about one third of the hydrants installed between 1990 and 1999.

Wells

The database of the Water System GIS Model Inventory contains six records for the six well assets. The six records correspond to the following groundwater supply wells currently used to provide water to the system:

- Well No. 1 (SEO No. HS-11-S-2)
- Well No. 2 (SEO No. HS-11)
- Well No. 4 (SEO No. HS-11-S-5)
- Well No. 6 (SEO No. HS-11-S-4)
- Well No. 7 (SEO No. HS-11-S-10)
- Well No. 8 (SEO No. HS-11-S-9)

The GIS Model represents the wells as a Shapefile that consists of nodes (points). There is a database file and an Attribute Table associated with the point Shapefile that represents the wells, which contain information on the characteristics of the wells such as location coordinates, production capacity, and age.

Pump Systems and Disinfection System

The GIS Model database contains 74 records regarding the components that make up the assets of the pump systems and the disinfection system used throughout the Water System. There are essentially nine distinct pump systems currently used in the system. The nine distinct pump systems are the following:

- 1. Cook St. Treatment Facility Pump Station
- 2. Booster Pump Station No. 2 (Morgan Pump Station)
- 3. Cielo Vista Booster Pump Station
- 4. Well No. 1 Pump System
- 5. Well No. 2 Pump System
- 6. Well No. 4 Pump System
- 7. Well No. 6 Pump System
- 8. Well No. 7 Pump System
- 9. Well No. 8 Pump System

The components of the nine pump systems are subdivided into smaller asset categories in the Water System GIS Model. The GIS Model represents the assets of the pump systems as six Shapefiles that consists of nodes (points). There is a database and an Attribute Table associated with each point Shapefile that represent the pump systems' assets, which contain information on the characteristics of the components such as



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location coordinates, equipment type and manufacturer, flow capacity, power usage, and age.

To allow practical asset management, the many components of a given pump system need to be divided up into simple categories that will each be defined as an asset. Based on historical replacement trends reported by operation staff and anticipated life expectancies for equipment, Smith defined pump asset categories for the purposes of this AMP. In the Water System Asset Inventory, each of the following components of a pump system is defined as an individual asset category:

- 1. Pump(s)
- 2. Pump Motor(s)
- 3. Pump Flow Meter(s)
- 4. Pump Manifold (piping, valves, gauges, and misc. components)
- 5. Pump Electrical Systems (power systems, control systems, and generators)
- 6. Supervisory Control And Data acquisition (SCADA) Components (software and Remote Terminal Units [RTUs])

Each one of the above categories has a Shapefile with associated database file and Attribute Table to represent it in the GIS Model.

There are also two horizontal split case pumps, with associated motors, currently in storage at the Wastewater Treatment Plant (WWTP), which are included as pump system assets in the inventory of the Water System. These pumps (and motors) used to be part of the Pershing Pump Station system, which is no longer in use as a pump system. The pumps of Pershing Pump Station were removed from the station in approximately 2012, leaving the piping manifold located at the station behind. The piping manifold, with the addition of a pressure-reducing valve (PRV), was repurposed to serve only as a pressure reducing station. The pressure reducing station is now used to separate the high and low pressure zones of the distribution system, making it a critical part of the Water System.

Buildings and Structures

The Water System GIS database contains 11 records relating the buildings and structures assets associated with the wells, pumps, and other components of the system. The Water System buildings and structures are as follows:

- 1. Cook St. Treatment Facility Pump Station Building
- 2. Cook St. Treatment Facility Chlorination System Shed
- 3. Cook St. Treatment Facility Storage Building
- 4. Booster Pump Station No. 2 (Morgan Pump Station) Building
- 5. Cielo Vista Booster Pump Station Housing
- 6. Well No. 1 Pump System Shade Structure
- 7. Well No. 2 Pump System Building
- 8. Well No. 4 Pump System Building



- 9. Well No. 6 Pump System Building
- 10. Well No. 7 Pump System Building
- 11. Well No. 8 Pump System Building

The GIS Model represents the building/structure assets as a Shapefile that consists of nodes (points). The database file and Attribute Table associated with the point Shapefile contain information on the assets such location coordinates, construction material, dimensions, and age.

Most of the buildings are CMU construction with wood and metal roofing. A few buildings are made of wood frame and stucco. The Chlorination System Shed and Cielo Vista Booster Pump Station Housing are fiberglass structures. The Well No. 1 Pump System Shade Structure is made of wood with a metal siding roof.

Storage Tanks

The database of Water System GIS Model Inventory contains records for five storage tank assets. The five records correspond to the following five storage tanks:

- 1. 0.2 MG Storage Tank, located at Cook St. Treatment Facility
- 2. 1.2 MG Storage Tank, located on Cemetery Rd.
- 3. 3.0 MG Storage Tank, located on Cemetery Rd.
- 4. 3.0 MG Storage Tank, located next to Booster Pump Station No. 2 (Morgan Pump Station)
- 5. 1.0 MG Storage Tank, located on a hill south of the Commission Chambers and Library

The GIS Model represents the storage tanks as a Shapefile that consists of nodes (points). The database file and Attribute Table associated with the storage tank Shapefile contain information on the characteristics of the tanks such as location coordinates, storage capacity, material, and age.

Four of the tanks are currently being used in the Water System. The 0.2 MG tank at the Cook St. Treatment Facility is used in conjunction with gas-chlorination disinfection system and pumps located at the Facility to store and distribute disinfected water to the rest of the Water System. The 1.2 MG and 3.0 MG tanks located on Cemetery Rd. store disinfected water and provide pressure for the northern portion of the distribution system.

The 3.0 MG tank located next to Booster Pump Station No. 2 stores disinfected water and provides pressure for the southern portion of the distribution system.

The remaining 1.0 MG tank inventoried in the GIS Model, located on a hill just south of the Commission Chambers and Library, was recently abandoned and is no longer used part of the water system. However, the tank remains a historical component of the City as it was original constructed in 1948. In addition, it has artistic and cultural value since there is a mural painted on it that can be seen from all around the City. Because the



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tank still has a value to the City it will continue to be considered one the assets of the water system and is part of the Inventory.

CONDITION AND RISK ASSESSMENTS OF ASSETS

The next steps in the AMP are the Condition Assessment of the existing assets of the Water System Inventory and the subsequent Risk Assessment of the assets. The condition of an asset provides a measure of how likely it is to fail, which is a factor used to determine the overall risk associated with the failure of that asset. The ultimate result of the condition and risk assessments of the assets is a ranking system, based on a calculated Risk Score, that will allow the City to prioritize replacement or rehabilitation of the Water System assets in an appropriate, efficient, and cost-effective manner.

Condition assessment of the Water System assets includes the following steps:

- 1. Determining the functionality and associated required level of service for the asset;
- 2. Determining the modes of failures for each type of asset;
- 3. Assigning a life expectancy to the asset;
- 4. Determining additional factors that affect the condition of the asset; and finally
- 5. Assigning a Condition Score to the asset.

The Condition Score serves as a measure of the asset's probability of failure. The condition assessment process is different for each type of asset and, therefore, discussed separately, by asset category, in the following section of the report.

The assessment of overall risk for the Water System assets is a very important element of the AMP. Risk, in this application, is defined by the degree of probability of failure (Condition Score) and a measurement of consequence of failure. For each asset, the risk assessment assigns a value for the consequence of failure, the Consequence Score, based on social, environmental, and economic impacts. Subsequently, the Condition Score is multiplied by the Consequence Score to calculate the Risk Score. As with the condition assessment, risk assessment for each type of asset and is discussed separately, by asset category, in the following section of the report.



Distribution System (Waterline Pipes, Valves, and Hydrants) – Condition and Risk Assessments

Functionality and Level of Service

The Water System distribution network of waterlines, valves, and hydrants provides a means of distributing potable water throughout the City at sufficient system pressure and quantity for customer use and fire protection.

The level of service for the distribution system can be defined by the following three categories:

- 1) customer driven (taste, odor, pressure, minimal water outages),
- 2) regulatory (contaminate levels, pipe sizes and types¹), and
- 3) management (operation and maintenance, including conservation efforts).

Prevention of waterline breaks and proper upkeep of valves and hydrants are the most obvious means to maintain the level of service of water distribution system. When a waterline breaks, service stops for some customers, and the pressure drops for other customers (reduced customer driven level of service). Waterline breaks also create a potential for contaminants to enter the pipe (reduced regulatory level of service). Inoperable valves and/or lack of valves create difficulty for operators to shutdown pipes for connections or repairs (reduction in management level of service). Inadequate flow and pressure due to waterline breaks and inoperable valves and hydrants prevent the system from providing proper fire protection.

Water and energy conservation are important elements of the management category of the level of service for the distribution system. Water and energy conservation can both be addressed at the same time by properly replacing leaking waterlines. Water conservation is addressed by replacing leaking waterlines. However, energy conservation is also addressed by replacing leaking waterline because the total amount of water pumped by the Water System decreases, thereby decreasing the energy used by the pump systems.

Recent evaluation of Water System records indicates that approximately 20% of the amount of water produced by the system is not accounted for by the amount of water sold by the system.² Some of this difference can be attributable to inaccuracies in the metering of the system and discrepancies in accounting, but the vast majority is believed to be the result of many leaking waterlines. Future implementation of water and energy conservation efforts must incorporate replacement of leaking waterlines. Systematic replacement of leaking waterlines in the distribution system will result in significant water and energy savings for the overall Water System. Estimated energy

²40-Year Water Development Plan (WHPacific, 2012).



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 $^{^1\}mathrm{This}$ study did not evaluate pipe capacity (flow or pressure) for the distribution system.

cost savings are discussed in more detail in the Level of Service section of Pump Systems.

A reasonable energy conservation goal is to reduce the total water loss of the system (and associated extra energy use) to approximately 5% of the total production of the system within the next 10 years.

Modes of Failure

A waterline "failure" refers to when a waterline is broken causing property damage and water outage or an interruption in water service. The most frequent failures occur in the Schedule 40 PVC and cast iron piping. A common cause of mainline pipe failure for the City is by corrosion of metal (cast iron, ductile iron) pipes from contact with water. Metal water pipe initially fails at the weld or joint. This is due to small leaks from the pipe corroding the exterior unprotected surface. Corrosion of metal pipes could be mitigated using cathodic protection (techniques used to control corrosion of metal by making it the cathode and connecting the pipe with another more easily corroded metal to act as the anode of an electrochemical cell). However, after pipe installation, cathodic protection is not cost effective.

Waterline pipe failures also result from poor installation or the use improper materials during construction. PVC and AC (asbestos cement) water pipes typically fail from disturbance (road vibration, water hammer, hit during construction, poor bedding, and backfill, etc.). Pipes in several regions of town are known to have inadequate bedding or the wrong schedule (thickness) of PVC (Schedule 40 PVC is not the proper thickness to be used in water system distribution lines – see FIGURE 2). This has resulted in a higher than average number of breaks. Service lines typically fail near the corporation stop (connection to the main line). This is because the connection point is where the service line bends - creating uneven forces and failure.

In the event of a pipe break, water outages can be minimized by creating a looped system and strategically placing valves. Some portions of the distribution system are set-up in this manner. However, in many instances, when a waterline break occurs, the operators cannot close the necessary valves because they have not been "exercised" and are stuck open. When this occurs, the operators must enlarge the shut-off area, which affects more customers and increases the time that the water leak runs. This increases the amount of water lost, and potentially worsens the property damage. Implementation of a valve exercise program, and a valve replacement program could minimize damage and loss from waterline breaks.

Life Expectancy

The life expectancy or "design life" of an asset is the average number of years before replacement is necessary. Multiple factors will either reduce or extend the design life. The life expectancy of any asset can vary based on the quality of the product, quality of



the installation, and operation and maintenance procedures. These factors are difficult to measure after installation. In general, the average expected useful life of a Water System distribution waterline pipe is 75 years, assuming correct installation, etc.

Knowing the design life of the assets, in this case the pipes, is useful in estimating the remaining useful life of an asset and planning appropriately for its replacement. This analysis does not calculate the exact date of required replacement. Instead, the expected useful life is used to formulate a scoring curve as part of the probability of failure calculation. The closer the pipe is to the end of its useful life, the more likely it is to fail. This study applies an age vs. remaining useful life curve as part of the probability of failure assessment. Smith derived scoring formulas for various pipe types based on the appropriate age vs. remaining useful life curve. The scoring formulas are based on the average life of PVC waterline pipe being 50-years, the average life of cast iron pipe being 100-years, and the average life of all other waterline pipe material in the system being 70-years, as shown in FIGURE 4.

It should be noted that there was limited age data available for the distribution system. As-builts showing the year of pipe installation are available for only a few regions of the City. Most of the ages used in this AMP are based on historic trends in waterline pipe material, hydrant ages in the immediate surrounding area and on interviews conducted with operators.

Probability of Failure

The condition of an asset is the major factor in determining the probability of failure. Since most waterline pipes are buried under pavement, visual condition assessment in the field is not possible. Due to the lack of field visit condition assessments for piping, Smith based the probability of waterline failure on pipe age and material, and input from operators during interviews. Smith used pipe age, material, and operator input to assign a value, the Condition Score, to the waterline assets which represents the probability of failure of the assets. Research and discussions with Water System operators revealed factors such as location of previous repairs, pipe age, pipe material, soil compaction, surface traffic, and previous customer complaints (pressure, odor, color, etc.) could increase the probability of waterline failure. However, some of the factors, such as customer complaints and soil compaction, had no usable existing data associated with them, so Smith eliminated them from the analysis. Factors, such as surface traffic, were eliminated from the condition scoring but are considered a consequence of failure.

The best data available that relates to failure probability is the age of the waterline pipe and the pipe material. Smith implemented an age scoring system of 1 to 10 points depending on the amount of pipe life utilized. A score of 10 is associated with a pipe

³ "Dawn of the Replacement Era/Reinvesting in Drinking Water Infrastructure", AWWA, May 2001.



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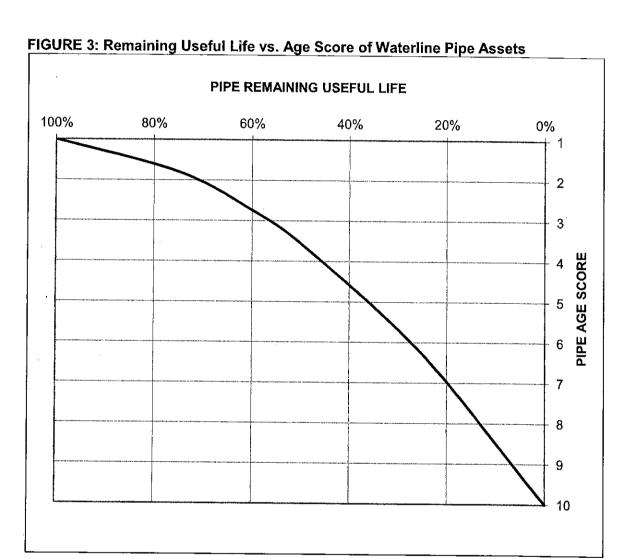
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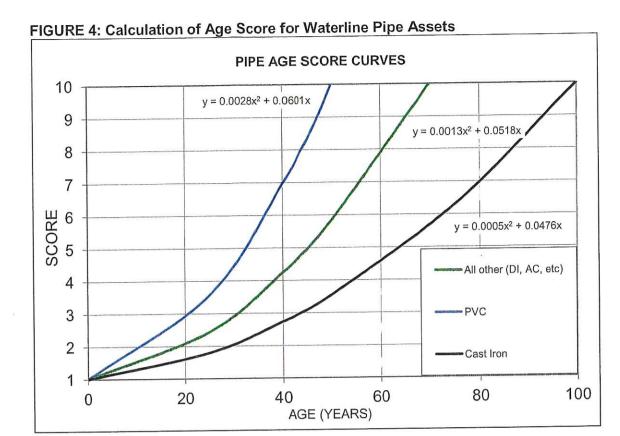
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that has reached the end of it useful life; a score of 1 is associated with a pipe with 90-100% of its useful life remaining (see FIGURE 3). For waterline pipes that have ages beyond the useful life, calculated age scores are greater than 10 based on the use of the appropriate formula for pipe material as shown in FIGURE 4.



Based on industry design standards and the experience of the City staff and others, Smith used the pipe life expectancies previously discussed combined with the approach to scoring shown in FIGURE 3 to produce the Age Score equations shown in FIGURE 4.





There are two components that make up the total Condition Score – the Age Score and the Operator Score. The Age Score value is based on the age curve appropriate for the pipe material (shown in **FIGURE 4**) and the pipe age. For this report, most ages are approximate due to limited documentation. Known ages are based on as-built documents. When as-builts are unavailable, pipe age is approximated based on one or more of the following:

- 1) Associated hydrant age, which are known and documented,
- 2) Known historical use of specific piping material⁴,
- 3) Known date of construction of specific area downtown has evidence of Work Projects Administration (WPA) work dated to approximately 1935; the elementary school was originally constructed in 1975; etc.,
- 4) Interview/discussions with City Staff regarding pipe age.

The Operator Score is additional points assigned to the Condition Score based on interviews with City Staff. Regions identified by operators that experience a high number of breaks and require frequent maintenance receive an Operator Score of 10. Pipes in these areas are predominantly Schedule 40 PVC and cast iron so there is also a likely correlation to pipe material and construction. Corrosive soil might be to blame for the decrease in useful life of cast iron; while insufficient pressure capacity (due to lack of

^{4&}quot;Buried No Longer: Confronting America's Water Infrastructure Challenge". AWWA



sufficient pipe thickness) and inadequate bedding are likely responsible for the decrease in useful life of Schedule 40 PVC pipe. Thus, all Schedule 40 PVC and all cast iron pipes of the entire distribution system were assigned an Operator Score of 5. TABLE 1 illustrates the condition scoring approach applied to the waterline pipes.

TABLE 1: Condition Assessment and Scoring of Waterline Pipe Assets

	Condition Score Pipe Material All Other (DI, AC, Cast Iron								
Criteria Pipe Age		etc)	Cascilon						
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Age = 100yrs	34.5	18.6	10.0						
Age = 75yrs	20.3	11.2	6.3						
Age = 50yrs	10.0	5.8	3.6						
Age = 40yrs	7.0	4.2	2.7						
Age = 25yrs	3.6	2.4	1.8						
Age = Oyrs	1.0	1.0	1.0						
Operator Interviews			1 200 3 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1						
Specific Problem Areas*	10	10	10						
Material Related Problems**	5		5						

Age Score component of Condition Score is calculated from decay curves presented in Figure 4.

Consequence of Failure

The probability of failure for a water pipe is calculated based on historical industry trends and the City's operators experience but overall, it is not a predictive calculation. However, formulating the consequences of a pipe failure is much more exact. For instance, we cannot predict exactly when a waterline pipe in a main arterial street will fail, but we are certain that when it does, the consequences (repair cost, disruption of traffic, etc.) will be greater than if it were a waterline in a residential street. In addition, we are certain that the cost of repair of the failure for this line is greater than the cost of replacement prior to failure.

Consequence scoring is based on a methodology, which includes social, environmental, and economic consequences of failure criteria. Items included in the scoring criteria were those for which GIS data was accessible and those that clearly had social, environmental, and/or economic impact as a result of failure. Examples of social consequence would be failures that affect high traffic areas or have an impact on public safety like those in a contaminated area or near schools or hospitals. Failures near



^{*} Condition Score calculated by adding 10 points to Age Score for waterline pipes located in specific areas identified by operators as having a high number of maintenance issues.

^{**} Condition Score calculated by adding 5 points to Age Score for all Schedule 40 PVC and Cast Iron waterline pipes based on operator input related to high number of maintenance issues.

populated areas could affect more customers as well as give the City a poor public image. Failures in a waterway could have an environmental impact. Economically, failures in higher traffic areas, more populated areas, and in environmentally sensitive areas all have increased repair and replacement costs.

TABLE 2 contains the different consequences of failure related the waterlines, associated Consequence Scores, and the factors that were considered in determining the values of the scores. A Consequence Score of 1 is considered the lowest consequence and a score of 5 is the highest. There is a consequence to any pipe failure; therefore, all pipe segments have a minimum consequence score of 1. The consequence of a water main pipe failure, for example, is given a higher score of 5 because of the impact on the overall system and the potential high cost of repair.

TABLE 2: Consequence of Failure Scoring of Waterline Pipe Assets

CONSEQUENCE		SOCIAL			VIRON	IMENT	AL	ECON	SCORE	
Criteria	Disruption of Service/ Access	Safety Impacts, internal and external	Public Image	Aesthetics	Permit Violation	Ecosystem	Sustamability	Level of Service, Reliability	High Repair/Restore Cost	
ALL PIPE	State Con		. 444					ELASION .		1
Water Main or Well Line							X		E 79/4 E	5
In Interstate										5
High Traffic Corridor (>1000 Average Daily Trips)										4
Moderate Traffic Corridor (<1000 Average Daily Trips)									10.2	3
Within 100 Ft. of Hospital		\$4.77 21.22								3
Crossing Waterway (River, Ditch, Arroyo, Wetland)			\$2.1-04.			Х				2
Within 100 ft. of Schools							ļ			2
Within 100 ft. of Business Center/ Commercial Zone			51651713 11651713							1



Risk of Failure

For a distribution waterline pipe asset, the overall risk of failure, or Risk Score, is the product of the Condition Score (probability of failure) and the Consequence Score (consequence of failure).

RISK = CONDITION SCORE X CONSEQUENCE SCORE

For example, if a pipe asset has a high Condition Score (10) and low Consequence Score (1), the Risk Score is 10 (10 \times 1), which is considered a medium level of risk. If another pipe asset has a medium Condition Score (6), but a high Consequence Score (5), the Risk Score is 30 which is considered a high level of risk.

Risk Scores were calculated and assigned to all the waterline assets of the distribution system and are contained in APPENDIX A. Valve and hydrant assets associated with a given waterline asset are assigned the same Risk Score as the waterline pipe and are assumed to be replaced at the same time as the pipe. The Risk Scores for the waterline assets of the distribution system ranged from 1 to 168. The higher the Risk Score for an asset, the higher the risk of failure and, ultimately, the higher the priority of repair or replacement. Based on the Risk Scores assigned to them, the waterline assets have been categorized into one of four Risk Levels, as follows:

Low Risk Level — Risk Score range from 1 to ≤8

Medium Risk Level — Risk Score range from >8 to ≤12

High Risk Level — Risk Score range from >12 to ≤56

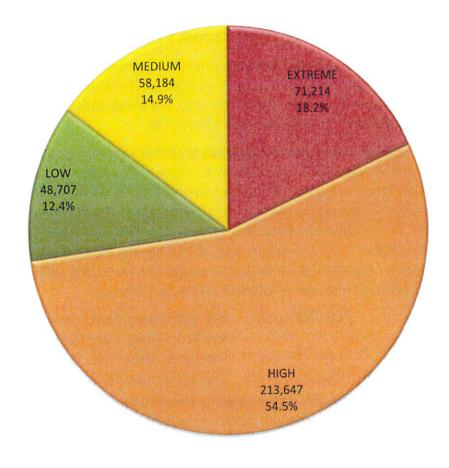
Extreme Risk Level — Risk Score range from >56 to ≤168

Assets in the Extreme Risk Level category are considered beyond an acceptable level of risk. Essentially, the City should replace or rehabilitate any waterline assets assigned to the Extreme Risk Level. As can be seen in the pie chart in FIGURE 5, 18.2% of the current system is considered "Extreme Risk". The map in FIGURE 6 geographically shows the entire distribution system and the Risk Level associated with each waterline pipe asset. Asset risk levels, priority of replacement, and scheduling of replacement are discussed in detail in the Replacement Schedule section of the report.

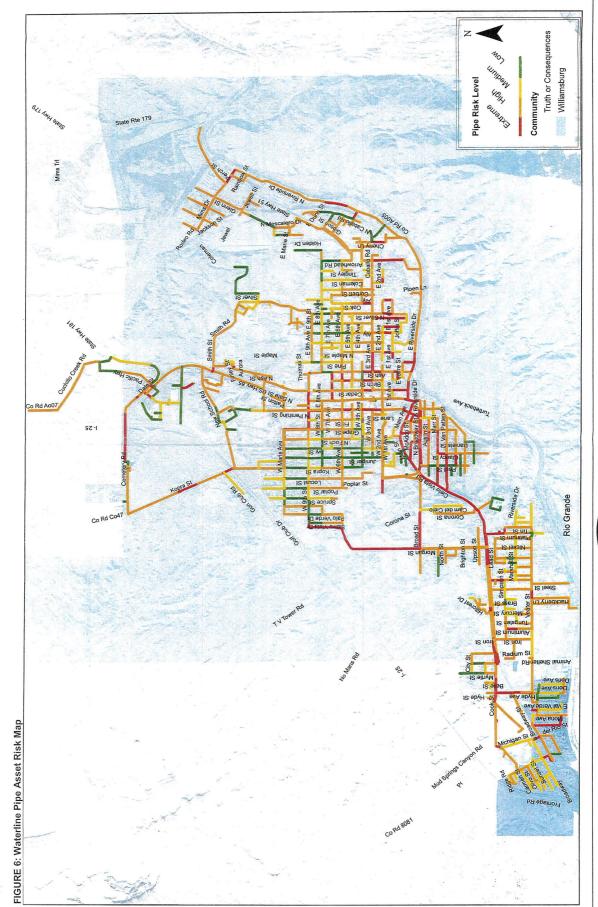


FIGURE 5: Waterline Pipe Asset Risk 2017

Current Waterline Risk Level by Lineal Feet









Wells - Condition and Risk Assessments

Functionality and Level of Service

The groundwater wells are critical components that are the foundation of the Water System since they provide the water supply to the system. The level of service for water supply wells can be defined by the following three categories:

- 1) customer driven (production level, minimal water outages),
- 2) regulatory (contaminate levels, sustainability), and
- 3) management (operation and maintenance, including conservation efforts).

In the event a water supply well fails and cannot produce water for an extended amount of time, water supply demands increase on the other wells in the system (reduced management level of service). If multiple wells fail at the same time, the pressure and flow of the distribution system may be affected and service may be limited or even stopped for some customers, (reduced customer driven level of service). If the quality of the groundwater being drawn from a particular well becomes unsuitable, the well may need to be removed from service (reduced regulatory and management level of service).

Water and energy conservation are important elements of the management category of the level of service for the wells. As discussed in the Level of Service Section for the distribution system, water and energy conservation can both be addressed at the same time by properly replacing leaking waterlines. The majority of the water and energy conservation efforts associated with the wells will be addressed by replacing the leaking waterlines in the distribution system since the wells provide the water supply. Future implementation of the replacement of leaking waterlines will make the wells more sustainable as there will be less demand on them and pump systems associated with the wells will use less energy correspondingly. Estimated energy cost savings are discussed in more detail in the Level of Service section of Pump Systems.

Modes of Failure

A well "failure" refers to one of the following three scenarios:

- 1) a portion of the well breaks causing the well to stop producing water or,
- 2) the well has exhausted the supply of groundwater that it draws from and has "gone dry" or,
- 3) the quality of the groundwater in the well has diminished due to contamination and can no longer be reasonably treated to provide potable water.

Life Expectancy

The life expectancy of a well asset is the average number of years before replacement or significant rehabilitation is necessary. The life expectancy of a well is dependent on its construction and the sustainability of the supply of water it draws upon. In general, the



typical life expectancy of a well asset used in this AMP is 25 years⁵. Smith estimated the remaining useful life for each of the well assets based on age information collected from the City. The estimated remaining useful life for each well asset is shown in TABLE 3.

Probability of Failure

The condition assessment of a well asset is very important for determining the probability of failure. As with waterline piping, visual condition assessment of a well is not possible because it is below the ground. Smith interviewed water system operators on the condition of the wells. According to operating staff, all wells are producing well and show no signs of operational problems. Since the operator comments did not indicate problems with the wells and visual inspection of the wells is not possible, the only factor used to assess condition was well age. All documented information on the well assets is contained in APPENDIX B.

Smith implemented a remaining useful life scoring system for the well assets based on a scale of 1 to 10 points. A score of 10 is associated with a well asset that has no remaining useful life; a score of 1 is associated with a well asset that has 90-100% of its useful life remaining. For assets older than their life expectancies, a scoring adjustment was applied by using a linear extrapolation of the typical life expectancy of 25 years. For example, a well that is 50 years old (200% of life expectancy) receives a score of 20 (2.0 x 10). Any additional points calculated as fractions were rounded to the nearest integer (whole number). TABLE 3 contains the Condition Scores for all the well assets, based on the calculated useful life scores.

TABLE 3: Condition Assessment and Scoring of Well Assets

Asset	Life Expectancy (Years)	Age (Years) ⁶	Estimated Remaining Useful Life (Years)	Useful Life Score	Condition Score
Well No. 2	25	72	0 ·	29	29
Well No. 4	25	59	0	24	24
Well No. 6	25	41	0	16	16
Well No. 1	25	27	0	11	11
Well No. 8	25	18	7	7	7
Well No. 7	25	2	23	1	1

Consequence of Failure

As is the case with waterline assets, the probability of failure (Condition Score) for a well asset is calculated based on an estimated life expectancy and, therefore, cannot be as

⁶ In 2017



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⁵ Typical expected life of a well asset defined as 25 years based on expected life range in "Asset Management: A Handbook for Small Water Systems", United States Environmental Protection Agency, 2003.

precisely determined as the consequence of failure. All the wells are critical because they are the source of the water on which the rest of the system is based. However, if one well fails, the other wells can be pumped more to make up the difference in production temporarily. Therefore, the consequence of failure for each well is relatively high, with the individual consequence of failure score for each well defined by the production capacity of the well. As shown in TABLE 4, the higher the capacity of a well results in a higher the Consequence Score.

TABLE 4: Consequence of Failure Scoring of Well Assets

CONSEQUENCE	50514			F 5 0 7 11					SCORE
	SOCIA			ENVI	RONM	NTAL		ECONOMIC	SCORE
Criteria	Disruption of Service/ Access	Safety Impacts, Internal & External	ublic Imago	Aesthetics	Permit Violation	cosystem	sustamability	evel Of Service, Reliability	
Failure of Well No. 7									:
(*Production Capacity = 900 GPM)							X		6
Failure of Well No. 6				_			.,		
(*Production Capacity = 600 GPM)							X		5
Failure of Well No. 8									_
(*Production Capacity = 640 GPM)							Х		5
Failure of Well No. 1							,,		_
(*Production Capacity = 375 GPM)							X		4
Failure of Well No. 2							.,		
(*Production Capacity = 280 GPM)				,			X		3
Failure of Well No. 4 (*Production Capacity = 240 GPM) *Production Capacity of each wall is beard.							Х		3

^{*}Production Capacity of each well is based on information provided in the 2007 NMED Drinking Water Bureau Sanitary Survey for the Truth or Consequence Water System

Risk of Failure

The risk of failure for a well asset, the Risk Score, is the product of the probability of failure (Condition Score) and the consequence of failure (Consequence Score). The Risk Score prioritizes, or ranks, the assets to appropriately plan and schedule replacement or rehabilitation as discussed in the Replacement Schedule Section of this report. Risk Scores for each of the well assets are shown in TABLE 5.



TABLE 5: Risk Scoring of Well Assets

Asset	Condition Score	Consequence Score	Risk Score		
Well No. 2	28	. 3	87		
Well No. 6	15	5	80		
Vell No. 4 22		3	72		
Well No. 1	10	4	44		
Well No. 8	6	5	35		
Well No. 7	1	6	6		

Pump Systems and Disinfection System - Condition and Risk Assessments

Functionality and Level of Service

The pumps systems are critical components of the Water System that provide the means to draw groundwater from the supply wells transfer it to the disinfection system and storage, and ultimately provide potable water to the end users of the system though the distribution network of waterlines. To provide the proper level of service, the pump systems need to work reliably and must provide the appropriate flow and pressure to meet demand requirements of the overall system. The level of service of the pump systems can be defined as the following category: management (operation and maintenance, including conservation efforts).

The gas-chlorination disinfection system is an extremely critical component of the overall Water System since it provides the means to disinfect the source groundwater so end users can safely consume it as potable water. To provide proper level of service, the gas-chlorination system must have capacity and reliability to properly disinfect the quantity of water required to meet the demand of all the end users of the system. The level of service of for the disinfection systems can be defined by the following two categories:

- 1) customer driven (taste, odor) and
- 2) regulatory (contaminant/pathogen levels).

Water and energy conservation are important elements of the management category of the level of service for the pump systems. Energy conservation in regards to the pump systems is particularly important as the pump systems consume most of the energy used by the overall Water System. As discussed in the Level of Service Section for the distribution system, water and energy conservation can both be addressed at the same time by properly replacing leaking waterlines. Majority of the water and energy conservation efforts associated with the pump systems will be addressed by replacing the leaking waterlines in the distribution system since all water used by the distribution system must be delivered by the pump systems. Future implementation of the replacement of leaking waterlines will decrease water demand resulting in less wear and tear on the pump systems and less energy use by the pump systems. Recent estimates of



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water loss in the Water System indicated it to be as high as 20% of the total water production of the system. Based on an estimated average daily water production of 1.2 MG per day, daily and annual energy use estimates for the Water System pump systems were calculated, as well as estimated energy costs, and are contained in **APPENDIX C**. At present, the existing Water System is estimated to use approximately 1,082,000 kW-Hr of energy annually at a cost of approximately \$117,000. If future waterline replacements in the system over the next ten years could reduce the water loss of the system to only 5%, the resulting annual energy savings would be approximately 162,300 kW-Hr, equating to approximately \$17,500 in annual cost savings.

There can be additional energy conservation by managing the pump systems to ensure they are used as efficiently as possible. This usage rate is constant throughout the day, so there are no cost savings related to running Water System equipment at off-load times of day. However, the pump systems can be managed to run as efficiently as possible. This can be achieved by instructing operators to frequently check the manufacturer-provided information on the pumps (pump curves showing duty points) to ensure pumps are operating at the design flows and pressures. Often, a pump can achieve the performance desired by the system by not operating at an efficient point on the pump curve. Slight changes to pump operational conditions to use it in a more energy-efficient manner will save energy (and money) in the long run. In some cases, the existing pump may need to be replaced with or supported by another pump with characteristics more suitable for the application.

A reasonable energy conservation goal is to reduce the total water loss of the system (and associated extra energy use) to approximately 5% of the total production of the system within the next 10 years.

Modes of Failure

A pump system "failure" refers to a scenario in which the pumps can no longer supply water to the intended location at the necessary flow or pressure for appropriate use. Pump systems are typically very complicated and have many individual critical components; each of which can fail, resulting in the failure of the overall pump system. Pump system failures can be either mechanical (i.e. pump impellor, valves associated with pump manifold) or electrical (i.e. power system, pump control system) in nature. The pump system component that most commonly fails is the electric pump motor.

A chlorination disinfection system "failure" refers to a scenario in which the system can no longer provide the amount of chlorine necessary to properly disinfect the amount of water passing though it and meet regulatory standards. A gas-chlorination system can fail due to pump, control, electrical malfunctions, and/or depletion of the gas chlorine source.

⁷⁴⁰⁻Year Water Development Plan (WHPacific, 2012).



Life Expectancy

The life expectancy of pump system or disinfection system equipment is the average number of years before replacement of the equipment or significant rehabilitation is necessary. The life expectancies of critical pump system equipment components and disinfection equipment vary and therefore, need to be defined separately. The critical pump system equipment and chlorination disinfection equipment estimated life expectancies used in this AMP are as follows⁸:

1}	Pumps (impellors, bearings, bowls, etc.):	15 years
2)	• • •	8 years
3)	Valves (check valves, isolation valves):	35 years
•	Flow Meters:	15 years
5)	Electrical Systems:	15 years
6)	SCADA Systems (HMIs, RTUs)	15 years
7)	Chlorination Disinfection System	15 years

The estimated life expectancies listed above consider variables such quality of the product, quality of the installation, and operation and maintenance procedures.

Smith estimated the remaining useful life for each of the Water System pump system assets based on age information collected from the City. TABLES 6-10 show the estimated remaining useful life for each of the pump system assets.

Probability of Failure

As with the other Water System assets, the condition of a pump system or disinfection system asset is the major factor in determining the probability of failure. The primary measurement of condition is the estimated remaining useful life of the asset, based on age. Other aspects of the condition assessment of the pump system components and the disinfection system were visual inspection during field visits and input from the operators. Smith performed visual condition assessment on accessible pump system assets during multiple field investigations and documented the overall condition of the pump systems and any damage due to age or wear from operation. Some of the pump system equipment is underground and could not be field assessed. In cases where the equipment could not be field visited, estimated useful life and operator input were the only factors taken into account for condition assessment. All documented data on the pump system and disinfection system assets are contained in APPENDIX B.

Smith implemented a remaining useful life scoring system for the pump system components based on a scale of 1 to 10 points. A score of 10 is associated with an asset that has no remaining useful life; a score of 1 is associated with an asset with 90-100% of its useful life remaining. For assets older than their life expectancies, a scoring adjustment was applied based on a linear relationship extrapolated from the defined

⁸ Typical life expectancies of pump system and chlorination equipment based on expected life ranges in "Asset Management: A Handbook for Small Water Systems", United States Environmental Protection Agency, 2003.



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estimated life expectancies of each pump system component. For example, a pump motor that is 16 years old (200% of life expectancy) receives a score of 20 (2.0×10).

Smith also implemented a scoring system, to augment the useful life score, which considers damage observed during field visits and recommendations of operators. When significant damage to a pump system asset was observed during a field visit, 1-10 points were added to the useful life score of the asset, depending on the severity of the damage. Similarly, when a member of the operational staff made a recommendation regarding replacement or rehabilitation of a pump system asset, 1-10 points were added to the useful life score of the asset, depending on the urgency of the recommendation. If the results of the field inspection and operator input were positive, 0 points were added.

TABLES 6-10 contain the Condition Scores for all the pump system component assets and the disinfection system asset, separated by asset category, as determined from the condition assessment scoring system



TABLE 6: Condition Assessment and Scoring of Pump System Assets-

Pumps and Motors

Pumps and Motors Asset	Life Expectancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Field Visit Assessment Score	Operator Interview Score	Condition Score
Well No. 2 Pump	15	72	0	48	6	4	58
Well No. 2 Pump Motor	8	32	0	40	4	3	47
Cook St. Treatment Facility Pump No. 2 Motor	8	21	0	26	3	2	31
Well No. 8 Pump	1 5	18	0	12	3	10	25
Well No. 7 Pump Motor	8	18	0	23	3	1	27
Well No. 6 Pump Motor	8	18	0	23	2	1	26
Well No. 8 Pump Motor	8	18	0	23	2	1	26
Well No. 1 Pump Motor	8	16	0	20	0	1	21
Well No. 4 Pump Motor	8	16	0	20	0	1	21
Cielo Vista Pump Station Pump No. 2 Motor	8	11	0	14	5	2	21
Cook St. Treatment Facility Pump No. 1	15	21	0	14	3	1	18
Cook St. Treatment Facility Pump No. 2	15	21	0	14	.3	1	18
Cielo Vista Pump Station Pump No. 1 Motor	8	11	0	14	2	2	18
Well No. 7 Pump	15	18	0	12	3	1	16
Well No. 6 Pump	15	18	0	12	1	1	14
Booster Pump Station No. 2 Pump No. 1 Motor	8	10	0	13	2	1	16
Well No. 1 Pump	15	16	O	11	0	1	12
Well No. 4 Pump	15	16	0	11	0	1	12
Cieło Vista Pump Station Pump No. 1	1 5	11	4	7	2	2	11
Cielo Vista Pump Station Pump No. 2	15	11	4	7	2	2	11
Booster Pump Station No. 2 Pump No. 1	15	10	5	7	2	1	10
Booster Pump Station No. 2 Pump No. 2	15	10	5	7	2	1	10
Booster Pump Station No. 2 Pump No. 2 Motor	8	4	4	5	0	5	10
Cook St. Treatment Facility Pump No. 1 Motor	8	3	5	4	3	1	8



It should be noted that two pumps (and their motors) that were previously part of the Pershing Pump Station have been removed from service and are now in storage at the WWTP facility. Condition and risk assessments were not performed on these two sets of pumps and motors due to them not being in service. However, they are accounted for in the GIS Model inventory and have documented information contained in APPENDIX B.

TABLE 7: Condition Assessment & Scoring of Pump System Assets – Manifolds & Flow Meters

Flow Meters												
Asset	Life Expectancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Field Visit Assessment Score	Operator Interview Score	Condition Score					
Pershing PRV Station Manifold	35	72	0	21	2	1	24					
Cook St. Treatment Facility Flow Meter	12	21	0	18	4	4	26					
Well No. 2 Pump Manifold*	35	38	0	11	7	4	22					
Well No. 1 Pump Manifold	35	26	9	7	8	5	20					
Well No. 4 Pump Manifold	35	38	0	11	6	4	21					
Well No. 2 Flow Meter	12	17	0	14	2	1	17					
Well No. 7 Flow Meter	12	18	0	15	1	0	16					
Well No. 8 Flow Meter	12	18	0	15	1	0	16					
Well No. 4 Flow Meter	12	13	. 0	11	3	1	15					
Well No. 6 Flow Meter	12.	18	0	15	0 .	0	15					
Cook St. Treatment Facility Pump Manifold	35	21	14	6	3	2	11					
Well No. 6 Pump Manifold	35	26	9	7	0	0	7					
Well No. 7 Pump Manifold	35	26	9	7	0	0	7					
Well No. 8 Pump Manifold	35	26	9	7	0	0	7					
Booster Pump Station No. 2 Flow Meter	12	10	2	8	0	0	8					
Cielo Vista Pump Station Manifold	35	10	25	3	3	1	7					
Booster Pump Station No. 2 Pump Manifold	35	10	25	3	2	0	5					
Well No. 1 Flow Meter	12	3	9	3	0	0	3					

^{*}Age used was an average: some components of the manifold are very old and some have been recently replaced.



TABLE 8: Condition Assessment and Scoring of Pump System Assets – Electrical Systems

Systems							
Asset	Life Expectancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Field Visit Assessment Score	Operator Interview Score	Condition Score
Well No. 2 Pump Electrical System	15	42	0	28	4	1	33
Weli No. 4 Pump Electrical System	15	32	0	21	3	1	25
Well No. 1 Pump Electrical System	15	26	0	17	5	3	25
Well No. 6 Pump Electrical System	15	18	0	12	1	0	13
Well No. 7 Pump Electrical System	15	18	0	12	1	0	13
Well No. 8 Pump Electrical System	15	18	0	12	1	0	13
Cielo Vista Pump Station Electrical System	15	11	4	7	1	0	8
Booster Pump Station No. 2 Electrical System	15	10	5	7	0	0	7
Booster Pump Station No. 2 Back-up Generator	15	10	5	7	0	0	7
Cook St. Treatment Facility Electrical System	15	4	11	3	1	0	4

It should be noted that the electrical system previously part of the old Pershing Pump Station (now out of service), which is still housed in the Pershing PRV building, has been abandoned and will ultimately be removed and disposed of. As a result, it is not considered an asset of the Water System and will not be addressed in this AMP.

TABLE 9: Condition Assessment and Scoring of Pump System Assets – SCADA System

Asset	Life Expectancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Field Visit Assessment Score	Operator Interview Score	Condition Score
SCADA System RTUs	15	20	0	13	10	10	33
SCADA System Software	15	16	0	11	10	10	31



TABLE 10: Condition Assessment and Scoring of Disinfection System Asset – Gas-Chlorination System

Asset	Life Expect- ancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Field Visit Assessment Score	Operator Interview Score	Condition Score
Gas-Chlorination System	15	21	O	14	2	2	18

Consequence of Failure

The consequences associated with the failure of pump systems are primarily the disruption in water distribution service resulting from the failure and the cost of repairing the pumping equipment quickly to resume service. All of the pump systems are critical for the distribution of potable water at the proper flow and pressure for customer use and for fire protection. Therefore, any pump system failure that results in loss of pumping output is of high consequence. Failure of a pump system electrical component or a critical manifold component has high consequence scoring because they result in the loss of function of the system. The pump systems for all the wells incorporate only one pump and associated motor so failure of one these components has moderate to high Consequence Score, which would be higher if the wells didn't provide some amount of redundancy to each other. The Cook St. Treatment facility, Booster Pump No.2 and Cielo Vista pump systems have more than one pump and, as a result, have some amount of redundancy built in. Should one pump fail in these systems there is not total loss of functionality which is reflected in the associated consequence scoring. TABLE 11 contains the consequence scores related to the different types of pump system failures with high scores correlated to the criticality of the particular pump systems.

There are potential health consequences associated with the failure of the gaschlorination disinfection system. If properly disinfected water can't be distributed to end users, they may either get sick or go without proper water service, both of which high consequences of failure as shown in TABLE 11.



TABLE 11: Consequence of Failure Scoring of Pump System Assets

CONSEQUENCE	SOCIA	.L		ENVIR	ONME	NTAL	ECON	IOMIC	SCOR
Criteria	Disruption of Service/ Access	Safety Impacts, Internal & External	Public Image	Aesthetics	Permit Violation	Sustamability	Level of Service, Reliability	High Repair/ Restore Cost	
Failure of Chlorination System, resulting in loss of ability to pump chlorinated water to entire distribution system.				х	х				12
Failure of Cook St. Treatment Facility Pump Electrical/Control System or critical manifold component, resulting in loss of ability to pump chlorinated water to entire distribution system.					X				14
Failure of Cook St. Treatment Facility Pump or Motor, resulting in only one pump available to pump chlorinated water to entire distribution system.					x				7
Failure of Booster Pump Station No. 2 (Morgan Pump Station) Pump Electrical/ Control System or critical manifold component, resulting in loss of ability to pump chlorinated water to Upper Zone of the distribution system.					х				10
Failure of Booster Pump Station No. 2 (Morgan Pump Station) Pump or Motor resulting in only one pump available to pump chlorinated water to the Upper Zone of the distribution system.					x				5
Failure of Pump System Pump/Motor/Elec./Control System/critical manifold component) of Well No. 6						х			5
Failure of Pump System Pump/Motor/Elec./Control System/critical manifold component) of Well No. 7						Х			6
Fallure of Pump System Pump/Motor/Elec./Control System/critical manifold component) of Well No. 8						х			5
Failure of Pump System Pump/Motor/Elec./Control System/critical manifold component) of Well No. 1			*			х			4
Fallure of Pump System Pump/Motor/Elec./Control System/critical manifold component) of Well No. 2						х			3
Failure of Pump System Pump/Motor/Elec./Control System/critical manifold component) of Well No. 4		18				х			3
Failure of Cielo Vista Booster Pump System (Pump/Motor/Electrical/Control System/critical manifold component				Х		х			4
Failure of Pershing PRV critical manifold component, resulting in loss of pressure reducing capability between HIgh and Low pressure zones of the distribution system.				Χ					8
Failure of any pump system flow meter		1041			Х		2	7. S.U	1
Failure of SCADA critical component							44		4



Risk of Failure

The risk of failure for a pump system or disinfection system asset is the product of the probability of failure (Condition Score) and the consequence of failure (Consequence Score). The Risk Score prioritizes, or ranks, the assets to appropriately plan and schedule replacement or rehabilitation as discussed in the Replacement Schedule section of this report. The Risk Score for each of the pump system assets and the disinfection system asset are shown in TABLES 12-16.

TABLE 12: Risk Scoring of Pump Systems Assets – Pumps and Motors

Asset	Condition	Consequence	Risk Score	
	Score	Score		
Cook St. Treatment Facility Pump No. 2 Motor	31	7	217	
Well No. 2 Pump	58	3	174	
Well No. 7 Pump Motor	27	6	162	
Well No. 2 Pump Motor	47	3	141	
Well No. 6 Pump Motor	26	5	130	
Well No. 8 Pump Motor	26	5	130	
Cook St. Treatment Facility Pump No. 1	18	7	126	
Cook St. Treatment Facility Pump No. 2	18	7	126	
Well No. 8 Pump*	25	5	125	
Well No. 7 Pump	16	6	96	
Well No. 1 Pump Motor	21	4	84	
Cielo Vista Pump Station Pump No. 2 Motor	21	4	84	
Booster Pump Station No. 2 Pump No. 1 Motor	16	5	80	
Cielo Vista Pump Station Pump No. 1 Motor	18	4	72	
Well No. 6 Pump	14	5	70	
Well No. 4 Pump Motor	21	3	63	
Cook St. Treatment Facility Pump No. 1 Motor	8	7	56	
Booster Pump Station No. 2 Pump No. 1	10	5	50	
Booster Pump Station No. 2 Pump No. 2	10	5	50	
Booster Pump Station No. 2 Pump No. 2 Motor	10	5	50	
Well No. 1 Pump	12	4	48	
Cielo Vista Pump Station Pump No. 1	11	4	44	
Cielo Vista Pump Station Pump No. 2	11	4	44	
Well No. 4 Pump	12	3	36	



TABLE 13: Risk Scoring of Pump Systems Assets - Manifolds and Flow Meters

Asset	Condition	Consequence	Risk Score		
	Score	Score			
Pershing PRV Station Manifold	24	8	192		
Cook St. Treatment Facility Pump Manifold	11	14	154		
Well No. 1 Pump Manifold	20	4	80		
Well No. 2 Pump Manifold	22	3	66		
Well No. 4 Pump Manifold	21	3	63		
Booster Pump Station No. 2 Pump Manifold	5	10	50		
Well No. 7 Pump Manifold	7	6	42		
Well No. 6 Pump Manifold	7	. 5	35		
Well No. 8 Pump Manifold	7	5	35		
Cielo Vista Pump Station Manifold	7	4	28 26		
Cook St. Treatment Facility Flow Meter	26	1			
Well No. 2 Flow Meter	17	1	17		
Well No. 7 Flow Meter	16	1	16		
Well No. 8 Flow Meter	16	1	16		
Well No. 6 Flow Meter	15	1	15		
Well No. 4 Flow Meter	15	1	15		
Booster Pump Station No. 2 Flow Meter	8	1	8		
Well No. 1 Flow Meter	3	1	3		

TABLE 14: Risk Scoring of Pump Systems Assets – Electrical Systems

Asset	Condition Score	Consequence Score	Risk Score
Well No. 1 Pump Electrical System	25	4	100
Well No. 2 Pump Electrical System	33	3	99
Well No. 7 Pump Electrical System	13	6	78
Well No. 4 Pump Electrical System	24	3	72
Booster Pump Station No. 2 Electrical System	7	10	70
Booster Pump Station No. 2 Back-up Generator	7	10	70
Well No. 6 Pump Electrical System	13	5	65
Well No. 8 Pump Electrical System	13	5	65
Cook St. Treatment Facility Electrical System	4	14	56
Cielo Vista Pump Station Electrical System	8	4	32



TABLE 15: Risk Scoring Pump System Assets – SCADA System

Asset	Condition	Consequence	Risk Score		
	Score	Score			
SCADA System RTUs	33	4	132		
SCADA System Software	31	4	124		

TABLE 16: Risk Scoring of Disinfection System Asset

Asset	Condition Score	Consequence Score	Risk Score	
Gas-Chlorination System	18	12	216	

Buildings and Structures - Condition and Risk Assessments

Functionality and Level of Service

The level of service for the buildings/structures assets of the Water System refers to the ability of the buildings/structures to protect system equipment from damage due to weather and vandalism and thereby keep the system in good operational condition. Therefore, level of service is defined by three categories:

- 1) customer driven (protection of equipment to provide potable water with minimal water outages),
- 2) regulatory (protection of equipment to provide potable, uncontaminated water), and
- 3) management (protection of equipment and conservation of energy to minimize operation and maintenance costs).

Energy conservation is an important element of the management category of the level of service for the buildings and structures. Majority of the Water System energy consumption is done by the pump systems, but some energy is also used by the buildings, primary the Cook Street Treatment Facility Pump Building, Booster Pump Station No. 2 Building, and the Pump Buildings for Wells No. 6, No. 7, and No. 8. Water System operators currently implement conservation of energy when using these buildings by making sure to turn off lights and heating/cooling systems when leaving the buildings. Future energy conservation efforts in regards to the buildings will be implemented in the same manner.

Modes of Failure

A building or structure "failure" refers to a scenario in which the condition of the building does not allow the components of the Water System within the building to be operated in a reasonable manner. For example, if the roof of a building asset at a level of disrepair that allows rainwater into the building that could damage equipment or create a



dangerous working environment for operators, the building asset would be considered in a state of failure. If buildings with important pump systems fail, there would be an interruption in service (reduced customer driven level of service). If the structure housing the gas-chlorination disinfection system failed and caused the system to stop disinfecting water, the water would not meet permit standards (reduced regulatory driven level of service).

Life Expectancy

The life expectancy of a building/structure asset is the average number of years before replacement or significant rehabilitation is necessary and is dependent on the manner in which it was constructed and the materials used to construct it. In general, the typical life expectancy of a building used in this AMP is 50 years⁹. Smith estimated the remaining useful life for each of the Water System building/structure assets based on age information collected from the City. The estimated remaining useful life for each of the building/structure assets are shown in TABLE 17.

Probability of Failure

As with the other assets of the Water System, the condition of a building/structure asset is the major factor in determining the probability of failure. The age of a building/structure and its remaining useful life is the primary measurement of condition. Since buildings and structures can be visually inspected, condition assessment based on field visits and operator input are also important measures of condition to be considered.

Smith implemented a remaining useful life scoring system based on a scale of 1 to 10 points. A score of 10 is associated with a building/structure that has no remaining useful life; a score of 1 is associated with a building/structure with 90-100% of its useful life remaining. For assets older than their life expectancies, a scoring adjustment was applied based on a linear relationship extrapolation from the typical life expectancy of 50 years. For example, a building that is 75 years old (150% of life expectancy) receives a score of 15 (10×1.5) .

Smith performed condition assessment on all the assets during multiple field investigations and documented the overall condition of the building/structures and any damage due to age and exposure to weather. In addition, Smith interviewed water system operators on the conditions of the buildings/structures and documented the operator comments and recommendations regarding replacement or rehabilitation of buildings/structures. The documented notes from the field condition assessments and operator interviews for each building/structure are contained in APPENDIX B. In general, the buildings were in good condition with some notable exceptions.

⁹ Typical expected life of a building defined as 50 years based on expected life ranges in "Asset Management: A Handbook for Small Water Systems", United States Environmental Protection Agency, 2003.



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Smith implemented a scoring system, to augment the useful life score, which considers asset condition observed during field visits and recommendations of Water System operation staff. Based on the condition of the building/structure asset observed during a field visit, 1-10 points were added to the useful life score of the asset, depending on the severity of the damage. Similarly, when a member of the operational staff made a recommendation regarding replacement or rehabilitation to a building/structure asset, 1-10 points were added to the useful life score of the asset, depending on the urgency of the recommendation. If the results of the field inspection and operator input were positive, 0 points were added.

TABLE 17 contains the Condition Scores for the building and structure assets of the Water System based on the condition assessment scoring system.

TABLE 17: Condition Assessment and Scoring of Building/Structure Assets

Asset	Life Expectancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Field Visit Assessment Score	Operator Interview Score	Condition Score
Pershing St. PRV Station Building	50	72	0	14	10	10	34
Well No. 4 Pump System Building	50	73	0	15	7	7	29
Well No. 2 Pump System Building	50	78	0	16	6	6	28
Well No. 1 Pump System Shade Structure	50	26	24	5	10	10	25
Cook St. Treat. Facility Storage Building	50	43	7	9	4	3	16
Gas-Chlorination System Fiberglass Shed	30	21	9	7	2	0	9
Cielo Vista Pump System Fiberglass Housing	30	11	19	4	2	0	6
Cook St. Treat. Facility Pump Building	50	21	29	4	1	0	5
Well No. 6 Pump System Building	50	18	32	4	0	0	4
Well No. 7 Pump System Building	50	18	32	4	0	0	4
Well No. 8 Pump System Building	50	18	32	4	0	0	4
Booster Pump Station No. 2 Building	50	10	40	2	0	0	2

Consequence of Failure

The function of an individual building or structure asset of the Water System is to protect the equipment housed within it. Therefore, the consequence of a building/structure failure, is defined as the same as the consequence of failure of the equipment housed



inside as shown in TABLE 18. Several of the buildings/structures protect critical components of the Water System, such as the chlorination system, key pumping systems, and the PRV that controls the pressure between the high and low pressure zones of the system, thus, have high consequence of failure scoring.

TABLE 18: Consequence of Failure Scoring of Building/Structure Assets

ABLE 18: Consequence of Failure S	comi	J 01 D	unui	ngrau	uctu	C AS	3613		CCODE
CONSEQUENCES	SOCI	AL		ENVI	RONME	NTAL	ECON	OMIC	SCORE
Criteria	Disruption of Service/ Access	Safety Impacts, Internal & External	Public Image	Aesthefics	Permit Violation	Sustainability	Level of Service, Reliability	High Repair/ Restore Cost	
Failure of Chlorination System Structure, resulting in loss of ability to pump chlorinated water to entire distribution system.		() <u></u>		x	х				12
Failure of Cook St. Treatment Facility Building critical component, resulting in loss of ability to pump chlorinated water to entire distribution system.					х				14
Failure of Booster Pump Station No. 2 Building critical component, resulting in loss of ability to pump chlorinated water to Upper Zone of the distribution system.					x				10
Failure of Pershing PRV Station Building, resulting in failure of PRV that separates High and Low Pressure Zones of Distribution System				Х	·	х			8
Failure of Pump System Building critical component of Well No. 6						х			5
Failure of Pump System Building critical component of Well No. 7						×			6
Failure of Pump System Building critical component of Well No. 8						x			5
Failure of Pump System Shade Structure critical component of Well No. 1			k ig			х			4
Failure of Pump System Building critical component o Well No. 2						X			3
Failure of Pump System Building critical component o Well No. 4						X			3
Failure of Cielo Vista Booster Pump Station Housing			1	Х			Sis.		4
Failure of Cook St. Facility Storage Building	.02)	W. Com							2

Risk of Failure

The risk of failure for a building/structure asset, the Risk Score, is the product of the probability of failure (Condition Score) and the consequence of failure (Consequence



Score). The Risk Score prioritizes, or ranks, the assets to appropriately plan and schedule replacement or rehabilitation as discussed in the Replacement Schedule section of this report. The Risk Scores assigned to each of the building and structure assets are shown in TABLE 19.

TABLE 19: Risk Scoring of Building/Structure Assets

Asset	Condition	Consequence	Risk Score
	Score	Score	
Pershing St. PRV Station Building	34	8	272
Gas-Chlorination System Fiberglass Shed	9	12	108
Well No. 1 Pump System Shade Structure	25	4	100
Well No. 4 Pump System Building	29	3	87
Well No. 2 Pump System Building	28	3	84
Cook St. Treat. Facility Pump Building	5	14	70
Cook St. Treat. Facility Storage Building	16	2	32
Cielo Vista Pump System Fiberglass Housing	6	4	24
Well No. 7 Pump System Building	4	6	24
Well No. 6 Pump System Building	4	5	20
Well No. 8 Pump System Building	4	5	20
Booster Pump Station No. 2 Building	2	· 10	20

It should be noted that the even though the Risk Score for the Cook St. Treatment Facility Pump Building is relatively high at 70, the building will not need to be replaced in the next ten years. In fact, the building still has an estimated useful life of 29 years and is in good condition. The Risk Score for the building is relatively high due to the criticality of the building and the pump system it houses (high Consequence Score).



Storage Tanks – Condition and Risk Assessments

Functionality and Level of Service

The storage tanks are extremely important parts of the Water System since they provide storage of potable water and create pressure and flow for proper distribution of water to customers and for fire protection. The level of service for water storage tanks can defined by three categories:

- 1) customer driven (water pressure and flow, minimal water outages),
- 2) regulatory (fire storage capacity¹⁰),
- 3) management (operation and maintenance, including water and energy conservation).

In the event a water storage tank fails, the pressure and flow drops for customers who are part of the pressure zone served by the tank and service may stop for some customers (reduced customer driven level of service). The loss of the function of the tank may significantly reduce the ability to conduct the normal water distribution operations (reduced management driven level of service).

Water and energy conservation are important elements of the management category of the level of service for the storage tanks. Conservation of both water and energy are accomplished by minimizing the amount of water loss from the tanks due to overflow. Avoiding tank overflow is achieved by properly coordinating the flow rate of water into the tank with the flow rate of water out of the tank so as not to exceed the capacity of the tank. The proper coordination of tank inlet and outlet flow rates also conserves the energy used by pumps, as well as the wear and tear on the pumps, by minimizing the number of times the pumps start up and shut-off (start/stops).

At present, the normal operation of the Water System tanks adequately conserves water and energy, with the exception of the 0.2 MG tank at the Cook Street Treatment Facility. Due to the relatively small volume of the tank and significant difference between the pumping flow rate in and the pumping flow rate out (flow rate out > flow rate in), operation of the tank sometimes results in water loss due to tank overflow and consistently results in an excessive number of pump start/stops. Future implementation water and energy conservation in regards to the Water System tanks should focus on improving the operation of the 0.2 MG tank at the Cook Street Treatment Facility. This most likely can be accomplished by either installing an addition tank to increase tank capacity or by modifying the pumps/pump flow rates associated with the existing tank.

Modes of Failure

A storage tank "failure" refers when a portion of the tank breaks rendering the tank incapable of storing water in a reasonable manner.

¹⁰ This study did not evaluate pipe capacity (flow or pressure) for the distribution system.



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Life Expectancy

The life expectancy of a storage tank asset is the average number of years before replacement or significant rehabilitation is necessary. The life expectancy of a steel tank is dependent on its construction and whether or not is has cathodic protection. In general, the typical life expectancy of a storage tank asset used in this AMP is 50 years¹¹. Smith estimated the remaining useful life for each of the Water System storage tank assets based on age information collected from the City. The estimated remaining useful life of each building/structure assets are shown in TABLE 20.

Probability of Failure

The condition of a storage tank asset is the most important factor in determining the probability of failure. The estimated remaining useful life of a storage tank, based on its age, is the primary measurement of condition. However, since storage tanks can be visually inspected, condition assessment based on field visits and operator input are also important measures of condition taken into account.

Smith implemented a remaining useful life scoring system based on a scale of 1 to 10 points. A score of 10 is associated with a storage tank that has no remaining useful life; a score of 1 is associated with a storage tank with 90-100% of its useful life remaining. For assets older than their life expectancies, a scoring adjustment was applied by using a linear extrapolation based the typical life expectancy of 50 years. For example, a storage tank that is 75 years old (150% of life expectancy) receives a score of 15 (1.5 x10).

Smith performed visual condition assessment on the outer portions of the storage tank assets during multiple field investigations and documented the overall condition of the storage tanks and any damage due to age and operational wear. In addition, Smith interviewed water system operators on the conditions of the storage tanks and documented the operator comments and recommendations regarding replacement or rehabilitation of buildings/structures. The documented notes from the field condition assessments and operator interviews on the storage tanks are contained in APPENDIX B. Overall, the storage tanks were in good or very good condition.

Smith implemented a scoring system, to augment the useful life score, which takes into account asset condition observed during field visits and recommendations of Water System operation staff. Based on the condition of the storage tank asset observed during a field visit, 1-10 points were added to the useful life score of the asset, depending on the severity of the damage. Similarly, when a member of the operational staff made a recommendation regarding replacement or rehabilitation to a storage tank asset, 1-10 points were added to the useful life score of the asset, depending on the urgency of the recommendation. If the results of the field inspection and operator input were positive, 0 points were added.

¹¹ Typical expected life of a well asset defined as 25 years based on expected life range in "Asset Management: A Handbook for Small Water Systems", United States Environmental Protection Agency, 2003.



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TABLE 20 contains the Condition Scores for the storage tank assets of the Water System based on the condition assessment scoring system.

TABLE 20: Condition Assessment and Scoring of Storage Tank Assets

Asset	Life Expectancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Field Visit Assessment Score	Operator Interview Score	Condition Score
3.0 MG Storage Tank next to Booster Pump Station No. 2	50	13	37	3	0	0	3
1.2 MG Storage Tank on Cemetery Rd.	50	13	37	3	0	0	3
3.0 MG Storage Tank on Cemetery Rd.	50	4	46	1	0	0	1
0.2 MG Storage Tank at Cook St. Facility	50	5	45	1	0	0	1

Consequence of Failure

The consequences of a storage tank failure are loss of proper storage capacity, pressure, and flow for portions of the system associated with the tank, which may result in the loss of service to some customers, as well as potential damage to the environment around the tank. In addition, repair or replacement of a storage tanks is very expensive. In general, the consequences of a storage tank failure are quite severe as indicated by the consequence scoring shown in TABLE 21. The Consequence Scores for the tanks of the upper zone are not as high as for the other two tanks because the two upper zone tanks provide some redundancy to each other.



TABLE 21: Storage Tank Assets Consequence of Failure Scoring

TABLE 21. Otorage Talik Assets		-					_			
	SOCIAL			ENV	RONN	1ENTA	L	ECONO	MIC	SCORE
Criteria	Disruption of Service/ Access	Safety Impacts, Internal & External	Public Image	Aesthetics	Permit Violation	Ecosystem	Sustainability	evel Of Service, Rehability	нgh Repair/ Restore Cost	
Failure of the 0.200 MG Cook St. Treatment Facility Tank, resulting in loss of ability to store and distribute chlorinated water to either the Upper or Lower Zones of the distribution system.	O S.	У G	<u>č</u>		X	x	X	7	王	20
Failure of the 3.0 MG Lower Zone Tank, resulting in loss of ability to store and distribute chlorinated water to Lower Zone of the distribution system.				x	×	`X	х		A CASA TO THE PARTY OF THE PART	20
Failure of the 3.0 MG Upper Zone Tank, resulting in loss of ability to store and distribute chlorinated water to Upper Zone of the distribution system with proper pressure and quantity.				х	x	x	х			18
Failure of the 1.6 MG Upper Zone Tank, resulting in loss of ability to store and distribute chlorinated water to Upper Zone of the distribution system with proper pressure and quantity.				х	x	х	x		Prodes and also have	15

Risk of Failure

The risk of failure score for a storage tank asset, the Risk Score, is the product of the probability of failure (Condition Score) and the consequence of failure (Consequence Score). The Risk Score prioritizes, or ranks, the assets to appropriately plan and schedule replacement or rehabilitation as discussed in the Replacement Schedule section of this report. The Risk Scores for each of the storage tank assets are shown in TABLE 22. It should be noted that the low Risk Scores for the tank assets indicate that none of the tanks will need to be replaced in the next 10 years.

TABLE 22: Storage Tank Assets Risk Scoring (Ranking)

	- ,	• •	
Asset	Condition Score	Consequence Score	Risk Score
3.0 MG Storage Tank next to Booster Pump Station No. 2	3	20	60
1.2 MG Storage Tank on Cemetery Rd.	3	15	45
0.2 MG Storage Tank at Cook St. Facility	1	20	20
3.0 MG Storage Tank on Cemetery Rd.	1	18	18



ASSET FINANCIALS AND CAPITAL IMPROVEMENTS PLAN

This AMP is intended to be a guide for improving the Water System by planning the most cost-effective use of the system assets. The Water System budget for future years needs to be developed by incorporating proper financial planning for continuing the operation and maintenance of the system while also funding the replacement of significant amounts of system assets. This section of the report outlines an approach to managing the financials of the Water System assets including a 10-year Capital Improvements Plan (CIP) for asset rehabilitation and replacement.

OPERATION AND MAINTENANCE

The approach to the operation and maintence of the different categories of Water System assets has an impact on the financials of the system. Depending on the nature of the asset, the most cost-effective approach to long-term use can vary from operating the asset until it fails (run to failure) to proactive replacement of the asset before it is expected to fail. Historically, the general approach to operation of the Water System assets was to run them to failure, the point when they stopped working and required emergency repair or replacement. This approach to asset maintence is generally referred to as Corrective Maintenance. Notable exceptions to this general approach were the planned rehabilitation and replacement of several storage tanks in the system, which were examples of Planned Maintenance. The Planned Maintenance approach was certainly more cost-effective (and safe) than running the tanks to failure.

This AMP recommends that the general approach to future operation and maintenance of all the Water System assets be to rehabilitate or replace assets prior to failure based on the Risk Scoring discussed in previous sections of this report. This approach is a combination of what are generally referred to as Planned Maintenance and Preventative Maintenance. Preventative maintenance of assets that can have their operational condition routinely checked and assessed, such as pumps, pump motors, buildings, and tanks is a cost-effective approach, allowing repair or rehabilitation to occur at a reasonable time and not as an emergency. Planned maintenance (scheduled rehabilitation or replacement) based on criticality is the most cost-effective approach with the vast majority of the Water System assets, in particular the waterline pipes, that can't be routinely assessed by operators.

To assist financial management decisions regarding Water System assets, the cost accounting of assets in this AMP and GIS Model includes the following cost components for an asset: initial cost, O&M costs (including minor repairs), and rehabilitation or replacement costs. However, for the vast majority of the assets, initial costs are not available and, as a result, are not a significant factor in the cost accounting of the assets.



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Similarly, O&M costs for individual assets are not identifiable and therefore not available due to the current Water System tracking of annual O&M expenditures in an aggregate form. As a result, the primary cost components presently available for managing the assets are the estimated rehabilitation or replacement costs of the assets, which are contained in APPENDIX A and APPENDIX B and in the GIS Model Attribute Tables.

In the future, the cost accounting of Water System assets will be much more detailed and useful in assisting asset management. This is because the Water System staff will be able to use the GIS Model to track initial costs and O&M costs for individual assets much more effectively allowing these cost components to be significant factors in management decisions. As previously mentioned, the GIS Model of the Water System contains Attribute Tables for each of the different types of assets. There is an individual, separate Attribute Table following asset categories:

- 1) Distribution System (Waterline pipes, Valves, Meters, Hydrants) "field assets"
- 2) Wells
- 3) Pump Systems (Pumps)
- 4) Pump Systems (Motors)
- 5) Pump Systems (Manifolds and Flow Meters)
- 6) Pump Systems (Electrical and control Systems)
- 7) Pump Systems (SCADA System)
- 8) Buildings and Structures
- 9) Storage Tanks

The Attribute Tables contain information on all the assets, including previous repair/rehab costs and replacement costs, which are tracked for each, individual asset separately. As part of regular future operation and maintenance practices, the Water System staff will be able to track the repairs to assets or replacement of assets by Work Order. Once a Work Order has been processed for an individual asset, the Attribute Table associated with the asset can be updated with the new data in the GIS Model using the ArcMap 10.3 software.

ASSET REPLACEMENT SCHEDULE

This AMP is intended to be a guide for a 10-year Capital Improvements Plan (CIP) for the replacement and rehabilitation of components of the City Water System. Based on the condition and risk assessment calculations described in previous sections, each asset of the Water System has a Risk Score assigned to it. As previously discussed, the Risk Score of an asset is a numeric value used to give the asset priority, or ranking, in the replacement and rehabilitation scheduling of the CIP. APPENDICES A and B contain information on the Water System assets used to determine Risks Scores. In addition to replacement and rehabilitation of Water System assets based on Risk Scores, the 10-year CIP also needs to take into account improvements to assets required for planned City growth and for improvement to conservation of water and energy.

The capacity of the overall Water System is essentially adequate for anticipated City growth over the next ten years. The wells, pump systems, buildings/structures, and



storage tanks do not need to have increased capacity as part of the improvements of the 10-year CIP, only rehabilitation or replacement based on Risk Score. On the other hand, increases to capacity and energy efficiency should be addressed as part of replacing the waterline pipes of the distribution system in the 10-year CIP. In areas of planned growth or redevelopment, such as the Downtown area, slight upsizing of waterline pipe where necessary, such as replacing a 4-inch pipe with a new 6-inch pipe, should be done as part of the waterline pipe replacement process. As previously discussed, improvements to conservation of water and energy will be addressed by replacing waterline pipes that are presently leaking. The best way to address needed increases in Water System capacity and conservation is to systematically replace old leaking waterlines, prioritized by Risk Score, throughout the distribution system.

REPLACEMENT OF WATER SYSTEM DISTRIBUTION SYSTEM ASSETS (WATERLINES, VALVES, METERS, AND HYDRANTS)

The assets of the Water System distribution system have been categorized into one of four Risk Levels, based on the Risk Score assigned to the waterline pipe assets. The four Risk Levels for the assets are as follows:

Low Risk Level −Risk Score range from 1 to ≤ 8 Medium Risk Level −Risk Score range from >8 to ≤ 12 High Risk Level −Risk Score range from >12 to ≤ 56 Extreme Risk Level −Risk Score range from >56 to ≤ 168

Ideally, the City should replace or rehabilitate Water System assets at a rate such that in by 2025, there will be no assets in the Extreme Risk Level remaining in the system. At the current spending rate, the City will not accomplish this goal.

In theory, each waterline should be replaced at the end of its estimated useful life to keep up with system maintenance dictated by the aging process of the pipes. Based on an average useful life of 75 years for the Water System distribution waterline pipes, this equates to an annual replacement rate of 1.3% of the waterlines assets, or about 5,000 feet. This means that replacing 1.3% of the system annually for the next 10 years would be the minimum amount to keep pace with the aging system. As previously discussed and shown in FIGURE 5, 18% of the existing distribution system has been assigned to the Extreme Risk Level. Due to continued aging, if no repair or replacement to the distribution waterlines is done over the next 10 years, the amount of the system that will be assigned to the Extreme Risk Level will increase to 27% by 2025 as shown in FIGURE 7.

Based on the risk analysis results, Smith recommends that the City target the replacement of all Extreme Risk Level waterline pipe assets over the next 10 years. This would include pipe that is already Extreme Risk and pipe that will become Extreme Risk by 2025 or approximately 105,800 lineal feet (LF) of waterlines. This equates to replacement of 2.7% of the existing system per year, or approximately 10,600 LF per year, at an estimated



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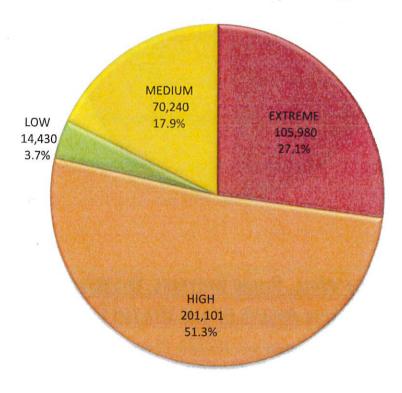
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average cost of $$185/LF^{12}$. This will require an annual replacement budget of approximately \$1,970,000.

FIGURE 7 and FIGURE 8 illustrate the impact on waterline failure risk of two replacement scenarios, no replacement by 2025 and the recommended replacement of all Extreme Risk Level waterlines by 2025.

FIGURE 7: Waterline Risk in 2025 with No Replacement



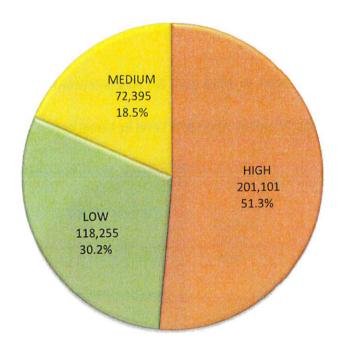


¹² Cost is the average cost per LF based on waterlines sizes of 6", 8", 10", 12" and 14" and includes replacement of waterline pipe and associated service connections, meters, valves, hydrants, and pavement, as well as excavation and trench backfill.



FIGURE 8: Waterline Risk with all Extreme Risk Level (Top 27%) Replaced





REPLACEMENT OF WELL, PUMP SYSTEMS, DISINFECTION SYSTEM AND BUILDING/STRUCTURE ASSETS OF THE WATER SYSTEM

The vast majority of the CIP budget over the next 10 years will need to be dedicated to replacing large portions of the distribution system waterlines (and associated valves, meters, and hydrants). However, a portion of the 10-year CIP budget needs to be dedicated to the replacement of components from the other Water System asset categories of wells, pump systems, disinfection system, and buildings/structures. The Risk Scores calculated for the storage tank assets of the Water system indicate that none of the tanks will need to be scheduled for replacement in the next 10 years.

As discussed previously, Risk Scores were calculated and assigned to the well, pump system, disinfection system and building/structure assets of the Water System, all of which are contained in early sections of the report and in APPENDIX B. In general, assets that were assigned Risk Scores above 50 are considered to be beyond an acceptable level of risk and should be ideally replaced or repaired in the next 10 years. Based on the individual Risk Scores for the assets (the higher the score, the higher the priority) and special circumstance associated with certain assets, Smith generated a schedule and budget for replacing well, pump system, disinfection system and building/structure assets for each fiscal year for 10 years. The CIP replacement schedule and budget for these



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assets is shown in conjunction with the waterline asset replacement schedule and budget in TABLE 23. The amount of CIP spending per year was scheduled in a manner designed to keep cost as consistent year to year as possible taking into account a practical replacement approach to the components (i.e. similar equipment replaced together).

10-YEAR CIP SCHEDULING AND BUDGET FOR COMPREHENSIVE REPLACEMENT OF WATER SYSTEM ASSETS

Based on analysis of the Risk Scores assigned to all the Water System assets, Smith has created a comprehensive 10-year CIP Schedule and estimated budget for asset replacement. The assets scheduled for replacement in each fiscal year and the estimated CIP cost amount budgeted for each year are illustrated in TABLE 23. Over the next 10 years, Smith recommends that the City spend between approximately \$2.3 million and \$2.9 million annually to proactively manage the replacement and repair Water System assets. The CIP budget amounts contained in TABLE 23 are the amounts estimated each year to replace the aging components of the system, before they fail, and should be considered "Replacement Funds" and is a new element beyond the what is presently included in the existing annual Water System budgeting done by the City.



TABLE 23: 10-Year CIP Schedule and Budget for Comprehensive Replacement of Water

System Assets

System	Assets		
Year	Water System Asset	Risk Score	Estimated Cost
2018/	Waterline Asset Replacement (2.7% of Distribution System)	>56	\$2,029,100
2019	New 250,000 Gallon Tank for Cook St. Treatment Facility	NA	\$400,000
	Pershing St. PRV Station Building (Total Replacement)	272	\$103,000
	Cook St. Treatment Facility Pump No. 2 Motor	196	\$15,450
	Gas-Chlorination System	192	\$154,500
	Gas-Chlorination System Fiberglass Shed	96	\$10,300
	Pershing PRV Station Manifold	184	\$20,600
	2018 Tot	al CIP Amount	\$2,732,950
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2020/	Waterline Asset Replacement (2.7% of Distribution System)	>56	\$2,152,672
2021	Cook St. Treatment Facility Back-up Generator	NA	\$240,400
1		al CIP Amount	\$2,393,072
202474	Warterlines Assauge placement to a war to pendicional systems.		
2022	cogs reaches and course the constant		
	The state of the s	are francollina	
2022/	Waterline Asset Replacement (2.7% of Distribution System)	>56	\$2,283,770
2023	Cook St. Treatment Facility Pump No. 2	112	\$139,113
	Well No. 6 Pump Motor	110	\$6,376
	Well No. 8 Pump Motor	110	\$6,376
		al CIP Amount	\$2,435,635
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TABLE 23 Con't: 10-Year CIP Schedule and Budget for Comprehensive Replacement of Water System Assets

Year	Water System Asset	Risk Score	Estimated Cost
2024/	Waterline Asset Replacement (2.7% of Distribution System)	>56	\$2,422,852
2025	Well No. 2 Pump Electrical System	93	\$43,046
	Well No. 2	84	\$30,747
	Well No. 2 Pump System Building	81	\$122,987
	Well No. 2 Pump Manifold	63	\$18,448
	2024 Tota	CIP Amount	\$2,638,079
11 A			
	Secretaria de la companya de la comp	2. Was - 3. Was 18 (S.	
2026/	Waterline Asset Replacement (2.7% of Distribution System)	>56	\$2,570,403
2027	Well No. 4 Pump System Building	84	\$130,477
	Well No. 4 Pump Electrical System	69	\$45,667
	Cielo Vista Pump Station Pump No. 2 Motor	68	\$652
	66	\$32,619	
	66	\$65,239	
:	Well No. 4 Pump Manifold	60	\$19,572
	Well No. 4 Pump Motor	51	\$391
	2026 Total	CIP Amount	\$2,865,021
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	Figure 200 to the control of the con		
	i projekti ki nje vjete se posledi se posledi se posledi se posledi se i posledi i i i i i i i i i i i i i i i Nastrova ve posledi seve i se posledi projekti se posledi se posledije se pasa se posledije se posledije po se		

Estimate Costs for replacement of Water system assets are based on a 3% inflation rate compounded annually. Inflation rate of 3% based on inflation rates over the past 10 years, as documented on www.usinflationrate.org, accessed 11-18-14.

Water System Budgeting

Implementation of the 10-Year CIP while continuing the normal operations and maintenance of the system will require very significant modifications to the Water System Annual Operating Budget. Smith recommends that an "Annual Replacement Fund" be set-up as part of the City's Water System Annual Operating Budget to provide funding



specifically designated for the replacement of assets that have reached levels of unacceptable risk due to age. As shown in TABLE 23, funding of the recommended CIP will require an Annual Replacement Fund of between \$2.3 million and \$2.9 million for each of the next 10 years. To achieve the necessary amounts of annual funding, the Water System will need to generate large amounts of revenue internally through increases in service rates and externally by acquiring loans and grants from various funding agencies.

TABLE 24 shows the existing Rate Schedule of the T or C Water System. The annual revenue generated from the existing Water System service rate structure (schedule) is adequate for covering the current annual operation and maintenance expenses and outstanding debt repayment of the system, (see Water System Historical Data in APPENDIX C). However, the annual revenue generated by the Water System is only adequate when no revenue is contributed to a dedicated Annual Replacement Fund as will be required by the CIP. As a result, the existing rate structure of the Water System must be dramatically increased to provide revenue to pay for very large increases in budget expenses that will be incurred by implementing the CIP.

TABLE 24: Existing Utility Rate Schedule for the City of T or C Water System

TABLE 24. Existing Starty Rate Seriodate 15. and Stay	
Residential Rates - City of Tor C and Village of Williamsburg	
Base Customer Charge (Minimum)	\$8.15
Rate per 1000 gallons for Level 1 Usage (1-7,000 gallons)	\$1.75
Rate per 1000 gallons for Level 2 Usage (7,001-30,000 gallons)	\$1.93
Rate per 1000 gallons for Level 3 Usage (30,001-50,000 gallons)	\$2.12
Rate per 1000 gallons for Level 4 Usage (Above 50,000 gallons)	\$2.33
Commercial Rates - City of <i>T or C</i> and Village of Williamsburg	
Base Customer Charge (Minimum)	\$8.15
Rate per 1000 gallons for Level 1 Usage (1-7,000 gallons)	\$1.75
Rate per 1000 gallons for Level 2 Usage (7,001-30,000 gallons)	\$1.93
Rate per 1000 gallons for Level 3 Usage (30,001-50,000 gallons)	\$2.12
Rate per 1000 gallons for Level 4 Usage (Above 50,000 gallons)	\$2.33
Industrial Rates - City of T or C	各个种的
Base Customer Charge (Minimum and for Usage 1-50,000 gallons)	\$91.91
Rate per 1000 gallons for Level 2 Usage (50,001-100,000 gallons)	\$1.84
Rate per 1000 gallons for Level 3 Usage (100,001-150,000 gallons)	\$2.02
Rate per 1000 gallons for Level 4 Usage (Above 150,000 gallons)	\$2.22
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Water System Utility Rate Schedule http://www.torcnm.org/departments/finance/utilities - billing.php accessed 3/15/17



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TABLE 25A shows the Projected 10-Year Water System Annual Operating Budget, including the 10-Year CIP recommended by this AMP. The table contains all the projected revenue and expenses of the Water System. Projected revenue is based on estimated increases in service connections, recommended service rate increases, and loans required for complete funding of the recommended 10-Year CIP (Phases 1-10) detailed in TABLE 23. Projected expenses are based on historical system financials taking into account future changes to the system, existing debt repayment, and additional debt repayment associated with the new loans that will be needed compensate for the estimated Water System annual service revenue shortfalls in funding the CIP.

Smith recommends a balanced funding approach that uses internal revenue from service rates in combination with external loans from funding programs to pay for the very large future annual expenses of the CIP. As shown in TABLE 25A, a large portion of future CIP funding is estimated as loans, at a total of approximately \$17 million dollars spread out over 10 years. To keep down the future annual loan debt repayment amounts and to generate sufficient internal revenue to operate the Water System with no additional loans after addressing the EXTREME piping, the existing water utility service rates must be dramatically increased. Smith shows that the existing Water System service rates, at all base and usage levels must be increased initially by 50%, followed by 10% annual increases for six years then followed by 8.0% annual increases for the next four years. These recommended rate increases are based on the projected requirements of the CIP Annual Replacement Fund for taking into account anticipate funding from loans averaging approximately \$1.7 million annually for those same years (see TABLE 25A).

There is the possibility that a significant portion of the external funding for the 10-Year CIP may come from grants or grant/loan combinations. For the purpose of comparison, a second version of the projected Annual Operating Budget was developed to take into account the impact of potential grant funding. The projected budget shown in TABLE 25B contains all the same information as the budget in TABLE 25A, except it considers 50% of the external revenue from funding programs to be in the form of grants instead of all external revenue being in the form of loans. Comparison of the two projected Annual Operating Budgets reveals significant benefits of 50% external revenue as grant funding. First of all, the total amount of loans taken out to fund the 10-Year CIP is estimated approximately \$8.0 million rather than \$17 million, resulting in annual loan repayments in the years of 2026/27 and beyond of approximately \$450,000 rather than almost \$1.0 million. Second, the recommended service revenue increases over the next 10 years are estimated to be significantly less. If 50% of the external revenue for funding the 10-Year CIP is accounted for as grants, the recommended Water System service rates increases are as follows: service rates at all base and usage levels, should be increased by 50% in FY 2016/17, followed by 8% annual increases from FY 2017/18 through FY 2022/23, followed by 6% annual increases from FY 2023/24 through FY 2025/26. It is important to note that, while the recommended rate increases when taking into account grants are less than if all



the external revenue was in the form of loans, the rate increases are still dramatic and will be challenging to implement.



TABLE 25A: Projected 10-Year Water System Annual Operating Budget Including CIP (with Debt Repayment- 100% Loans)

FISCAL YEAR	3 2017/18	2018/19	2019/20	7070/71	2021/22	56/6606	אבן צבטר	ac/ Veoc	24/ 1404	/ 5.00	
Tor C Residential Connections [1]	Ĺ.		2657	2683		7737	7765		07/07/7	17/9707	87//707
Tor C Commercial Connections[1]	593		605			673	679		2020	2848	7/87
Tor C Governmental Connections	8		8				3	CCO C	240	040	655
Willaimsburg Residential Connections [1]	249	251	254	256	25	262	264	296	270	5 770	37.0
Willaimsburg Commercial Connections [1]	22	23	23	23	23	24	24	24	24	75	27.2
Total Service Fee Revenue [2]	\$1,516,721	\$1,685,078	\$1,872,121	\$2,079,927	\$2,310,798	\$2,567,297	\$2,852,267	\$3,111,253	\$3,393,755	\$3.701.908	\$ 3.738.977
Revenue from Other Services	\$ 40,164	\$ 40,566	\$ 40,972	\$ 41,381	\$ 41,795	\$ 42,213	\$ 42,635	\$ 43,061	\$ 43,492	\$ 43.927	i
Funding - LOANS (100% of Shortfall)	\$1,870,000	\$2,280,000	\$1,801,000	\$1,816,000	\$1,535,000	\$1,430,000	\$1,341,000	\$1,290,000	\$1,030,000	1.0	١.,
Rollever from Previous Year	\$ 10,640	\$ 10,640 \$ 12,070	\$ 5,516	\$ 894	\$ 363	\$ 6,524	\$ 7,891	\$ 12,480	\$ 7,198	\$ 3,664	\$ 901
OPERATING REVENUE (INCOME)	\$3,437,525	\$3,437,525 \$4,017,713	\$3,719,609	\$3,938,202	\$3,887,956	\$4,046,034	\$4,243,793	\$4,456,794	\$4,474,445	786	\$ 3,784,194
Exist. Loan Debt Repayment [3]	\$ 253,563	\$ 253,563	\$ 253,563	\$ 253,563	\$ 133,322	\$ 99,413	\$ 99,413	\$ 99,413	\$ 99,413	\$ 99,413	\$ 99,413
Exist. Loan Debt Service Reserve [4]	\$ 25,356	\$ 25,356	\$ 25,356	\$ 25,356	\$ 13,332	\$ 9,941	\$ 9,941	\$ 9,941			
New Water System Improvement											
Construction Projects - PHASES 1 -10	\$2,307,458	\$2,732,950	\$2,307,458	\$2,393,072	\$2,352,313	\$2,435,635	\$2,523,033	\$2,638,079	\$2,571,543	\$2,865,021	\$ 1 500 000
(Annual Replacement Fund) [5]									21 26 206 2		200,000,000
New Loans Debt Repayment [6]	\$ 105,726	\$ 234,632	\$ 336,456	\$ 439,129	\$ 525,914	\$ 606,763	\$ 682,580	\$ 755,514	\$ 813,748	\$ 813.748	\$ 813.748
New Loans Debt Service Reserve [4]	\$ 10,573	\$ 23,463	\$ 33,646	\$ 43,913	\$ 52,591	\$ 60,676	\$ 68,258	\$ 75,551	1	1	
Personnel/Admin [7]	\$ 410,580	\$ 422,897	\$ 435,584	\$ 448,651	\$ 462,111	\$ 475,974	\$ 490,253	\$ 504,961	\$ 520,110	\$ 535,713	 3
Maintenance/Repair [7]	\$ 157,677	\$ 162,408	\$ 167,280	\$ 172,298	\$ 177,467	\$ 182,791	\$ 188,275	\$ 193,923	\$ 199,741	\$ 205,733	\$ 211,905
Supplies [7]	\$ 10,109	10,109 \$ 10,413	\$ 10,725	\$ 11,047	\$ 11,378	\$ 11,719	\$ 12,071	\$ 12,433	\$ 12,806	\$ 13,190	\$ 13,586
Utility [8]	\$ 144,414 \$ 146,515	\$ 146,515	\$ 148,647	\$ 150,810	\$ 153,004	\$ 155,230	\$ 157,489	\$ 159,780	+ -	\$ 164,464	+
OPFRATING EXPENSES (with debt repayment)	\$3,425,456	\$3,425,456 \$4,012,197 \$1	\$3,718,715	\$3,937,839	\$3,881,433	\$4,038,143	\$4,231,313	\$4,449,596	-	865'881'	500
NET INCOME (LOSS) (with debt repayment)	\$ 12,070	\$ 5,516	\$ 894	\$ 363	\$ 6,524	7,891	\$ 12,480	\$ 7,198	\$ 3,664	\$ 901	\$ 335,585
[1] Increases in connections are based on an applied population growth cate of 1% and the	wondation grow	th rate of 1% and t		a contract of the second secon	A COC modern C 30						

[1] Increases in connections are based on an annual population growth rate of 1% and the number existing connections as of October 2014.

[2] Service Fee Revenue is based on projected number of connections and the following increases to the Water Rate schedule shown in TABLE 24: 50% increase to all base and usage rates initially; 10% increase to all base and usage rates each year for 6 years; 8% annual increase to all base and usage rates for 3 years. Revenue is expected to grow at about 1% per year if rates remain the same.

[3] Annual repayment amount of all existing loan amount estimated as approx \$287,716 based on recent loan information provided by City, contained in APPENDIX C.

[4] Debt Service Reserve defined as 10% of annual debt repayment requirement.

[5] The amount of each Water System Improvement Construction Project (Phases 1-10) is based on the estimated annual costs 10-Year CIP, as shown in TABLE 23. Each Phase of the CIP represents the Annual Replacement Fund of the Water System

[6] Repayment of New Loan amounts assumed at 1.2% annualized over 20 years as per Appendix C of OMB Circular A-94.

[7] Expenses based on S-Year Average of Water System Expenses from data provided by the City for FY 2009/10 through 2013/14, contained in APPENDIX C. Expenses for years after 2016/17 were calculated based on 2016/17 Including 3% annual [8] Power Expenses based on 5-Year Average of Water System Expenses from data provided by the City for FY 2009/10 through 2013/14, contained in APPENDIX C. Power Expenses for years after 2016/17 were calculated based on 2016/17 costs including 3% annual energy savings due to improved energy conservation based on replacement of leaking pipes.



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FISCAL YEAR	K 201//18	2018/19	2019/20	2020/21	2021/22	2022/23	2023/24	2024/25	2025/26	2026/27	2027/28
Tor C Residential Connections [1]	2604	2630	2657	2683	2710	2737	2765	2792	2820	2848	7877
Tor C Commercial Connections[1]	593	599	909	611	(19	623	629	635	642	648	655
Tor C Governmental Connections	3	æ	3	E	E	m	3	m		r r	t
Willaimsburg Residential Connections [1]	249	251	254	256	259	262	264	797	27	272	275
Willaimsburg Commercial Connections [1]	22	23	23	23	23	24	24	24		25	25
Total Service Fee Revenue [2]	\$1,516,721	\$1,685,078	\$1,872,121	\$2,079,927	\$2,310,798	\$2,567,297	\$2,852,267	\$3,111,253	\$3,393,755	\$3,701,908	\$ 3.738.927
Revenue from Other Services	\$ 40,164	\$ 40,566	\$ 40,972	\$ 41,381	\$ 41,795	\$ 42,213	\$ 42,635	\$ 43,061	\$ 43,492	\$ 43,927	,
Funding - LOANS (50% of Shortfall)	\$ 900,000	\$1,080,000	\$ 805,500	\$ 778,000	\$ 603,000	\$ 527,500	\$ 453,500	\$ 405,500	\$ 250,500	\$ 246,000	\$
Funding - GRANT (50% of Shortfall)	\$ 900,000	900,000 \$1,080,000	\$ 805,500	\$ 778,000	\$ 603,000	\$ 527,500	\$ 453,500	\$ 405,500	\$ 250,500	\$ 246,000	
Rollever from Previous Year	\$ 10,640 \$	\$ 2,395	\$ 10,798	\$ 13,043	\$ 13,934	\$ 10,479	\$ 12,358	\$ 13,655	\$ 15,089	\$ 16,749	\$ 180
OPFRATING REVENUE (INCOME)	\$3,367,525	\$3,888,039	\$3,534,890	\$3,690,351	\$3,572,527	\$3,674,989	\$3,814,261	\$3,978,969	\$3,953,336	\$4,254,584	\$ 3,783,473
Exist. Loan Debt Repayment [3]	\$ 253,563	\$ 253,563 \$ 253,563	\$ 253,563	\$ 253,563	\$ 133,322	\$ 99,413	\$ 99,413	\$ 99,413	\$ 99,413	\$ 99,413	\$ 99,413
Exist. Loan Debt Service Reserve [4]	\$ 25,356	\$ 25,356	\$ 25,356	\$ 25,356	\$ 13,332	\$ 9,941	\$ 9,941	\$ 9,941	\$ 9,941	\$ 9,941	
New Water System Improvement								ŀ			
Construction Projects - PHASES 1 -10	\$2,307,458	\$2,732,950	\$2,307,458	\$2,393,072	\$2,352,313	\$2,435,635	\$2,523,033	\$2,638,079	\$2,571,543	\$2,865,021	\$ 1.500.000
(Annual Replacement Fund) [5]											,
New Loans Debt Repayment [6]	\$ 50,884	\$ 111,945	\$ 157,486	\$ 201,472	\$ 235,565	\$ 265,388	\$ 291,028	\$ 313,954	\$ 328,117	\$ 328,117	\$ 328,117
New Loans Debt Service Reserve [4]	\$ 5,088 \$	\$ 11,194	\$ 15,749	\$ 20,147	\$ 23,556	\$ 26,539	\$ 29,103	\$ 31,395	\$ 32,812	\$ 32,812	\$ 32,812
Personnel/Admin [7]	\$ 410,580	\$ 422,897	\$ 435,584	\$ 448,651	\$ 462,111	\$ 475,974	\$ 490,253	\$ 504,961	\$ 520,110	\$ 535,713	\$ 551,785
Maintenance/Repair [7]	\$ 157,677	\$ 162,408	\$ 167,280	\$ 172,298	\$ 177,467	\$ 182,791	\$ 188,275	\$ 193,923	\$ 199,741	\$ 205,733	\$ 211,905
Supplies [7]	\$ 10,109	\$ 10,413	\$ 10,725	\$ 11,047	\$ 11,378	\$ 11,719	\$ 12,071	\$ 12,433	\$ 12,806	\$ 13,190	\$ 13,586
Utility [8]	\$ 144,414	\$ 144,414 \$ 146,515	\$ 148,647	\$ 150,810	\$ 153,004	\$ 155,230	\$ 157,489	\$ 159,780	\$ 162,105	\$ 164,464	\$ 166,857
OPFRATING EXPENSES	\$3,365,130 \$3,877,24	\$3,877,241	53,571,848	\$3,676,417	\$3,562,048	\$3,662,631	\$3,800,606	53,963,880	53,036,587	\$4,254,404	\$ 2.914,415
(with debt cepayment)											
NET INCOME (LOSS) (with debt repayment)	\$ 2,395 \$	\$ 10,798	\$ 13,043	\$ 13,934	\$ 10,479	\$ 12,358	\$ 13,655	\$ 15,089	\$ 16,749	\$ 180	\$ 869,058

[1] Increases in connections are based on an annual population growth rate of 1% and the number existing connections as of October 2014.

[2] Service Fee Revenue Is based on projected number of connections and the following increases to the Water Rate schedule shown in TABLE 24:50% increase to all base and usage rates initially, 10% increase to all base and usage rates for 6 years; 8% annual increase to all base and usage rates for 3 years. Revenue is expected to grow at about 1% per year if rates remain the same.

[3] Annual repayment amount of all existing loan amount estimated as approx \$287,716 based on recent loan information provided by City, contained in ApPENDIX C.

[4] Debt Service Reserve defined as 10% of annual debt repayment requirement.

[5] The amount of each Water System Improvement Construction Project (Phases 1-10) is based on the estimated annual costs 10-Year CIP, as shown in TABLE 23. Each Phase of the CIP represents the Annual Replacement Fund of the Water System

[6] Repayment of New Loan amounts assumed at 1.2% annualized over 20 years as per Appendix C of OMB Circular A-94.

[7] Expenses based on 5-Year Average of Water System Expenses from data provided by the City for FY 2009/10 through 2013/14, contained in APPENDIX C. Expenses for years after 2016/17 were calculated based on 2016/17 including 3% annual [8] Power Expenses based on 5-Year Average of Water System Expenses from data provided by the City for FY 2009/10 through 2013/14, contained in APPENDIX C. Power Expenses for years after 2016/17 were calculated based on 2016/17 costs including 3% annual inflation rate and including an estimated 1.5% annual energy savings due to improved energy conservation based on replacement of leaking pipes.



To fund the recommended 10-Year CIP, the Water System needs a well-planned funding strategy. To assure adequately financial preparation, the City should anticipate that all future external funding for the CIP will be in the form of loans. Any future grant funding would clearly be a benefit to the community, but should not be counted on as part of the Water System financial planning. The CIP prioritization shown in TABLE 23 and the recommended service rate increases and loans contained in TABLE 25A should be the basis for the Water System funding strategy. To implement the CIP effectively, service revenue rate increases and external sources of revenue (loans and grants) should be reviewed and updated annually. Ideally, review of funding to implement the CIP should occur each year at the time the overall cost accounting of the Annual Water System Budget is done.

Water System Budgeting Beyond 10 year CIP

The 10-year CIP budget recommended in this AMP represents the spending deemed necessary to replace all the assets of the Water System that are at unacceptable levels of risk with very high potential for failure. However, CIP efforts to keep the Water System in good operational condition will not end in ten years, but rather will continue for the life of the City. In future years, the City will still need to dedicate large amounts of revenue to the Annual Replacement Fund to continue replacing the aging Water System infrastructure at the rate necessary. In regards to the waterline assets, the City should plan to dedicate enough revenue in the Annual Replacement Fund to replace 1.3% of the waterline assets of the distribution system every year. In addition, there must also be revenue set aside in the fund for the replacement of other Water System equipment such as pumps, motors, electrical components, buildings, etc. Based on the same cost estimating approach used to determine the replacement costs contained in TABLE 23, the planned amount for the Annual Replacement Fund in years beyond the 10 years reviewed in the plan should be approximately \$1.5 million. The annual revenue generated from the recommended service rate increases is expected to be sufficient to cover all annual operating expenses of the system..



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CONCLUSIONS AND RECOMMENDATIONS

To develop this comprehensive plan for managing assets of the Water System, the five core components of asset management were evaluated in detail:

- 1. What is the current state of the Water System's assets?
- 2. What is the required level of service?
- 3. Which assets are critical for performance?
- 4. What are the best Capital Improvement Project (CIP) and Operation and maintenance (O&M) Strategies (Life-cycle Costs) for the Water System?
- 5. What is the City's best long-term financing strategy for the Water System?

Based on the Water System data assessed in this study, it can be concluded that due to the age and condition of many of the system assets, the City needs to dramatically increase CIP spending levels on replacement/rehabilitation of Water System assets to reduce the future risk of the overall system and maintain the desired level of service. This AMP recommends a 10-Year CIP that, when implemented, will significantly reduce the overall risk of the system's assets and will conserve water and energy by replacing leaking distribution waterline pipes.

To achieve an overall acceptable level of risk in the Water System while maintaining level of service, Smith recommends a total CIP budget of approximately \$25.6 million to be implemented over the scheduled 10-year timeline as defined in TABLE 23. This total estimated amount of CIP spending is what is considered necessary to allow the City to proactively manage the replacement and repair of the aging Water System assets. Each year of recommended 10-year CIP budget should be considered by the City as Annual Replacement Funds, which are to be a new element of the Water System Annual Operation Budget in addition to the existing budget components currently used by the City to operate the system. In addition, each year of the 10-Year CIP should be considered a separate Phase and a milestone for measuring progress. Another measure of progress should be the annual accounting of the decrease in Water System water losses as each year of the CIP will replace more and more of the leaking pipes in the system. The beginning of every FY of the CIP should be planned to be a set time to review and update funding strategies, such as service rate increases and acquisition of external funding sources.

To implement the recommended 10-year CIP, it is clear the City will need to generate large amounts of revenue. To acquire the necessary revenue for the 10-year CIP, the City needs to consider using multiple revenue sources, both internal and external. Internally, the City needs to raise Water System service rates dramatically to generate more revenue from the Water System consumers. Smith recommends that the existing Water System service rates, at all base and usage levels, be increased by 50% initially followed by 10% annual increases for six years then followed by 8.0% annual increases for three years. These recommended rate increases are based on the projected requirements of the CIP Annual Replacement Fund, taking into account anticipate funding from loans averaging



approximately \$1.7 million annually for those same years (see TABLE 25A). Externally, the City should pursue multiple outside funding sources. The City could apply for funds from programs such as the Rural Infrastructure Program (RIP) through the New Mexico Environmental Department (NMED) Construction Programs Bureau (CPB), the New Mexico Finance Authority (NMFA) Water Trust Board, the NMFA Colonias Program, the Drinking Water State Revolving Fund, and the United States Department of Agriculture (USDA) Rural Development (RD) Colonias Program.

ADDITIONAL RECOMMENDATIONS

The ultimate goal of the Water System AMP is to reduce the risk associated with the system and maximize the resources of the City. Management of the existing Water System is especially challenging due to a high percent of the system being old and beyond its theoretical useful life. The following items are recommendations on how to improve the City's management of the Water System assets.

WATER SYSTEM STAFFING

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The City should consider the Water System operational staff when planning for the future. If the City is to keep up with the demands of the large number of aging Water System assets, it will likely need to continue to increase the Water System staff. Based on the anticipated levels of maintenance and replacement that the assets of the Water System will require, Smith recommends that the City consider hiring at least one additional staff member.

WATER SYSTEM SERVICE RATE STUDY

As discussed throughout the report, the City will need to raise water utility rates periodically as part of managing the Water System assets. Keeping the service rates of the Water System at adequate levels will provide much needed revenue to support the Annual Replacement Fund. In addition to the rate analysis done in this AMP, Smith recommends that a rate study be done by another entity to help establish the appropriate periodic water rate increases required to manage the Water System properly in the future.

WATER SYSTEM DATA

Any plan is only as good as the data used to create it. The City should dedicate resources to improve record keeping procedures in regards to tracking details of repair or replacement work done on components of the Water System. Creation of this AMP report included the development of the Water System GIS Model, which can now serve as a digital database of information on all the assets of the system. As previously mentioned, future repairs and other improvements made to the system should be incorporated in the GIS Model by updating the information on the appropriate assets. There needs to be the development of an official procedure for coordination between the Water System staff and the Accounting Department for how future asset cost data is collected, recorded, and



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updated, by Work Order (Purchase Order). As part of developing this procedure, the City should track future capital improvement costs of the Water System Annual Replacement Fund separately from the standard operation and maintenance costs of the system. This will allow the City to predict more accurately the capital improvement spending for the Annual Replacement Fund.

MAINTENANCE PRACTICES

Water System asset failures can also be minimized through better maintenance practices. Smith recommends that the City develop a planned maintenance program for all the Water System assets. Unfortunately, the proper maintenance and replacement of aging infrastructure requires large amounts of labor. To implement the planned maintenance program, as well as to assist with the replacement of assets, Smith recommends the City consider hiring another staff member for the Water System.

Waterlines

Generally, the major damage from waterline breaks comes from the leaking water. By minimizing the time between the break and the waterline shut-off, the damage can be reduced. Well-placed, mapped and operable valves are the best way to improve water system shut-offs. The Water System GIS Model created with this AMP has the existing valve locations mapped. However, the City should consider implementing a waterline valve exercising and/or location verification program. This program could save the City money through lessening the damage caused by breaks, extending the life of the valves, and possibly reducing system head losses by discovering valves left closed. In addition, improvements to waterline leak-detection methods and practices can help prevent severe waterline failures and the high repair costs associated with them.

Water System Equipment

Smith recommends that the City employ maintenance scheduling for the various equipment components of the Water System based on maintenance information contained in the O&M manuals provided by the manufacturers of the equipment. A "Master Maintenance Schedule" could be assembled by drawing on the information contained in the individual manuals of each important piece of equipment. The new Master Maintenance Schedule could work hand-in-hand with the new Water System GIS Model. Planned maintenance scheduling information could be entered in to the equipment Attribute Tables of the GIS Model to provide guidance to the Water System operators so critical equipment can be maintained properly.



Condition Assessment Practices

Waterlines

As discussed throughout the report, a very important factor in the risk calculation for an asset is the probability of failure of the asset based on condition. Ideally, each waterline pipe scheduled for replacement should have a field inspection of visible appurtenances, and the City should have leak detection performed on the pipe to evaluate the condition. This is especially true for the pipes in the system that are very expensive to replace. These evaluations of condition are cost prohibitive for the entire distribution system, but can be cost effective for small portions of the distribution system selected for replacement or rehabilitation in a given year.

The City should consider purchasing equipment that will allow Water System Staff to perform condition assessment of waterlines themselves. Condition assessment should be done as part of the basic waterline replacement process, especially on pipes that will be expensive to replace. There are new types of condition assessment equipment available that are relatively inexpensive and easy to operate. For example, there is now acoustical testing equipment that uses sound waves to detect leaks and estimate the remaining wall thickness of pipes. This type of equipment could likely be suitable for use by the City in the future waterline replacement program and should be evaluated in detail.

Water System Equipment

Many of the assets of the Water System, such as pumps, motors, control panels and other electrical components, the disinfection system, and buildings/structures can have condition assessment performed on them as part of basic day-to-day operations and maintenance. The Water System Staff already does this by making close visual inspections of equipment and taking note of performance issues. The City should consider using a regimented and organized process for keeping track of issues observed during routine condition assessments. A standardized form could be put together and used to organize information during field assessments of equipment. This information could then be entered into the GIS Model of the Water System for tracking of asset condition over time.

Construction Practices

The best way to prevent future failure and extend the life of an asset is the use of proper products and methods during construction. The City often discovers, especially with waterlines, that the cause of a failure was poor construction. The City should continue to implement quality control procedures during design and construction and continue to evaluate new products (for new installations and rehabilitation) for better quality or more affordability.



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APPENDICES



APPENDIX A

INVENTORY, RISK SCORING, AND ESTIMATED REPLACEMENT
VALUE/CURRENT VALUE OF WATER SYSTEM DISTRIBUTION SYSTEM
(WATERLINES, VALVES, AND HYDRANTS) ASSETS



TOTAL CONTRACTOR OF THE PARTY O		A STATE OF THE PARTY OF THE PAR			מווונטים עבוווו	or water bys	יכווו הופנו והמנוחי	System Assets	Distribution System Assets and Estimated Replacement Value/Current Value	epiacellielli va	Ide/ current	t Value				
GISTO	NCITOIGUSTO	PIPE	MANTEDIAL	YEAR	SHAPE	AGE (in	10000		CONSEQUENCE	CONDITION	RISK		SUM TOTAL	Estimated Asset		Estimated Asset
	and the second	R	R INSTALLED	INSTALLED	LENGTH	2017)	AGE_SCORE	HydrantAge	SCORE	SCORE	SCORE	RISK_LEVEL	PERCENT (LENGTH)	Replacement Value (as of 2015)	ent Value 2015)	Current Value (Based on
941	ASH ST.	9	PVC-SCH40	1980	199.31	37	6.057	1980	11.0	16.057	176.63	EXTREME	0.05%	\$	26,217 \$	10,338
478	E. SEVENTH AVE.	4	AC	1960	330.01	57	7.176	1962	0.6	17.176	154.59	EXTREME	0.14%	₩.	43,410 \$	12,258
409	BROADWAY ST.	12	AC	1935	601.52	82	12.989	1998	11.0	12.989	142.88	EXTREME	0.29%	\$	158,254 \$	158,254
592	DATE ST.	9	AC	1935	140.07	82	12.989	1995	11.0	12.989	142.88	EXTREME	0.32%	\$	18,424 \$	18,424
809	BROADWAY ST.	12	AC	1935	418.23	82	12.989	1998	11.0	12.989	142.88	EXTREME	0.43%	\$	110,031 \$	110,031
613	BROADWAY ST.	12	AC	1935	243.61	82	12.989	1998	11.0	12.989	142.88	EXTREME	0.49%	\$	\$ 060,090	64,090
1009		10	AC	1935	946.33	82	12.989	2005	11.0	12.989	142.88	EXTREME	0.73%	\$	207,472 \$	207,472
346	N. CEDAR ST.	9	PVC-SCH40	1970	538.00	47	9.010	1973	10.0	14.010	140.10	EXTREME	0.87%	\$	\$ 692,07	7,007
817		8	PVC-SCH40	1970	66.94	47	9.010	1971	7.0	19.010	133.07	EXTREME	0.89%	\$	11,740 \$	1,162
883	DATE ST.	∞	PVC-SCH40	1971	129.28	46	8.689	1971	7.0	18.689	130.83	EXTREME	0.92%	\$		2,972
483	BROADWAY ST.	4	AC	1935	470.26	82	12.989	1997	10.0	12.989	129.89	EXTREME	1.04%	\$		61,858
290	BROADWAY ST.	9	AC	1935	217.42	82	12.989	1997	10.0	12.989	129.89	EXTREME	1.10%	*	\$ 009'82	
765	MAIN ST.	9	AC	1935	278.48	82	12.989	1963	10.0	12.989	129.89	EXTREME	1.17%	\$	36,631 \$	
394	COOK ST.	12	PVC-SCH40	1960	168.12	57	12.523	1969	7.0	17.523	122.66	EXTREME	1.21%	\$		
781	BROADWAY ST.	12	PVC-SCH40	1980	370.63	37	6.057	1969	11.0	11.057	121.63	EXTREME	1.31%	\$	\$ 805'26	38,448
727	ASH ST.	9	AC	1935	175.83	82	12.989	1963	0.6	12.989	116.90	EXTREME	1.35%	\$	23,128 \$	
359	DATE ST.	∞	PVC-SCH40	1971	113.64	46	8.689	1971	6.0	18.689	112.14	EXTREME	1.38%	\$	\$ 186,61	2,612
1011		12	PVC-SCH40	1980	590.23	37	6.057	1998	10.0	11.057	110.57	EXTREME	1.53%	*	155,284 \$	61,230
647	PERSHING ST.	∞	O	1947	361.14	70	5.782	1981	7.0	15.782	110.47	EXTREME	1.62%	\$	63,340 \$	26,717
418		12	PVC-SCH40	1991	796.08	56	3.455	1991	13.0	8.455	109.92	EXTREME	1.83%	\$	209,441 \$	137,071
1006		9	AC	1935	275.01	82	12.989	1995	8.0	12.989	103.91	EXTREME	1.90%	\$	36,175 \$	36,175
477	E. SEVENTH AVE.	4	AC	1960	709.20	57	7.176	1962	6.0	17.176	103.06	EXTREME	2.08%	\$	\$ 882,286	26,342
470	N. CEDAR ST.	2	PVC-SCH40	1970	322.28	47	9.010	1999	7.0	14.010	98.07	EXTREME	2.16%	\$	42,393 \$	4,197
981		10	PVC-SCH40	1970	45.90	47	9.010	1971	7.0	14.010	98.07	EXTREME	2.17%	\$	10,063 \$	966
1043		1	PVC-SCH40	1970	108.99	47	9.010	1984	7.0	14.010	98.07	EXTREME	2.20%	\$	14,337 \$	1,419
629	ASH ST.	∞	ō	1947	105.00	70	5.782	1998	9.0	10.782	97.04	EXTREME	2.23%	\$	18,416 \$	7,768
287		9	PVC-SCH40	1971	255.62	46	8.689	1971	7.0	13.689	95.83	EXTREME	2.29%	\$	33,624 \$	4,407
086		10	PVC-SCH40	1971	466.94	46	8.689	1971	7.0	13.689	95.83	EXTREME	2.41%	\$	102,373 \$	13,417
380	BROADWAY ST.	12	PVC-SCH40	1990	149.43	27	3.664	1998	11.0	8.664	95.30	EXTREME	2.45%	\$	39,314 \$	24,910
402	STEEL ST.	12	PVC-SCH40	1990	70.81	27	3.664	1997	11.0	8.664	95.30	EXTREME	2.47%	\$	18,628 \$	11,803
780	BROADWAY ST.	12	PVC-SCH40	1990	483.37	72	3.664	1998	11.0	8.664	95.30	EXTREME	2.59%	\$	\$ 177,171 \$	775'08
403	BROADWAY ST.	12	AC	1960	1135.82	57	7.176	1969	13.0	7.176	93.29	EXTREME	2.88%	\$	\$ 228,862	84,378

最後的ではおおけられること	Current Value	39,429	47,261	18,438	72,905	211,180	14,771	4,187	6,432	2,986	4,299	2,743	4,730	2,037	24,823	18,991	13,772	680'5	20,999	777,72	25,647	26,833	27,504	25,991	113,506	4,828	146,795	59,194	93,360	40,697	49,988	24,004	33,956
	et alue	139.636 \$	167,373 \$	\$ 02,570	115,063 \$	322,679 \$	149,190 \$	42,292 \$	64,967 \$	30,156 \$	43,415 \$	27,702 \$	\$ 692,74	15,544 \$	\$ 806,78	\$ 952,75	48,774 \$	18,023 \$	74,367 \$	65,854 \$	\$ 60,803	63,615 \$	43,408 \$	41,021 \$	179,141 \$	7,620 \$	231,681 \$	59,194 \$	\$ 098'86	40,697 \$	49,988 \$	24,004 \$	33 056 ¢
一年 一日	Estima Replacer	\$. \$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	٠	\$	\$	\$	\$	\$	\$	\$	ų
出版を あるいかい はんしゅん はいまま ない	PERCENT	3.02%	3.18%	3.20%	3.31%	3.78%	4.07%	4.15%	4.28%	4.34%	4.42%	4.48%	4.57%	4.60%	4.77%	4.83%	4.88%	4.90%	4.97%	2.07%	5.16%	5.25%	5.33%	5.41%	2.59%	2.59%	2.82%	5.93%	6.11%	6.19%	6.29%	6.34%	/000/
Control of	RISK_1EVEL	FXTRFMF	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	EXTREME	CVTDFAAF
AND DESCRIPTION OF THE PERSON NAMED IN	RISK	93.29	93.29	88.80	86.64	84.55	84.06	84.06	84.06	84.06	84.06	84.06	84.06	82.14	78.94	78.94	78.94	78.94	78.94	78.91	78.91	78.91	77.98	77.98	77.98	77.98	77.98	77.93	77.93	77.93	77.93	77.93	11.00
THE RESIDENCE OF THE PERSONS ASSESSED.	CONDITION	7 176	7 176	6.831	8.664	8.455	14.010	14.010	14.010	14.010	14.010	14.010	14.010	13.689	7.176	7.176	7.176	7.176	7.176	15.782	15.782	15.782	8.664	8.664	8.664	8.664	8.664	12.989	12.989	12.989	12.989	12.989	22,000
and Education	CONSEQUENCE	13.0	13.0	13.0	10.0	10.0	6.0	6.0	6.0	0.9	6.0	6.0	6.0	0.9	11.0	11.0	11.0	11.0	11.0	5.0	5.0	5.0	9.0	9.0	0.6	9.0	9.0	6.0	6.0	6.0	6.0	6.0	
al a	HydrantAge	1969	1969	1969	1991	1991	1976	1978	2013	1984	1977	1979	1978	1971	1984	1969	1969	1969	1969	1981	1970	1970	1972	1963	1994	1994	1994	1993	1998	1999	1998	1995	0000
manage ma	AGE_SCORE	7 176	7 176	1.831	3.664	3.455	9.010	9.010	9.010	9.010	9.010	9.010	9.010	8.689	7.176	7.176	7.176	7.176	7.176	5.782	5.782	5.782	3.664	3.664	3.664	3.664	3.664	12.989	12.989	12.989	12.989	12.989	
מו ממנכו בלבו	AGE (in 2017)	5.7	57	17	27	26	47	47	47	47	47	47	47	46	57	57	57	57	57	70	70	70	7.7	7.7	27	7.7	7.7	82	82	82	82	82	
NISK SCOLING	SHAPE	520.75	636.18	85.79	437.35	1839.78	1134.18	321.52	493.90	229.25	330.05	210.59	363.15	118.17	668.30	255.64	185.39	68.51	282.67	375.47	346.67	362.71	330.00	311.85	680.91	28.96	880.61	450.00	709.74	309.39	380.02	182.48	
	YEAR	1060	1960	2000	1990	1991	1970	1970	1970	1970	1970	1970	1970	1971	1960	1960	1960	1960	1960	1947	1947	1947	1990	1990	1990	1990	1990	1935	1935	1935	1935	1935	
	MATERIAL	Ų,	2 4	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	AC	AC	AC	AC	AC	C	CI	ō	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	AC	AC	AC	AC	AC	
	PIPE DIAMETE	. 2	1, 1,	17	12	00	4	4	9	9	9	4	4	9	9	12	12	12	12	∞	∞	00	4	9	12	12	12	4	4	4	9	9	
	DESCRIPTION	TO WALKING A Odd	BROADWAY ST.	BROADWAY ST			ARROWHEAD RD.	OAK ST.	CHERRY LN.	TIN ST.	VEATER ST.	E. SECOND AVE.			PLATINUM ST.	BROADWAY ST.	BROADWAY ST.	BROADWAY ST.	BROADWAY ST.	PERSHING ST.	PERSHING ST.	PERSHING ST.	N. CHARLIES LN.	JOFFRE ST.	W. NINTH AVE.	W. NINTH AVE.		AUSTIN AVE.	MARR AVE.	CLANCY ST.	RIVERSIDE DR.		
	GIS ID	401	403	404	420	412			199	260	448	709	858	962	263	396	408	453	784	646	648	671	134	202	389	390	391	484	491	496	561	563	

A STATE OF THE PARTY OF THE PAR	を通じます というできる ののできる ののできる	Charles of the course			KISK Scoring of Water Sy		em Distribution	System Assets	istem Distribution System Assets and Estimated Replacement Value/Current Value	placement Va.	lue/Current	Value				
CI SIS	DESCRIPTION	PIPE	MATCOLA	YEAR	SHAPE	AGE (in			CONSEQUENCE	CONDITION	RISK		SUM TOTAL	Estimated Asset		Estimated Asset
		R	WALEDIAL.	NSTALLED	LENGTH	(2017)	AGE_SCURE	HydrantAge	SCORE	~	SCORE	RISK_LEVEL	PERCENT (I FNGTH)	Replacement Value		Current Value
628	PERSHING ST.	9	AC	1935	403.43	82	12.989	1995	0.9	12.989	77.93	EXTREME	6.49%	\$ 53	53,067 \$	53.067
673	CHARLES AVE.	4	AC	1935	450.40	82	12.989	1978	6.0	12.989	77.93	EXTREME	6.60%			59 245
793	VAN PATTEN AVE	∞	AC	1935	449.03	82	12.989	1999	0.9	12.989	77.93	EXTREME	6.72%			78.755
794	MARR AVE.	4	AC	1935	903.07	82	12.989	1993	0.9	12.989	77.93	EXTREME	6.95%	1		118,790
832	BIRCH ST.	4	AC	1935	325.25	82	12.989	1991	0.9	12.989	77.93	EXTREME	7.03%			42.784
841	BIRCH ST.	4	AC	1935	366.37	82	12.989	1981	0.9	12.989	77.93	EXTREME	7.13%			48.192
1003		9	AC	1935	1459.27	82	12.989	1984-1998	6.0	12.989	77.93	EXTREME	7.50%			191.953
1007		9	AC	1935	283.49	82	12.989	1995	0.9	12.989	77.93	EXTREME	7.57%			37.290
11	SILVER ST.	4	PVC-SCH40	1980	1115.30	37	6.057	1983	7.0	11.057	77.40	EXTREME	7.86%			57.848
369		∞	PVC-SCH40	1980	30.44	37	6.057	1971	7.0	11.057	77.40	EXTREME	7.86%			2.105
917	ARENA DR.	9	PVC-SCH40	1994	486.92	23	2.864	1993	6.0	12.864	77.18	EXTREME	7.99%			45.709
1118	<null></null>	9	ō	1958	201.66	59	4.549	<inn>></inn>	8.0	9.549	76.39	EXTREME	8.04%			14.460
398	BROADWAY ST.	12	PVC-SCH40	2000	104.48	17	1.831	1969	11.0	6.831	75.14	EXTREME	8.07%			22,454
399	RADIUM ST.	12	PVC-SCH40	2000	35.82	17	1.831	1969	11.0	6.831	75.14	EXTREME	8.07%		9,425 \$	7,699
406	BROADWAY ST.	12	PVC-SCH40	2000	181.90	17	1.831	1969	11.0	6.831	75.14	EXTREME	8.12%			39.094
407	BROADWAY ST.	12	PVC-SCH40	2000	225.38	17	1.831	1969	11.0	6.831	75.14	EXTREME	8.18%			48.439
783	BROADWAY ST.	12	PVC-SCH40	2000	362.05	17	1.831	1969	11.0	6.831	75.14	EXTREME	8.27%			77,813
785	COOK ST.	12	PVC-SCH40	2000	361.92	17	1.831	1969	11.0	6.831	75.14	EXTREME	8.36%	\$ 95	95,217 \$	77,783
1014		12	PVC-SCH40	2000	5925.73	17	1.831	2005	11.0	6.831	75.14	EXTREME	888.6	1	\$ 200,655,	1,273,564
833	E. SIXTH AVE.	9	ū	1970	334.34	47	3.342	1991	0.6	8.342	75.08	EXTREME	896.6	\$ 43	43,979 \$	29,283
986	RIVERSIDE DR.	9	PVC-SCH40	1992	832.50	25	3.253	1991	9.0	8.253	74.27	EXTREME	10.17%		\$ 709,501	73,890
1130	<null></null>	∞	ū	1991	340.38	26	1.576	<null></null>	11.0	6.576	72.33	EXTREME	10.26%	\$ 59	\$ 669'65	50,293
350	W. FOURTH AVE.	8	PVC-SCH40	1960	38.10	57	12.523	1998	4.0	17.523	70.09	EXTREME	10.27%		\$ 689'9	6,683
381	W. SECOND AVE.	12	PVC-SCH40	1990	567.80	27	3.664	1984	8.0	8.664	69.31	EXTREME	10.42%	\$ 149	149,383 \$	94,651
629	PERSHING ST.	9	ū	1970	292.03	47	3.342	1995	5.0	13.342	66.71	EXTREME	10.49%	\$ 38	38,414 \$	25,577
17	MAPLE ST.	4	PVC-SCH40	1980	369.09	37	6.057	1981	6.0	11.057	66.34	EXTREME	10.58%			19,144
111	MAGNOLIA ST.	4	PVC-SCH40	1980	601.29	37	6.057	1995	6.0	11.057	66.34	EXTREME	10.74%		\$ 860'62	31,187
138	OAK ST.	4	PVC-SCH40	1980	312.55	37	6.057	1986	6.0	11.057	66.34	EXTREME	10.82%	\$ 41	41,113 \$	16,211
179	MAGNOLIA ST.	4	PVC-SCH40	1980	296.14	37	6.057	1995	6.0	11.057	66.34	EXTREME	10.89%		38,955 \$	15,360
251	CORONA AVE.	9	PVC-SCH40	1980	124.32	37	6.057	2011	6.0	11.057	66.34	EXTREME	10.92%		16,353 \$	6,448
766		2	PVC-SCH40	1980	249.20	37	6.057	1995	6.0	11.057	66.34	EXTREME	10.99%		32,779 \$	12,925
1115		9	IJ	1945	76.63	72	6.019	<null></null>	6.0	11.019	66.12	EXTREME	11.01%	\$ 10	10,080 \$	4,013

Demontry 17. A	GIS ID	DESCRIPTION	PIPE DIAMETE	PIPE DIAMETE MATERIAL	YEAR	SHAPE	AGE (in 2017)	AGE_SCORE	HydrantAge	CONSEQUENCE	CONDITION SCORE	RISK	RISK_LEVEL	SUM TOTAL PERCENT (LENGTH)	Estimated Asset Replacement Value (as of 2015)		Estimated Asset Current Value (Based on
DANNELYST. 4 A CC 1355 315.00 82 12.2890 1937 5.0 12.2890 64.94 CKTRENE 113.34 \$ BRODOWNYST. 6 AC 1355 412.80 82 12.2890 1937 5.0 12.2890 64.94 CKTRENE 114.44K \$ BRODOWNYST. 6 AC 1355 412.80 82 12.2890 1937 5.0 12.2890 64.94 CKTRENE 11.55W \$ BRODOWNYST. 6 AC 1355 412.80 82 12.2890 1937 5.0 12.2890 64.94 CKTRENE 11.55W \$ BRODOWNYST. 6 AC 1355 34.52 82 12.2890 1937 5.0 12.2890 64.94 CKTRENE 11.55W \$ BRODOWNYST. 6 AC 1355 34.52 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 11.55W \$ BRODOWNYST. 6 AC 1355 34.52 82 12.2890 1937 5.0 12.2890 64.94 CKTRENE 11.55W \$ BRODOWNYST. 6 AC 1355 45.00 82 12.2890 1937 5.0 12.2890 64.94 CKTRENE 11.56W \$ BRODOWNYST. 6 AC 1355 45.00 82 12.2890 1937 5.0 12.2890 64.94 CKTRENE 11.56W \$ BRODOWNYST. 6 AC 1355 45.00 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 11.56W \$ BRODOWNYST. 6 AC 1355 45.00 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 11.56W \$ BRODOWNYST. 6 AC 1355 45.00 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 11.26W \$ BRODOWNYST. 6 AC 1355 45.00 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 6 AC 1355 28.30 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 6 AC 1355 28.30 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 6 AC 1355 28.30 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 6 AC 1355 28.30 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 6 AC 1355 28.30 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 6 AC 1355 28.30 82 12.2890 1939 5.0 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 6 AC 1355 28.30 92 12.2890 1939 5.0 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 6 AC 1355 28.30 92 12.2890 1939 5.0 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 6 AC 1355 28.30 92 12.2890 1939 64.94 CKTRENE 12.25W \$ BRODOWNYST. 7 A AC 1355 28.30 92 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 8 AC 1355 28.30 92 12.2890 1939 64.94 CKTRENE 12.25W \$ BRODOWNYST. 8 AC 1355 28.30 92 12.2890 1939 64.94 CKTRENE 12.25W \$ BRODOWNYST. 1 A AC 1355 28.30 92 12.2890 64.94 CKTRENE 12.25W \$ BRODOWNYST. 1 A AC 1355 28	464	BROADWAY ST.	4	AC	1935	874.14	82	12.989	1997	5.0	12.989	64.94	EXTREME	11.23%	\$ 114,985	\$ 5	114,985
BOODOWNYST 6	485	DANIEL ST.	4	AC	1935	315.00	82	12.989	1997	5.0	12.989	64.94	EXTREME	11.31%	\$ 41,435	5 \$	41,435
Handle Coloment	510	BROADWAY ST.	4	AC	1935	429.84	82	12.989	1997	5.0	12.989	64.94	EXTREME	11.42%	\$ 56,541	1. \$	56,541
BRODADWAYST, 6 AC 1935 418 80 82 12,989 1997 50 12,989 64-94 CKTREME 1155% 5 BRODADWAYST, 6 AC 1335 46-34 82 12,989 150 12,989 64-34 CKTREME 1155% 5 AGAST, 7 6 AC 1395 36,75 82 12,989 50 12,989 64-34 CKTREME 1166% 5 MANISST, 6 AC 1395 43,02 12,989 16-39 CKTREME 1166% 5 BRODADWAYST, 6 AC 1395 43,00 82 12,989 1997 50 12,989 64-34 CKTREME 1176% 5 BRODADWAYST, 6 AC 1935 43,116 82 12,989 1997 50 12,989 64-34 CKTREME 1176% 5 BRODADWAYST, 6 AC 1935 21,989 1997 50 12,989 64-34 CKTREME 11,166% 5	518	BROADWAY ST.	9	AC	1935	71.37	82	12.989	1997	5.0	12.989	64.94	EXTREME	11.44%	\$ 9,388	\$ \$	9,388
AMAINST. 6 AC 1935 46.94 EVIRENE 11.56% S ASHIST. 6 AC 1935 347.35 82 12.989 2009 12.989 64.94 EVIRENE 11.66% S MANIST. 6 AC 1935 347.35 82 12.989 1997 5.0 12.989 64.94 EVIRENE 11.10% S BROADWAYST 6 AC 1935 451.20 82 12.989 1997 5.0 12.989 64.94 EVIRENE 11.10% S BROADWAYST 6 AC 1935 495.0 12.989 1997 5.0 12.989 64.94 EVIRENE 11.10% S BROADWAYST 6 AC 1935 37.116 82 12.989 1991 5.0 12.989 64.94 EVIRENE 11.10% S BROADWAYST 6 AC 1938 37.116 82 12.989 1991 5.0 12.9	531	BROADWAY ST.	9	AC	1935	418.80	82	12.989	1997	5.0	12.989	64.94	EXTREME	11.55%	\$ 55,089	\$ 6	55,089
AMHST. 6 AC 1935 347.25 22 12.989 2009 5.0 12.989 64.34 EXTRENDE 11165% 5 POCHST. 6 AC 1935 34.72 22 12.989 1299 64.34 EXTRENDE 11.06% 5 BROADWAYST. 6 AC 1935 45.00 82 12.989 1997 5.0 12.989 64.34 EXTRENDE 11.09% 5 BROADWAYST. 6 AC 1935 450.00 82 12.989 1997 5.0 12.989 64.34 EXTRENDE 11.02% 5 BROADWAYST. 6 AC 1935 37.116 82 12.989 1999 5.0 12.989 64.34 EXTRENDE 11.29% 5 BROADWAYST. 6 AC 1935 279.33 82 12.989 1999 5.0 12.989 64.34 EXTRENDE 11.29% 5 BROADWAYST. 6 AC	533	BROADWAY ST.	9	AC	1935	46.94	82	12.989	1997	5.0	12.989	64.94	EXTREME	11.56%	\$ 6,175	5 \$	6,175
MANINST. 6 AC 1935 36.57 82 12.989 1963 6.9 CKTREME 11.70% 5 FOCHST. 6 AC 1935 45.0 12.989 1997 5.0 12.989 64.94 EKTREME 11.20% 5 BROADWAYST. 6 AC 1935 450.00 82 12.989 1997 5.0 12.989 64.94 EKTREME 11.20% 5 BROADWAYST. 6 AC 1935 450.00 82 12.989 1991 5.0 12.989 64.94 EKTREME 11.20% 5 BROADWAYST. 6 AC 1935 279.33 82 12.989 1991 5.0 12.989 64.94 EKTREME 12.11% 5 MANINST. 6 AC 1935 279.34 82 12.989 1991 5.0 12.989 64.94 EKTREME 12.11% 5 BROADWAYST. 6 AC 1935 282	541	ASH ST.	9	AC	1935	347.25	82	12.989	2009	5.0	12.989	64.94	EXTREME	11.65%	\$ 45,678	\$ \$	45,678
POCHAST, 6 AC 1935 1913 82 1939 939 64.94 EXTREME 1170% 5 BROADWAYST, 6 AC 1935 453.03 82 12.989 1997 50 12.989 64.94 EXTREME 11.82% 5 BROADWAYST, 6 AC 1935 301.83 82 12.989 1997 50 12.989 64.94 EXTREME 11.97% 5 BROADWAYST, 6 AC 1935 301.83 82 12.989 1991 50 12.989 64.94 EXTREME 11.91% 5 MANINST, 8 AC 1935 324.00 82 12.989 1991 50 12.389 64.94 EXTREME 12.11% 5 MANINST, 6 AC 1935 329.40 82 12.989 1991 50 12.989 64.94 EXTREME 12.24% 5 BROADWAYST, 6 AC 1935	564	MIMS ST.	9	AC	1935	36.57	82	12.989	1963	5.0	12.989	64.94	EXTREME	11.66%	\$ 4,810	\$ 0.	4,810
BROADWAYST. 6 AC 1935 45.30 82 12.989 1997 5.0 12.989 64.94 ENTREME 11.87% 5 BROADWAYST. 6 AC 1935 45.00 82 12.989 1997 5.0 12.989 64.94 ENTREME 11.034% 5 BROADWAYST. 6 AC 1935 485.10 82 12.989 1991 5.0 12.989 64.94 ENTREME 12.01% 5 MANIN ST. 8 AC 1935 236.30 82 12.989 1994 5.0 12.989 64.94 ENTREME 12.03% 5 MANIN ST. 8 AC 1935 23.93.0 82 12.989 1994 5.0 12.989 64.94 ENTREME 12.03% 5 MANIN ST. 8 AC 1935 23.93.0 82 12.989 1994 5.0 12.989 64.94 ENTREME 12.03% 5 MANIN ST.	999	FOCH ST.	9	AC	1935	191.23	82	12.989	1991	5.0	12.989	64.94	EXTREME	11.70%	\$ 25,154	\$ 4	25,154
BROADWAYST. 6 AC 1935 45000 82 12.989 1997 5.0 12.989 64.94 EKTREME 11.94% \$ BROADWAYST. 6 AC 1935 301.83 82 12.989 1997 5.0 12.989 64.94 EKTREME 12.01% \$ MANIN ST. 6 AC 1935 371.16 82 12.989 1991 5.0 12.989 64.94 EKTREME 12.11% \$ MANIN ST. 8 AC 1935 279.33 82 12.989 1991 5.0 12.989 64.94 EKTREME 12.13% \$ MAIN ST. 6 AC 1935 279.33 82 12.989 1991 5.0 12.989 64.94 EKTREME 12.13% \$ BROADWAYST. 6 AC 1935 279.33 82 12.989 1991 5.0 12.989 EKTREME 12.13% \$ BROADWAYST. 6	567	BROADWAY ST.	9	AC	1935	453.03	82	12.989	1997	5.0	12.989	64.94	EXTREME	11.82%	\$ 59,592	\$ 2	29,592
ROCHANIA ST. 6 AC 1935 301.83 62 12.989 1997 SO 12.989 64.94 EKTREME 12.01% S FCCH ST. 6 AC 1935 37.116 82 12.989 1991 50 12.989 64.94 EKTREME 12.11% S MANIN ST. 6 AC 1935 236.30 82 12.989 1991 50 12.989 64.94 EKTREME 12.23% S BROADWLYST. 6 AC 1935 239.40 82 12.989 1991 50 12.989 64.94 EKTREME 12.23% S BROADWLYST. 6 AC 1935 239.50 82 12.989 1991 50 12.989 64.94 EKTREME 12.23% S MANIN ST. 8 AC 1935 239.50 82 12.989 1991 50 12.989 64.94 EKTREME 12.23% S MAIN ST. <td< td=""><td>587</td><td>BROADWAY ST.</td><td>9</td><td>AC</td><td>1935</td><td>450.00</td><td>82</td><td>12.989</td><td>1997</td><td>5.0</td><td>12.989</td><td>64.94</td><td>EXTREME</td><td>11.94%</td><td>\$ 59,193</td><td>3 \$</td><td>59,193</td></td<>	587	BROADWAY ST.	9	AC	1935	450.00	82	12.989	1997	5.0	12.989	64.94	EXTREME	11.94%	\$ 59,193	3 \$	59,193
FOCH ST. 6 AC 1935 371 16 82 12989 50 12989 64-34 EXTREME 1211% \$ MAIN ST. 8 AC 1393 485,10 82 12989 2006 50 12989 64-34 EXTREME 12,23% \$ MAIN ST. 6 AC 1935 226,30 82 12,989 1991 50 12,989 64-34 EXTREME 12,23% \$ BROADWAYST. 6 AC 1935 23940 82 12,989 50 12,989 64-34 EXTREME 12,29% \$ BROADWAYST. 6 AC 1935 211,42 82 12,989 50 12,989 64-34 EXTREME 12,23% \$ MAIN ST. 8 AC 1935 21,248 82 12,989 1991 50 12,989 64-34 EXTREME 12,23% \$ BROADWAYST. 8 AC 1935 17,298	589	BROADWAY ST.	9	AC	1935	301.83	82	12.989	1997	5.0	12.989	64.94	EXTREME	12.01%	\$ 39,703	3 \$	39,703
MAIN ST. 6 AC 1935 485.10 82 12.989 2006 5.0 12.989 64.94 EKTREME 12.23% 5 MAIN ST. 6 AC 1935 236.30 82 12.989 1994 5.0 12.989 64.94 EKTREME 12.29% 5 BROADWAYST. 6 AC 1935 239.340 82 12.989 1991 5.0 12.989 64.94 EKTREME 12.29% 5 BROADWAYST. 6 AC 1935 219.50 82 12.989 1991 5.0 12.989 64.94 EKTREME 12.24% 5 MANINST. 8 AC 1935 112.96 82 12.989 1991 5.0 12.989 64.94 EKTREME 12.24% 5 BROADWAYST. 8 AC 1935 112.989 1991 5.0 12.989 64.94 EKTREME 12.23% 5 MAINST. 8 AC	591	FOCH ST.	9	AC	1935	371.16	82	12.989	1991	5.0	12.989	64.94	EXTREME	12.11%	\$ 48,823	3 \$	48,823
MAIN ST. 6 AC 1935 236.30 82 12.989 1984 S.O 12.989 64.94 EXTREME 12.29% S MAIN ST. 6 AC 1935 279.33 82 12.989 1991 SO 12.989 64.94 EXTREME 12.36% \$ BROADWAY ST. 6 AC 1935 2234.00 82 12.989 1999 SO 12.989 64.94 EXTREME 12.36% \$ MAIN ST. 8 AC 1935 21.142 82 12.989 1991 SO 12.989 64.94 EXTREME 12.44% \$ MAIN ST. 8 AC 1935 312.95 82 12.989 1994 SO 12.989 64.94 EXTREME 12.28% \$ MAIN ST. 6 AC 1935 350.59 82 12.989 1994 SO 12.989 64.94 EXTREME 12.24% \$ RIVERSIDE DR.	595	MAIN ST.	00	AC	1935	485.10	82	12.989	2006	5.0	12.989	64.94	EXTREME	12.23%	\$ 85,081	31 \$	85,081
MAIN ST. 6 AC 1935 27933 82 12989 1991 5.0 12.989 64.94 EXTREME 12.36% \$ BROADWAYST. 6 AC 1935 293.40 82 12.989 1999 5.0 12.989 64.94 EXTREME 12.44% \$ BROADWAYST. 4 AC 1935 329.50 82 12.989 1991 5.0 12.989 64.94 EXTREME 12.52% \$ MAIN ST. 8 AC 1935 54.862 82 12.989 1991 5.0 12.989 64.94 EXTREME 12.52% \$ BROADWAYST. 6 AC 1935 54.862 82 12.989 1994 5.0 12.989 64.94 EXTREME 12.53% \$ BROADWAYST. 6 AC 1935 335.22 82 12.989 1994 5.0 12.989 64.94 EXTREME 12.53% \$ RIVERISION ON ST. <td>599</td> <td>MAIN ST.</td> <td>9</td> <td>AC</td> <td>1935</td> <td>236.30</td> <td>82</td> <td>12.989</td> <td>1984</td> <td>5.0</td> <td>12.989</td> <td>64.94</td> <td>EXTREME</td> <td>12.29%</td> <td>\$ 31,083</td> <td>33 \$</td> <td>31,083</td>	599	MAIN ST.	9	AC	1935	236.30	82	12.989	1984	5.0	12.989	64.94	EXTREME	12.29%	\$ 31,083	33 \$	31,083
BROADWAYST. 6 AC 1935 29340 82 12989 150 12.989 64.94 EXTREME 12.44% \$ BROADWAYST. 4 AC 1935 329.50 82 12.989 1999 5.0 12.989 64.94 EXTREME 12.52% \$ MANIN ST. 8 AC 1935 111.42 82 12.989 1991 5.0 12.989 64.94 EXTREME 12.55% \$ BROADWAYST. 6 AC 1935 548.62 82 12.989 1997 5.0 12.989 64.94 EXTREME 12.55% \$ BROADWAYST. 6 AC 1935 352.22 82 12.989 1994 5.0 12.989 64.94 EXTREME 12.55% \$ RROADWAYST. 6 AC 1935 335.22 82 12.989 1994 EXTREME 12.55% \$ RIVERSIDE DR. AC 1935 3350.59 82	609	MAIN ST.	9	AC	1935	279.33	82	12.989	1991	5.0	12.989	64.94	EXTREME	12.36%	\$ 36,743	13 \$	36,743
BRODAWAY ST. 4 AC 1935 329.50 82 12.989 1999 50 12.989 64.94 EXTREME 12.52% \$ MAIN ST. 8 AC 1935 111.42 82 12.989 1991 50 12.989 64.94 EXTREME 12.55% \$ MAIN ST. 8 AC 1935 548.62 82 12.989 2006 5.0 12.989 64.94 EXTREME 12.59% \$ BROADWAY ST. 6 AC 1935 357.28 82 12.989 1994 5.0 12.989 64.94 EXTREME 12.59% \$ RROADWAY ST. 6 AC 1935 357.88 82 12.989 1998 5.0 12.989 64.94 EXTREME 12.59% \$ RIVERSIDE DR AC 1935 350.59 82 12.989 1998 5.0 12.989 64.94 EXTREME 12.59% \$ ASH ST. AC	766	BROADWAY ST.	9	AC	1935	293.40	82	12.989	1999	5.0	12.989	64.94	EXTREME	12.44%	\$ 38,594	34 \$	38,594
MANIN ST. 8 AC 1935 11142 82 12,989 1991 5.0 12,989 64.94 EXTREME 12,55% 5 MANIN ST. 8 AC 1935 548.62 82 12,989 2006 5.0 12,989 64.94 EXTREME 12,69% 5 BROADWAY ST. 6 AC 1935 375.22 82 12,989 1997 5.0 12,989 64.94 EXTREME 12,73% 5 MANINTHANE ST. 6 AC 1935 335.22 82 12,989 1998 5.0 12,989 64.94 EXTREME 12,91% 5 RIVERSIDE DR. 6 AC 1935 350.59 82 12,989 1997 5.0 12,989 64.94 EXTREME 12,91% 5 ASH ST. 4 AC 1935 350.12 82 12,989 1997 5.0 12,989 64.94 EXTREME 12,91% 5 W. NINTHANE	767	BROADWAY ST	4	AC	1935	329.50	82	12.989	1999	5.0	12.989	64.94	EXTREME	12.52%	\$ 43,342	\$ 21	43,342
MANIN ST. 8 AC 1935 548.62 82 12.989 2006 5.0 12.989 64.94 EXTREME 12.69% \$ BROADWAYST. 6 AC 1935 172.96 82 12.989 1997 5.0 12.989 64.94 EXTREME 12.73% \$ MAIN ST. 6 AC 1935 172.96 82 12.989 1994 5.0 12.989 64.94 EXTREME 12.73% \$ RIVERSIDE DR. 6 AC 1935 357.88 82 12.989 5.0 12.989 64.94 EXTREME 12.91% \$ ASH ST. 4 AC 1935 350.59 82 12.989 5.0 12.989 64.94 EXTREME 12.91% \$ ASH ST. AC 1935 431.45 82 12.989 5.0 12.989 64.94 EXTREME 13.18% \$ W. NINTH AVE 8 AC 1937 7.716	776	MAIN ST	000	AC	1935	111.42	82	12.989	1991	5.0	12.989	64.94	EXTREME	12.55%	\$ 19,543	t3 \$	19,543
BROADWAYST. 6 AC 1935 172.96 82 12.989 1997 5.0 12.989 64.94 EXTREME 12.73% \$ MAIN ST. 6 AC 1935 335.22 82 12.989 1984 5.0 12.989 64.94 EXTREME 12.82% \$ RIVERSIDE DR. 6 AC 1935 357.88 82 12.989 5.0 12.989 64.94 EXTREME 12.91% \$ ASH ST. 4 AC 1935 350.59 82 12.989 5.0 12.989 64.94 EXTREME 12.91% \$ ASH ST. 4 AC 1935 700.12 82 12.989 5.0 12.989 64.94 EXTREME 13.13% \$ ASH ST. 4 AC 1935 431.45 82 12.989 5.0 12.989 64.94 EXTREME 13.13% \$ W. NINTH AVE 8 CI 1947 37.10	777	MAIN ST.	0	AC	1935	548.62	82	12.989	2006	5.0	12.989	64.94	EXTREME	12.69%	\$ 96,222	22 \$	96,222
MAIN ST. 6 AC 1935 335.22 82 12.989 1984 5.0 12.989 64.94 EXTREME 12.91% 5 RIVERSIDE DR. 6 AC 1935 357.88 82 12.989 1998 5.0 12.989 64.94 EXTREME 12.91% 5 ASH ST. 4 AC 1935 350.59 82 12.989 2009 5.0 12.989 64.94 EXTREME 13.00% 5 ASH ST. 4 AC 1935 700.12 82 12.989 2009 5.0 12.989 64.94 EXTREME 13.00% 5 W. NINTH AVE. 8 AC 1947 351.16 70 5.782 2004 6.0 10.782 64.69 EXTREME 13.32% 5 COOK ST. 12 AC 1947 371.01 70 5.782 2004 6.0 10.782 64.59 EXTREME 13.47% 5 COOK ST. <t< td=""><td>779</td><td>BROADWAY ST.</td><td>9</td><td>AC</td><td>1935</td><td>172.96</td><td>82</td><td>12.989</td><td>1997</td><td>5.0</td><td>12.989</td><td>64.94</td><td>EXTREME</td><td>12.73%</td><td>\$ 22,752</td><td>52 \$</td><td>22,752</td></t<>	779	BROADWAY ST.	9	AC	1935	172.96	82	12.989	1997	5.0	12.989	64.94	EXTREME	12.73%	\$ 22,752	52 \$	22,752
RIVERSIDE DR. 6 AC 1935 357.88 82 12.989 1998 5.0 12.989 64.94 EXTREME 12.91% \$ ASH ST. 4 AC 1935 350.59 82 12.989 2009 5.0 12.989 64.94 EXTREME 13.00% \$ W. NINTH AVE. 8 C1 1947 351.16 70 5.782 2004 6.0 10.782 64.69 EXTREME 13.29% \$ W. NINTH AVE. 8 C1 1947 371.01 70 5.782 2004 6.0 10.782 64.69 EXTREME 13.47% \$ COOK ST. 12 AC 1947 371.01 70 5.782 2004 6.0 7.176 64.59 EXTREME 13.47% \$ COOK ST. 12 AC 1960 612.06 5.0 7.176 64.59 EXTREME 13.47% \$ 13.47% \$ COOK ST. 12 <	797	MAIN ST.	9	AC	1935	335.22	82	12.989	1984	5.0	12.989	64.94	EXTREME	12.82%	\$ 44,095	\$ \$	44,095
ASH ST. 4 AC 1935 350.59 82 12.989 2009 5.0 12.989 64.94 EXTREME 13.09% \$ 4 AC 1935 700.12 82 12.989 1997 5.0 12.989 64.94 EXTREME 13.18% \$ 8 AC 1935 431.45 82 12.989 2006 5.0 12.989 64.94 EXTREME 13.29% \$ W. NINTH AVE. 8 CI 1947 351.16 70 5.782 2004 6.0 10.782 64.69 EXTREME 13.47% \$ COOK ST. 12 AC 1960 612.06 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.64% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.64% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.64% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.64% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.64% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.64% \$ COOK ST. 12 AC 1960 57 7.176 196	818	RIVERSIDE DR.	9	AC	1935	357.88	82	12.989	1998	5.0	12.989	64.94	EXTREME	12.91%	\$ 47,075	75 \$	47,075
4 AC 1935 700.12 82 12.989 1997 5.0 12.989 64.94 EXTREME 13.18% \$ W. NINTH AVE. 8 AC 1935 431.45 82 12.989 2006 5.0 12.989 64.94 EXTREME 13.29% \$ W. NINTH AVE. 8 CI 1947 351.16 70 5.782 2004 6.0 10.782 64.69 EXTREME 13.38% \$ COOK ST. 12 AC 1960 193.75 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.47% \$ COOK ST. 12 AC 1960 612.06 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 61.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ AC 1960 61.56	839	ASH ST.	4	AC	1935	350.59	82	12.989	2009	5.0	12.989	64.94	EXTREME	13.00%	\$ 46,116	\$ 91	46,116
8 AC 1935 431.45 82 12.989 2006 5.0 12.989 64.94 EXTREME 13.29% \$ W. NINTH AVE. 8 CI 1947 351.16 70 5.782 2004 6.0 10.782 64.69 EXTREME 13.38% \$ COOK ST. 12 AC 1960 193.75 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.47% \$ COOK ST. 12 AC 1960 612.06 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 612.06 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.82% \$ LOOK ST. 12 AC 1960 61.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.32% \$ LOOK ST. 12 AC	1015		4	AC	1935	700.12	82	12.989	1997	5.0	12.989	64.94	EXTREME	13.18%	\$ 92,094	94 \$	92,094
W. NINTH AVE. 8 CI 1947 351.16 70 5.782 2004 6.0 10.782 64.69 EXTREME 13.47% \$ COOK ST. 12 AC 1960 193.75 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.52% \$ COOK ST. 12 AC 1960 612.06 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ 1 COOK ST. 12 AC 1960 612.06 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ 1 LOOK ST. 12 AC 1960 612.06 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.82% \$ 1	1016		00	AC	1935	431.45	82	12.989	2006	5.0	12.989	64.94	EXTREME	13.29%	\$ 75,672	72 \$	75,672
8 CI 1947 371.01 70 5.782 2004 6.0 10.782 64.69 EXTREME 13.47% \$ COOK ST. 12 AC 1960 193.75 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.82% \$ LUNC ST. 12 AC 1960 561.56 57 7.176 1989 9.0 7.176 64.59 EXTREME 13.82% \$ LUNC ST. 12 AC 1960 561.56 57 7.176 1989-1999 9.0 7.176 64.59 EXTREME 13.94% \$	650	W. NINTH AVE.	00	ō	1947	351.16	70	5.782	2004	0.9	10.782	64.69	EXTREME	13.38%	\$ 61,590	\$ 06	25,979
COOK ST. 12 AC 1960 193.75 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.52% \$ COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.82% \$ 13.62% \$	759		00	D	1947	371.01	70	5.782	2004	0.9	10.782	64.69	EXTREME	13.47%	\$ 65,071	71 \$	27,447
COOK ST. 12 AC 1960 612.06 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.68% \$ 1 COOK ST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.82% \$ 1	909	COOK ST.	12	AC	1960	193.75	57	7.176	1969	0.6	7.176	64.59	EXTREME	13.52%	\$ 50,974	74 \$	14,394
COOKST. 12 AC 1960 561.56 57 7.176 1969 9.0 7.176 64.59 EXTREME 13.82% \$ 1	607	COOK ST.	12	AC	1960	612.06	57	7.176	1969	0.6	7.176	64.59	EXTREME	13.68%	\$ 161,026	\$ 97	45,469
1050 1050 1050 1050 1050 1050 1050 1050	717	COOK ST.	12	AC	1960	561.56	57	7.176	1969	0.6	7.176	64.59	EXTREME	13.82%	\$ 147,740		41,717
HYDE SI. 6 AL 1960 446.33	798	HYDE ST.	9	AC	1960	448.99	57	7.176	1998-1999	9.0	7.176	64.59	EXTREME	13.94%	\$ 59,060	\$ 09	16,677

を できる 日本	2	DIDE		KISK SCORING OF WATER		em Distribution	Jystem Distribution Jystem Assets and Estimated Replacement Value/Current Value	nd Estimated Ke	piacement val	ue/current	Value			Section Section 1999	The second second second
CII SIE	DESCRIPTION DIAN	DIAMETE MATERIAL		SHAPE	AGE (in	ACE COOR		CONSEQUENCE	CONDITION	RISK		SUM TOTAL	Estimated Asset		ed Asset
		1	INSTALLED		(7102	AGE_SCORE	пуагалижде	SCORE	SCORE	SCORE	MISK_LEVEL	PERCENT (LENGTH)	Replacement Value (as of 2015)	Current Value (Based on	t Value ed on
918	ARENA DR.	6 PVC-SCH40	1994	112.65	23	2.864	1993	5.0	12.864	64.32	EXTREME	13.97%	\$ 14,818	\$	10,575
55	ALUMINUM ST. 4	4 PVC-SCH40	1990	435.15	7.7	3.664	1995	7.0	8.664	60.65	EXTREME	14.08%	\$ 57,240	*	36,268
248	STEEL ST. (6 PVC-SCH40	1990	504.59	7.7	3.664	1997	7.0	8.664	60.65	EXTREME	14.21%	\$ 66,374	\$	42,055
354	W. NINTH AVE.	8 PVC-SCH40	0661 01	712.41	27	3.664	1994	7.0	8.664	60.65	EXTREME	14.39%	\$ 124,950	\$	79,170
387		12 PVC-SCH40	1990	63.28	7.7	3.664	1995	7.0	8.664	60.65	EXTREME	14.40%	\$ 16,650	\$	10,549
388	₹	12 PVC-SCH40	1990	551.42	27	3.664	1995	7.0	8.664	60.65	EXTREME	14.55%	\$ 145,072	\$	91,919
452	BROADWAY ST. 1	12 PVC-SCH40	1990	154.41	77	3.664	1995	7.0	8.664	60.65	EXTREME	14.58%	\$ 40,624	\$	25,740
950	ASH ST. 6	6 PVC-SCH40	1990	101.32	77	3.664	1995	7.0	8.664	60.65	EXTREME	14.61%	\$ 13,328		8,445
1128	«Null»	8 CI	1991	371.62	26	1.576	<null></null>	9.0	6.576	59.18	EXTREME	14.71%	\$ 65,178		54,909
1129	«Null»	8 CI	1991	527.30	26	1.576	<null></null>	9.0	6.576	59.18	EXTREME	14.84%	\$ 92,484		77,912
837	E. THIRD AVE. 4	t CI	1970	724.86	47	3.342	2009	7.0	8.342	58.39	EXTREME	15.02%	\$ 95,348		63,486
854	W. THIRD AVE.	D 9	1970	30.05	47	3.342	1997	7.0	8.342	58.39	EXTREME	15.03%	\$ 3,953		2,632
1013	14	4 DI	1970	2452.33	47	5.306	1995	11.0	5.306	58.37	EXTREME	15.66%	\$ 752,719	\$	353,304
1114	9 < InN>	5 CI	1958	634.67	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	15.82%	\$ 83,485	\$	45,508
1116	<null></null>	l) 9	1958	841.28	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	16.04%	\$ 110,662	\$	60,323
1117	<null></null>	CI CI	1958	85.97	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	16.06%	\$ 11,308	\$	6,164
1119	<null> 6</null>	5 CI	1958	808.76	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	16.26%	\$ 106,385	\$	57,991
1120	9 <inn></inn>	5 CI	1958	259.60	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	16.33%	\$ 34,148	\$	18,614
1121	9	S CI	1958	723.90	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	16.51%	\$ 95,222	\$	51,906
1122	9	5 CI	1958	67.02	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	16.53%	\$ 8,815	\$	4,805
1123	<null> 6</null>	C C	1958	256.58	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	16.60%	\$ 33,750	s	18,397
1124	<null> 6</null>	D C	1958	349.13	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	16.69%	\$ 45,925	\$	25,034
1125	9 <null></null>	5	1958	674.48	59	4.549	<null></null>	6.0	9.549	57.29	EXTREME	16.86%	\$ 88,722	\$	48,363
130	E. NINTH AVE. 4		.0 1970	360.00	47	9.010	1971	3.0	19.010	57.03	EXTREME	16.95%	\$ 47,355	\$	4,689
430	RIVERSIDE DR. 6	5 PVC-SCH40	.0 1970	9.80	47	9.010	1976	4.0	14.010	56.04	EXTREME	16.95%	\$ 1,289	45	128
78	ORE ST. 4		.0 1970	547.98	47	9.010	2003	4.0	14.010	56.04	EXTREME	17.09%	\$ 72,081	-γ-	7,137
120	W. SIXTH AVE. 4	PVC-SCH40	.0 1970	310.47	47	9.010	1978	4.0	14.010	56.04	EXTREME	17.17%	\$ 40,839	\$	4,043
121	W. SIXTH AVE. 4	PVC-SCH40	.0 1970	37.96	47	9.010	1978	4.0	14.010	56.04	EXTREME	17.18%	\$ 4,993	\$	494
235	VEATER ST. 6	PVC-SCH40	0 1970	190.38	47	9.010	2003	4.0	14.010	56.04	EXTREME	17.23%	\$ 25,042	\$	2,479
236	VEATER ST. 6	PVC-SCH40	0 1970	190.00	47	9.010	2003	4.0	14.010	56.04	EXTREME	17.28%	\$ 24,993	₩.	2,475
243			0 1970	554.00	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.42%	\$ 72,873	\$	7,215
244	VEATER ST. 6	PVC-SCH40	0 1970	150.05	47	9.010	1977	4.0	14.010	56.04	EXTREME	17.46%	\$ 19,738	\$	1,954

CHANTERINGE IN TABLE AND			PIPE		YEAR	SHAPE	AGE (in	Acr conf		CONSEQUENCE	CONDITION	RISK	DICK LEVEL	SUM TOTAL	Estimated Asset	Estimated Asset	The St
WEATERST 6 PROCESSION 47 5000 1877 400 1400 5604 OFFRENKE 17,50% 5 21,053 5 RURISIDE DIR 6 FROCCARIO 1970 410 510	GISID	DESCRIPTION	DIAMETE	MATERIAL	INSTALLED	LENGTH	2017)	AGE_SCURE	нуагаптАве	SCORE		SCORE	MISH_LEVEL	(LENGTH)	(as of 2015)	(Based on	08
PRINCEDIDE IN. C PRINCEDIDE IN. C PRINCEDIDE IN. C CONTRACTOR 579-85 C 19,875 S 43,118 S RIVERDIDE IN. C FVECKHOL 1377 3277.9 47 9100 1376 40 14010 56.04 DYTRAME 17,785 5 43,118 5 RIVERSIDE IN. C FVECKHOL 1370 324.04 47 9100 1376 40 14010 56.04 DYTRAME 17,786 5 42,290 5 RIVERSIDE IN. C FVECKHOL 1370 375 47 9100 1376 40 41010 56.04 DYTRAME 17,788 5 42,290 5 RIVERSIDE IN. C FVECKHOL 1370 375 40 9100 1376 40 400 400 56.04 DYTRAME 17,888 43,230 5 432.95 5 432.95 5 432.95 5 432.95 5 432.95	258	VEATER ST.	9	PVC-SCH40	1970	160.05	47	9.010	1977	4.0	14.010	56.04	EXTREME	17.50%		\$ 2,084	C
MNERSIDE DR. 6 PVCSCHOLD 3573 47 9000 1976 400 4000 5504 CNTREM 17,66% 5 41,210 5 RUNESSIDE DR. 6 PVCSCHOL 1790 21,28 40 400 14010 5504 CNTREME 17,68% 5 41,280 5 RUNESSIDE DR. 6 PVCSCHOL 1370 1356 47 9000 1976 40 4000 5004 CNTREME 17,88% 5 42,28 5 RUNESSIDE DR. 6 PVCSCHOL 1370 4133 47 9000 1376 40 14000 56.04 CNTREME 17,88% 5 42,23 5 RUNESSIDE DR. 6 PVCSCHOL 1370 47 9000 1376 40 1400 56.04 CNTREME 17,88% 5 42,28 5 RUNESSIDE DR. 6 PVCSCHOL 1370 40 9000 1376 40 1400 400	265	RIVERSIDE DR.	9	PVC-SCH40	1970	149.57	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.54%		\$ 1,948	00
PRINTENDE DIR. 6 PRINTENDE DIR. 6 PRINTENDE DIR. 6 PRINTENDE DIR. 6 CATALLA STARIA 4 9 100 130 5504 EVITABLE DIR. 5 0.4230 5 13.25 9 13.25 3 13.25 3 13.25 3 13.25 3 13.25 3 13.25 3 4 9 100 13.06 4 10.00 5504 EVITABLE DIR. 17.38% 5 4.02 5 4 9 10.00 13.06 4 0 10.00 13.06 4 0 10.00 13.06 4 0 10.00 13.06 4 0 10.00 13.06 4 0 10.00 13.00 5 0 10.00 13.00 10.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00 13.00	266	RIVERSIDE DR.	9	PVC-SCH40	1970	327.79	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.62%		\$ 4,269	0
MUNESIDE DIR. 6 PUNCSCHIAD 1970 321.50 47 91010 1976 4.0 1400 56.04 DITTREMIS 7770% 9 1786 9 1400 56.04 DITTREMIS 7770% 9 1786 9 1400 56.04 DITTREMIS 7780% 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4 9 1786 4	267	RIVERSIDE DR.	9	PVC-SCH40	1970	238.04	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.68%		\$ 3,100	0
MUNESIOE DR. 6 PVCSCHAID 1970 135 63 47 9100 1376 40 14010 56.04 DITREMIC 1783W 5 1784B 5 1784B 5 5 MUNESIOE DR. 6 PVCSCHAID 1970 37.65 47 9100 1376 40 14.010 56.04 DITREMIC 17.83W 5 8.24.2 5 RWINSION DR. 6 PVCSCHAID 1970 41.0 9100 1976 4.0 14.010 56.04 DITREMIC 17.83W 5 56.43 5 6.64 5 6.64 DITREMIC 17.83W 5 6.64 5 6 6 7.64 5 6 7.64 5 6 7.64 8 6 6 7.64 8 6 7.64 1.00 5 6 7.64 8 7.64 8 7.64 8 7.64 8 7.64 8 7.64 8 7.64 8 7.64 8 7.64	268	RIVERSIDE DR.	9	PVC-SCH40	1970	321.50	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.76%		\$ 4,187	7
NAMESTORE DR. 6 PVCSCH40 1970 376 470 1400 56.04 ENTREME 1733% 5 4652 5 RIVERSIDE DR. 6 PVCSCH40 1970 6706 470 1400 56.04 ENTREME 1733% 5 88.23 5 RIVERSIDE DR. 6 PVCSCH40 1970 63.74 470 90.00 1976 40 140.00 56.04 ENTREME 17,88% 5 56.95 5 7 56.95 SCREAD 87.05 5 56.04 ENTREME 17,88% 5 56.95 5 6 56.95 5 6 7 56.04 ENTREME 17,88% 5 56.95 5 6 7 5 5 6 7 5 6 7 5 6 7 5 7 5 7 5 7 4 9 10 140.00 56.04 ENTREME 17,88% 5 8,84.5 5 8,	269	RIVERSIDE DR.	9	PVC-SCH40	1970	135.62	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.80%		\$ 1,766	9
NUMERSIDE DR. 6 PVCSCH40 1970 67.06 4.0 14010 56.04 FITEMER 1788% 5 8.8.11 5 RIVERSIDE DR. 6 PVCSCH40 1970 41.31 4.0 140.00 56.04 FITEMER 17.88% 5 56.43 5 RIVERSIDE DR. 6 PVCSCH40 1970 66.37 4.7 9.00 1976 4.0 140.0 56.04 EVTRAME 17.88% 5 56.35 5 56.04 EVTRAME 17.88% 5 56.35 5 56.04 EVTRAME 17.88% 5 56.05 5 56.04 EVTRAME 17.88% 5 56.05 5 56.04 EVTRAME 17.88% 5 56.04 5 56.04 EVTRAME 17.88% 5 56.05 5 56.04 EVTRAME 17.88% 5 56.05 5 56.04 EVTRAME 17.88% 5 56.05 5 56.04 EVTRAME 17.88% 5 15.	270	RIVERSIDE DR.	9	PVC-SCH40	1970	37.65	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.81%		\$ 490	0
RIVERSIDE DR. 6 PVC-SCH40 1970 41.31 47 9010 1976 4.0 14010 56.04 EXTREM 1788% 5.444 <t< td=""><td>271</td><td>RIVERSIDE DR.</td><td>9</td><td>PVC-SCH40</td><td>1970</td><td>90.79</td><td>47</td><td>9.010</td><td>1976</td><td>4.0</td><td>14.010</td><td>56.04</td><td>EXTREME</td><td>17.83%</td><td></td><td>\$ 873</td><td>2</td></t<>	271	RIVERSIDE DR.	9	PVC-SCH40	1970	90.79	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.83%		\$ 873	2
RIVERSIDE DR. 6 PWCSCHAO 1970 38.43 47 9100 1976 40 4100 S604 ENTREM 1788% 5 5.055 5 505 5 40 400 400 400 400 400 400 400 400 400 400 400 56.04 EXTREME 1788% 5 8,055 8,055 8 7 7 40 400 400 56.04 EXTREME 1788% 5 8,055 8 8,055 8 8,055 8 9 9 9 9 100 201 40 40 400 56.04 EXTREME 1788% 5 8,055 8 3,344 5 9 9 9 9 0 100	272	RIVERSIDE DR.	9	PVC-SCH40	1970	41.31	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.84%		\$ 538	∞
MARROLINE GR. 6 PVCSCH40 1970 66.37 47 9010 1976 40 14010 56.04 EXTREME 1787% 5 3,844 5 CHERRYLIA, 6 PVCSCH40 1370 22.18.2 47 9010 2013 4.0 14,010 56.04 EXTREME 17,87% 5 3,844 5 CHERRYLIA, 6 PVCSCH40 1370 20.00.1 47 9010 2013 4.0 14,010 56.04 EXTREME 17,99% 5 3,844 5 VARROLIA STILL 6 PVCSCH40 1970 1970 40 14,010 56.04 EXTREME 17,99% 5 26,209 5 ARROLIA STILL 6 PVCSCH40 1970 1970 1970 40 140 50.0 140 50.0 140 50.0 140 50.0 140 50.0 140 50.0 140 50.0 140 50.0 140 140 140	273	RIVERSIDE DR.	9	PVC-SCH40	1970	38.43	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.85%		\$ 501	1
CHERRY ILL. 6 PVC-SCH4ID 1970 23.22 47 9010 2013 4.0 14010 56.04 EXTREME 17,87% 5 33,125 5 CHERRY ILL. 6 PVC-SCH4ID 1970 2021 371 4.0 14010 56.04 EXTREME 17,93% 5 33,125 5 W. PHERRS ILL. 6 PVC-SCH4ID 1970 2001 197 4.0 14010 56.04 EXTREME 17,93% 5 15,425 5 W. PHERRS ILL. 6 PVC-SCH4ID 1970 2010 1976 4.0 14010 56.04 EXTREME 17,93% 5 15,445 5 WARROWILL ST. 6 PVC-SCH4ID 1970 2010 1976 4.0 14010 56.04 EXTREME 17,93% 5 12,945 5 12,945 5 14,945 5 14,945 5 14,945 5 14,945 5 14,945 5 14,945 5 <	274	RIVERSIDE DR.	9	PVC-SCH40	1970	66.37	47	9.010	1976	4.0	14.010	56.04	EXTREME	17.86%		\$ 864	4
CHERRYLIN, 6 PUCSCHIAIO 1970 2518.2 47 9010 2013 40 14010 SGO4 EKTREME 17,93% 5 33,125 5 VEATER ST. 6 PUCSCHIAI 1970 2001 1977 40 14010 SGO4 EKTREME 18,03% 5 25,393 5 ARROWHAGD RAD 6 PUCSCHIAI 1970 2001 1976 40 14010 56.04 EKTREME 18,13% 5 52,293 5 MARROWHAGD RAD 6 PUCSCHIAI 1970 2000 2005 40 14010 56.04 EKTREME 18,13% 5 52,293 5 MARROWHAGD RAD 6 PUCSCHIAI 1970 2005 2001 1970 40 14010 56.04 EKTREME 18,13% 5 25,293 5 25,293 5 25,293 6 7,293 7 7,293 7 7 1,200 2001 19,00 2001 19,00 </td <td>279</td> <td>CHERRY LN.</td> <td>9</td> <td>PVC-SCH40</td> <td>1970</td> <td>29.22</td> <td>47</td> <td>9.010</td> <td>2013</td> <td>4.0</td> <td>14.010</td> <td>56.04</td> <td>EXTREME</td> <td>17.87%</td> <td></td> <td>\$ 381</td> <td>1</td>	279	CHERRY LN.	9	PVC-SCH40	1970	29.22	47	9.010	2013	4.0	14.010	56.04	EXTREME	17.87%		\$ 381	1
W. THRD AVE. 6 PVCSCH40 1970 2001 47 9101 1977 40 14010 56.04 EVTREME 13.9% 5 26.309 5 ARROWHEAD RO. 6 PVCSCH40 1970 118.11 47 9.010 1997 4.0 14.010 56.04 EVTREME 18.07% 5 25.293 5 27.293 5	280	CHERRY LN.	9	PVC-SCH40	1970	251.82	47	9.010	2013	4.0	14.010	56.04	EXTREME	17.93%		\$ 3,280	0
W.THIRD ANE, 6 PVC-SCH4D 1970 18.13 4.0 14.010 56.04 EYTREME 18.12% 5 15.545 5 ARROWHEAD RD, 4 PVC-SCH4D 1970 1976 4.0 14.010 56.04 EYTREME 18.12% 5 15.243 5 MARONULIAST, 6 PVC-SCH4D 1970 239.40 47 9.010 2005 4.0 14.010 56.04 EYTREME 18.12% 5 52.223 5 VACROLIAST, 6 PVC-SCH4D 1980 239.40 67 20.0 11.057 55.28 HIGH 18.15% 5 63.124 5 7 N CEDATION AND AVE. 6 PVC-SCH4D 1990 23.7 20.0 5.0 10.78 5.39 HIGH 18.54% 5 63.124 5 7 E. NINTH ANE 8 CT 1947 55.65 70 5.02 5.0 10.78 5.39 HIGH 18.54% 5 5.1245 5 7.41	449	VEATER ST.	9	PVC-SCH40	1970	200.01	47	9.010	1977	4.0	14.010	56.04	EXTREME	17.99%		\$ 2,605	2
ARRONNIERD RD. 4 9010 1976 40 14010 56.04 EXTREME 18.12% 5 52.293 5 MAGNOUIA ST. 6 PVC-SCH40 1970 2034 4 9.010 2005 4.0 14.010 56.04 EXTREME 18.18% 5 31,490 5 VETERANS WY. 6 PVC-SCH40 1980 2033.2 3 6.057 2011 5.0 11.057 55.28 HIGH 18.23% 5 26,745 5 N. CEDAR ST. 6 PVC-SCH40 1980 479.88 3 6.057 2011 5.0 11.057 55.28 HIGH 18.35% 5 6.173 5 6.057 5 7 11.057 55.28 HIGH 18.23% 5 93.25 5 93.25 5 93.25 5 93.25 5 93.25 5 93.25 5 93.25 5 93.25 5 93.25 5 93.25 5 93.25	634	W. THIRD AVE.	9	PVC-SCH40	1970	118.18	47	9.010	1997	4.0	14.010	56.04	EXTREME	18.02%		\$ 1,539	6
MAGNOLIAST, 6 PVCSCH4IO 1970 23940 47 9010 2005 40 14.010 56.04 EXTREM< 1813% 5 26,445 5 VETERANS WY. 6 PVCSCH4IO 1980 479.88 37 6.057 2011 5.0 11.057 55.28 HIGH 18.33% 5 26,445 5 1 N CEDAR ST. 6 PVCSCH4IO 1980 479.88 37 6.057 11.057 55.28 HIGH 18.33% 5 26,445 5 1 PRESSION ST. 6 PVCSCH4IO 1980 47.0 1983 5.0 11.057 55.28 HIGH 18.83% 5 26,445 5 7 <	898	ARROWHEAD RD		PVC-SCH40	1970	397.54	47	9.010	1976	4.0	14.010	56.04	EXTREME	18.12%		\$ 5,178	∞
VETERANS WY. 6 POVESCHAIG 1980 203.3.2 37 6.057 2011 5.0 11.057 55.28 HIGH 18.23% 5 26,745 5 N. CEDAR ST. 6 PVC-SCH40 1980 479.88 37 6.057 1983 5.0 11.057 55.28 HIGH 18.35% 5 631.24 5 7 E. THIRD ANE 6 PVC-SCH40 1980 7.0 1.831 2009 8.0 6.831 54.65 HIGH 18.45% 5 93.53 5 7 E. THIRD ANE 8 Cl 1947 55.565 70 5.782 10.782 53.91 HIGH 18.45% 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5 91.535 5	1104	MAGNOLIA ST.		PVC-SCH40	1970	239.40	47	9.010	2005	4.0	14.010	56.04	EXTREME	18.18%		\$ 3,118	00
N. CEDAR ST. 6 PUC-SCH40 1880 479.88 37 6057 1983 50 11057 55.28 HIGH 18.35% 5 63.144 5 E. THIRD AVE. 6 PUC-SCH40 2000 714.25 17 1831 2009 5.0 10.782 53.91 HIGH 18.6% 5 93.953 5 PERSHINGST. 8 CI 1947 55.565 70 5.782 20.4 5.0 10.782 53.91 HIGH 18.6% 5 93.953 5 93.953 5 93.955 5 93.955 5 93.955 5 93.955 5 93.955 5 93.955 5 93.955 5 93.955 5 93.955 6 93.955 93.955 93.955 5 93.955 5 93.955 5 93.955 5 93.955 5 93.955 5 93.955 5 93.955 93.955 93.955 5 93.955 5	250	VETERANS WY.	9	PVC-SCH40	1980	203.32	37	6.057	2011	5.0	11.057	55.28	HIGH	18.23%		\$ 10,546	9
E. HIRID AVIC. 6 PVCSCHAIO 2000 714.25 1 18.31 2009 8.0 6.831 54.65 HIGH 18.63% 5 93.953 5 PERSHINGST. 8 CI 1947 555.65 70 5.782 1905 5.0 10.782 53.91 HIGH 18.68% 5 97.455 5 9 5	309	N. CEDAR ST.	9	PVC-SCH40	1980	479.88	37	6.057	1983	5.0	11.057	55.28	HIGH	18.35%		\$ 24,890	0
PERSHINGST. 8 CI 1947 555.65 70 5.782 1995 5.0 10.782 53.91 HIGH 18.68% \$ 97.455 \$ E. NINTH AVE. 8 CI 1947 52.79 70 5.782 2004 5.0 10.782 53.91 HIGH 18.69% \$ 97.455 \$ E. NINTH AVE. 8 CI 1947 607.30 70 5.782 2004 5.0 10.782 53.91 HIGH 18.69% \$ 97.455 \$ W. THIRDAYE. 8 CI 1970 906.74 47 5.306 190 4.0 13.342 53.95 4.0 13.342 53.95 4.0 13.342 53.95 4.0 13.342 53.95 4.0 13.342 53.95 4.0 13.342 53.95 4.0 13.342 53.95 4.0 13.342 53.95 4.0 13.342 53.95 4.0 13.342 53.95 4.0 13.342	283	E. THIRD AVE.	. 9	PVC-SCH40	2000	714.25	17	1.831	2009	8.0	6.831	54.65	HIGH	18.54%		\$ 76,751	Н
E. NINTH AVE. 8 CI 1947 5.782 2004 5.0 10.782 53.91 HIGH 18.69% \$ 9,259 \$ E. NINTH AVE. 8 CI 1947 60.730 70 5.782 2004 5.0 10.782 53.91 HIGH 18.85% \$ 9,259 \$ W. THIRD AVE. 6 CI 1970 906.74 47 5.36 1997 40 13.342 53.37 HIGH 19.08% \$ 106.514 \$ W. THIRD AVE. 6 CI 1970 36.74 47 5.366 10.0 5.366 5.306 HIGH 19.08% \$ 11.21.23 \$ 7 HILLCREST ST. 4 PVC-SCH40 1990 457.45 27 3.664 1996 6.0 8.664 51.98 HIGH 19.15% \$ 11.819 \$ 7 13.69 \$ 13.10% \$ 13.10% \$ 13.10% \$ 13.10%	649	PERSHING ST.	8	D	1947	555.65	70	5.782	1995	5.0	10.782	53.91	HIGH	18.68%		\$ 41,106	9
E. NINTH AVE. 8 CI 1947 607.30 70 5.782 2004 5.0 10.782 53.91 HIGH 18.85% \$ 106,514 \$ W. THIRD AVE. 6 CI 1970 906.74 47 5.366 1997 4.0 13.342 53.37 HIGH 19.08% \$ 11,819 \$ 7 HILLCREST ST. 4 DVC-SCH40 1990 64.09 27 3.664 1995 6.0 8.664 51.98 HIGH 19.10% \$ 8,431 \$ HILLCREST ST. 4 PVC-SCH40 1990 64.09 27 3.664 1995 6.0 8.664 51.98 HIGH 19.10% \$ 8,431 \$ W. SECOND AVE. 6 PVC-SCH40 1990 27 3.664 1998 6.0 8.664 51.98 HIGH 19.56% \$ 24,1539 \$ W. SECOND AVE. 12 PVC-SCH40 1990 218.64 1995 </td <td>658</td> <td>E. NINTH AVE.</td> <td>∞</td> <td>IJ</td> <td>1947</td> <td>52.79</td> <td>70</td> <td>5.782</td> <td>2004</td> <td>5.0</td> <td>10.782</td> <td>53.91</td> <td>HIGH</td> <td>18.69%</td> <td></td> <td>\$ 3,906</td> <td>9</td>	658	E. NINTH AVE.	∞	IJ	1947	52.79	70	5.782	2004	5.0	10.782	53.91	HIGH	18.69%		\$ 3,906	9
W, THIRD ANE, 6 CI 1970 966.74 47 3.342 1997 4.0 13.342 53.37 HIGH 19.08% \$ 11,813 \$ HILLCREST ST. 4 DI 1970 38.51 47 5.366 1969 10.0 5.306 53.06 HIGH 19.09% \$ 11,819 \$ HILLCREST ST. 4 PVC-SCH40 1990 64.09 27 3.664 1995 6.0 8.664 51.98 HIGH 19.10% \$ 23,690 \$ MERCURY ST. 6 PVC-SCH40 1990 457.45 27 3.664 1995 6.0 8.664 51.98 HIGH 19.10% \$ 23,690 \$ W, SECOND AVE. 12 PVC-SCH40 1990 457.45 27 3.664 1995 6.0 8.664 51.98 HIGH 19.50% \$ 241,539 \$ W, SECOND AVE. 12 PVC-SCH40 1990 222.65 2	820	E. NINTH AVE.	∞	IJ	1947	607.30	70	5.782	2004	5.0	10.782	53.91	HIGH	18.85%		\$ 44,927	7
HILCREST ST. 4 PVC-SCH40 1990 64.09 27 3.664 1995 6.0 8.664 51.98 HIGH 19.09% \$ 11,819 \$ 11,819 \$ 1411LCREST ST. 4 PVC-SCH40 1990 64.09 27 3.664 1995 6.0 8.664 51.98 HIGH 19.15% \$ 23,690 \$ 141,819 \$ 1411LCREST ST. 4 PVC-SCH40 1990 457.45 27 3.664 1995 6.0 8.664 51.98 HIGH 19.15% \$ 23,690 \$ 141,839 \$ 15	755	W. THIRD AVE.	9	D	1970	906.74	47	3.342	1997	4.0	13.342	53.37	HIGH	19.08%		\$ 79,415	N
HILLCREST ST. 4 PVC-SCH40 1990 64.09 27 3.664 1995 6.0 8.664 51.98 HIGH 19.10% \$ 8.431 \$ 8.431 \$ 11LCREST ST. 4 PVC-SCH40 1990 180.10 27 3.664 1995 6.0 8.664 51.98 HIGH 19.17% \$ 23,690 \$ 1.4 IIILCREST ST. 6 PVC-SCH40 1990 457.45 27 3.664 1995 6.0 8.664 51.98 HIGH 19.57% \$ 60,173 \$ 1.4 IIICREST ST. 6 PVC-SCH40 1990 222.65 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IIICREST ST. 6 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IIICREST ST. 6 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 4,286 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 1.4 IICREST ST. 6 PVC-SCH40 1990 1990 16.29 27 3.664 1995 1990 16.29 28 3.665 5 1.4 IICREST ST. 6 PVC-SCH40 1990 1990 16.29 28 3.665 5 1.4 IICREST ST. 6 PVC-SCH40 1990 1990 16.29 27 3.664 1995 1990 16.29 27 3.664 1995 1990 16.29 28 3.665 5 1.4 IICREST ST. 6 PVC-SCH40 1990 1990 16.29 27 3.664 1990 1990 16.29 27 3.664 1990 1990 16.29 27 3.664 1990 1990 16.29 27 3.664 1990 1990 16.29 27 3.664 1990 1990 16.29 27 3.664 1990 1990 16.29 27 3.664 1990 1990 16.29 27 3.664 1990 1990 16.29 27 3.664 1990 1990 1990 1990 1990 1990 1990 199	892		14	10	1970	38.51	47	5.306	1969	10.0	5.306	53.06	HIGH	19.09%		\$ 5,548	∞ l
HILLCRESTST. 4 PVC-SCH40 1990 180.10 27 3.664 1995 6.0 8.664 51.98 HIGH 19.15% \$ 23,690 \$ 11 MERCURY ST. 6 PVC-SCH40 1990 457.45 27 3.664 1969 6.0 8.664 51.98 HIGH 19.70% \$ 60.173 \$ W. SECOND AVE. 12 PVC-SCH40 1990 918.09 27 3.664 1984 6.0 8.664 51.98 HIGH 19.50% \$ 241,539 \$ 15 W. SECOND AVE. 12 PVC-SCH40 1990 222.65 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ W. SECOND AVE. 12 PVC-SCH40 1990 16.29 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 4.286 \$ W. SECOND AVE. 12 PVC-SCH40 1990	64	HILLCREST ST.	4	PVC-SCH40	1990	64.09	27	3.664	1995	6.0	8.664	51.98	HIGH	19.10%		\$ 5,342	7
MERCURY ST. 6 PVC-SCH40 1990 457.45 27 3.664 1969 6.0 8.664 51.98 HIGH 19.27% \$ 60,173 \$ W, SECOND AVE. 12 PVC-SCH40 1990 218.09 27 3.664 1995 6.0 8.664 51.98 HIGH 19.50% \$ 241,539 \$ 15 W, SECOND AVE. 12 PVC-SCH40 1990 222.65 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ W, SECOND AVE. 12 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 4,286 \$ W, SECOND AVE. 12 PVC-SCH40 1990 203.83 27 3.664 1995 6.0 8.664 51.98 HIGH 19.61% \$ 4,286 \$	65	HILLCREST ST.	4	PVC-SCH40	1990	180.10	27	3.664	1995	6.0	8.664	51.98	HIGH	19.15%		\$ 15,010	0
W. SECOND AVE. 12 PVC-SCH40 1990 918.09 27 3.664 1995 6.0 8.664 51.98 HIGH 19.50% \$ 241,539 \$ 15 W. SECOND AVE. 12 PVC-SCH40 1990 222.65 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58,578 \$ 13 W. SECOND AVE. 12 PVC-SCH40 1990 203.83 27 3.664 1995 6.0 8.664 51.98 HIGH 19.51% \$ 4,286 \$ W. SECOND AVE. 12 PVC-SCH40 1990 203.83 27 3.664 1995 6.0 8.664 51.98 HIGH 19.61% \$ 4,286 \$	246	MERCURY ST.	9	PVC-SCH40	1990	457.45	27	3.664	1969	6.0	8.664	51.98	HIGH	19.27%		\$ 38,126	9
W, SECOND AVE. 12 PVC-SCH40 1990 222.65 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 58.578 \$ W, SECOND AVE. 12 PVC-SCH40 1990 16.29 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 4,286 \$ W, SECOND AVE. 12 PVC-SCH40 1990 203.83 27 3.664 1995 6.0 8.664 51.98 HIGH 19.61% \$ 53,625 \$	383	W. SECOND AVE		PVC-SCH40	1990	918.09	7.2	3.664	1984	6.0	8.664	51.98	HIGH	19.50%		\$ 153,042	7
W, SECOND AVE. 12 PVC-SCH40 1990 203.83 27 3.664 1995 6.0 8.664 51.98 HIGH 19.56% \$ 4,286 \$ \$ W, SECOND AVE. 12 PVC-SCH40 1990 203.83 27 3.664 1995 6.0 8.664 51.98 HIGH 19.61% \$ 53,625 \$ 3	384	W. SECOND AVE		PVC-SCH40	1990	222.65	27	3.664	1995	0.9	8.664	51.98	HIGH	19.56%		\$ 37,115	5
W. SECOND AVE. 12 PVC-SCH40 1990 203.83 27 3.664 1995 6.0 8.664 51.98 HIGH 19.61% \$ 53,625 \$	385	W. SECOND AVE		PVC-SCH40	1990	16.29	7.2	3.664	1995	6.0	8.664	51.98	HIGH	19.56%		\$ 2,716	9
	386	W. SECOND AVE		PVC-SCH40	1990	203.83	27	3.664	1995	6.0	8.664	51.98	HIGH	19.61%		\$ 33,977	7

APPENDIX A Risk Scoring of Water System Distribution System Assets and Estimated Replacement Value/Current Value

PIPE		VEAR	SHADE	AGE (In			CONSCILENCE	MOUTION	DICK		SUM IOTAL	ESTIMA	Estimated Asset Est	Estimated Asset
5	DIAMETE MATERIAL INSTALLED R	ISTALLED	LENGTH	2017)	AGE_SCORE	HydrantAge	SCORE	SCORE	SCORE	RISK_LEVEL	PERCENT (LENGTH)	Replacer (as o	Replacement Value C (as of 2015)	Current Value (Based on
VC	PVC-SCH40	1990	301.02	7.2	3.664	1991	6.0	8.664	51.98	HIGH	19.69%	\$	\$ 961,67	50,180
>	PVC-SCH40	1990	35.88	27	3.664	1991	6.0	8.664	51.98	HIGH	19.70%	\$	9,441 \$	5,982
>	PVC-SCH40	1990	880.61	7.7	3.664	1997	6.0	8.664	51.98	HIGH	19.92%	\$	231,678 \$	146,794
	PVC-SCH40	1990	140.37	27	3.664	1995	0.9	8.664	51.98	HIGH	19.96%	\$	\$ 086,98	23,399
	PVC-SCH40	1990	182.64	7.2	3.664	1984	6.0	8.664	51.98	HIGH	20.01%	\$	48,050 \$	30,445
	PVC-SCH40	1990	635.69	77	3.664	1991	6.0	8.664	51.98	HIGH	20.17%	\$	167,245 \$	105,968
	PVC-SCH40	1990	905.34	27	3.664	1997	6.0	8.664	51.98	HIGH	20.40%	\$	238,186 \$	150,917
	PVC-SCH40	1990	1464.78	7.7	3.664	2006	0.9	8.664	51.98	HIGH	20.77%	\$	\$85,368 \$	244,173
0	PVC-SCH40	1990	436.09	27	3.664	1995	6.0	8.664	51.98	HIGH	20.88%	\$	114,730 \$	72,694
100	AC	1935	320.95	82	12.989	2009	4.0	12.989	51.96	HIGH	20.97%	\$	42,218 \$	42,218
55,625	AC	1935	357.33	82	12.989	2009	4.0	12.989	51.96	HIGH	21.06%	\$	47,003 \$	47,003
	AC	1935	1157.07	82	12.989	1975-1978	4.0	12.989	51.96	HIGH	21.35%	\$	152,201 \$	152,201
	AC	1935	270.46	82	12.989	1984	4.0	12.989	51.96	HIGH	21.42%	\$	\$ 92,576	35,576
	AC	1935	370.60	82	12.989	1984	4.0	12.989	51.96	HIGH	21.52%	\$	48,748 \$	48,748
	AC	1935	575.90	82	12.989	1976	4.0	12.989	51.96	HIGH	21.66%	\$	75,754 \$	75,754
	AC	1935	507.28	82	12.989	1995	4.0	12.989	51.96	HIGH	21.79%	\$	66,727 \$	66,727
	AC	1935	114.59	82	12.989	1975	4.0	12.989	51.96	HIGH	21.82%	\$	15,074 \$	15,074
	AC	1935	1420.00	82	12.989	1995	4.0	12.989	51.96	HIGH	22.19%	\$	249,054 \$	249,054
	AC	1935	477.88	82	12.989	1998	4.0	12.989	51.96	HIGH	22.31%	\$	62,861 \$	62,861
	AC	1935	365.63	82	12.989	1963	4.0	12.989	51.96	HIGH	22.40%	\$	\$ 360,095	48,095
	AC	1935	313.50	82	12.989	1998	4.0	12.989	51.96	HIGH	22.48%	\$	41,238 \$	41,238
	AC	1935	334.26	82	12.989	2009	4.0	12.989	51.96	HIGH	22.57%	\$	43,969 \$	43,969
	AC	1935	364.70	82	12.989	1991	4.0	12.989	51.96	HIGH	22.66%	\$	47,972 \$	47,972
۵	PVC-SCH40	1991	1940.62	26	3.455	1991	6.0	8.455	50.73	HIGH	23.15%	\$	510,557 \$	334,139
۵	PVC-SCH40	1991	212.90	26	3.455	1991	6.0	8.455	50.73	HIGH	23.21%	\$	56,013 \$	36,658
۵	PVC-SCH40	1991	90.49	26	3.455	1991	6.0	8.455	50.73	HIGH	23.23%	\$	23,808 \$	15,581
Р	PVC-SCH40	1991	875.09	26	3.455	1991	6.0	8.455	50.73	HIGH	23.46%	\$	\$ 622,082	150,675
Р	PVC-SCH40	1991	456.10	26	3.455	1991	6.0	8.455	50.73	HIGH	23.57%	\$	\$ 566,611	78,532
Д	PVC-SCH40	1991	67.08	26	3.455	1991	6.0	8.455	50.73	HIGH	23.59%	\$	17,647 \$	11,549
٩	PVC-SCH40	1991	81.47	26	3.455	1991	6.0	8.455	50.73	HIGH	23.61%	\$	21,435 \$	14,028
	AC	1960	533.37	57	7.176	1980	7.0	7.176	50.23	HIGH	23.75%	\$	70,160 \$	19,811
	AC	1960	502.10	57	7.176	1980	7.0	7.176	50.23	HIGH	23.87%	\$	66,046 \$	18.649

RROADWAY ST. 6 AC 1960 290.03 57 7.176	GIS ID DESCRIPTION	PIPE ON DIAMETE	MATERIAL	YEAR	SHAPE	AGE (in	AGE_SCORE	HydrantAge	CONSEQUENCE	CONDITION	RISK	RISK_LEVEL	SUM TOTAL PERCENT	Estimated Asset Replacement Value		Estimated Asset Current Value	ti e
IRON ST. 6 AC 1960 290.03 57 7,176 BROADWAY ST. 12 AC 1960 972.14 57 7,176 BROADWAY ST. 12 AC 1960 299.79 57 7,176 BROADWAY ST. 12 AC 1960 299.79 57 7,176 BROADWAY ST. 12 AC 1960 567.50 57 7,176 BROADWAY ST. 12 AC 1960 583.25 57 7,176 BROADWAY ST. 12 AC 1960 38.92 57 7,176 BROADWAY ST. 12 AC 1960 38.92 57 7,176 BROADWAY ST. 12 AC 1960 38.92 57 7,176 BROADWAY ST. 12 AC 1960 38.93 57 7,176 BROADWAY ST. 12 AC 1960 38.93 57 7,176 BROADWAY ST. 12 AC 1960 38.93 57 7,176 BROADWAY ST. 12 AC 1970 38.43 57 7,176 BROADWAY ST. 12 AC 1970 34.48 57 7,176 BROADWAY ST. 12 AC 1970 34.48 47 3,342 BROADWAY ST. 12 PVC-SCH40 1975 244.97 42 7,463 BROADWAY ST. 12 PVC-SCH40 1975 235.50 42 7,463 BROADWAY ST. 12 PVC-SCH40 2000 603.92 17 1,831 BROADWAY ST. 12 PVC-SCH40 2000 50.01.91 17 1,831 COOK ST. 12 PVC-SCH40 2000 50.01.91 17 1,831 BROADWAY ST. 12 PVC-SCH40 2000 50.01.91 17 1,831 BROADWAY ST. 12 PVC-SCH40 2000 50.01.91 17 1,831 COOK ST. 12 PVC-SCH40 2000 50.01.91 17 1,831 BROADWAY ST. 12 PVC-SCH40 2000 50.01.91 17 1,831 COOK ST. 12 PVC-SCH40 2000 50.01.91 17 1,831 ST. 12 PVC-SCH40 2000 20.01 17 1,1831 ST. 13 PVC-SCH40 2000 20.01.91 17 1,831 ST. 14.44 14.44 14.44 14.44 14.44 ST. 14.44 14.44 14.44 14.44 ST. 14.44 14.44 14.44 14.44 14.44		R		INSTALLED	LENGIA	1/107			SCORE	SCURE	SCORE		(LENGTH)	(as of 2015	2015)	(Based on	
BROADWAYST. 12 AC 1960 972.14 57 7.176 BROADWAYST. 12 AC 1960 980.52 57 7.176 BROADWAYST. 12 AC 1960 299.79 57 7.176 BROADWAYST. 12 AC 1960 567.50 57 7.176 BROADWAYST. 12 AC 1960 563.25 57 7.176 BROADWAYST. 12 AC 1960 360.00 57 7.176 BROADWAYST. 12 AC 1960 360.00 57 7.176 BROADWAYST. 12 AC 1960 360.00 57 7.176 W. SIXTH AVE. 6 AC 1960 348.39 57 7.176 BROADWAYST. 12 AC 1960 348.39 57 7.176 W. SIXTH AVE. 6 CI 1970 348.38 47 3.342 W. SIXTH AVE. 6 CI 1970	IRON ST.	9	AC	1960	290.03	57	7.176	1969-1995	7.0	7.176	50.23	HIGH	23.95%	\$	38,150 \$	10,773	773
BROADWAYST. 12 AC 1960 980.52 57 7.176 BROADWAYST. 12 AC 1960 299.79 57 7.176 BROADWAYST. 12 AC 1960 567.50 57 7.176 BROADWAYST. 12 AC 1960 563.25 57 7.176 BROADWAYST. 12 AC 1960 348.92 57 7.176 E. FOURTH AVE. 4 AC 1960 348.92 57 7.176 W. SIXTH AVE. 6 AC 1960 348.92 57 7.176 BROADWAYST. 12 AC 1960 35.014 57 7.176 W. SIXTH AVE. 6 AC 1960 35.014 57 7.176 BROADWAYST. 12 AC 1960 35.014 57 7.176 BROADWAYST. 12 AC 1960 35.014 57 7.176 BROADWAYST. 4 AC 1960	BROADWAY		AC	1960	972.14	57	7.176	1967	7.0	7.176	50.23	нен	24.20%	\$	255,761 \$	72,219	219
BROADWAYST. 12 AC 1960 299.79 57 7.176 BROADWAYST. 12 AC 1960 567.50 57 7.176 BROADWAYST. 12 AC 1960 563.25 57 7.176 BROADWAYST. 12 AC 1960 348.92 57 7.176 COOK ST. 12 AC 1960 348.92 57 7.176 W. SIXTH AVE. 6 AC 1960 348.92 57 7.176 W. SIXTH AVE. 6 AC 1960 348.92 57 7.176 BROADWAY ST. 12 AC 1960 35.43 57 7.176 W. SIXTH AVE. 6 CI 1970 343.82 47 3.342 E. THIRD AVE. 4 CI 1970 344.85 47 3.342 E. THIRD AVE. 6 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 CI 1970	BROADWAY		AC	1960	980.52	57	7.176	1969	7.0	7.176	50.23	HIGH	24.45%	\$	\$ 596,752	72,842	842
BROADWAY ST. 12 AC 1960 567.50 57 7.176 BROADWAY ST. 12 AC 1960 563.25 57 7.176 BROADWAY ST. 12 AC 1960 348.92 57 7.176 COOK ST. 12 AC 1960 360.00 57 7.176 W. SIXTH AVE. 6 AC 1960 348.39 57 7.176 BROADWAY ST. 12 AC 1960 346.39 57 7.176 BROADWAY ST. 12 AC 1960 345.30 57 7.176 BROADWAY ST. 4 AC 1960 345.30 57 7.176 BROADWAY ST. 4 AC 1960 345.30 57 7.176 BROADWAY ST. 4 CI 1970 344.85 47 3.342 BROADWAY ST. 4 CI 1970 344.85 47 3.342 BROADWAY ST. 4 PVC-SCH40 <	BROADWAY		AC	1960	299.79	57	7.176	1969	7.0	7.176	50.23	HIGH	24.52%	\$	78,873 \$	22,271	271
BROADWAY ST. 12 AC 1960 563.25 57 7.176 BROADWAY ST. 12 AC 1960 348.92 57 7.176 COOK ST. 12 AC 1960 360.00 57 7.176 W. SIXTH AVE. 6 AC 1960 334.39 57 7.176 BROADWAY ST. 12 AC 1960 346.39 57 7.176 MAPLE ST. 6 CI 1970 328.75 47 7.176 MAPLE ST. 6 CI 1970 328.75 47 7.176 MAPLE ST. 6 CI 1970 328.75 47 3.342 E. THIRD AVE. 6 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 PVC-SCH40 1975	BROADWAY		AC	1960	567.50	57	7.176	1969	7.0	7.176	50.23	HIGH	24.67%	\$	149,305 \$	42,1	42,159
BROADWAY ST. 12 AC 1960 348.92 57 7.176 E. FOURTH AVE. 4 AC 1960 36.00 57 7.176 COOK ST. 12 AC 1960 334.39 57 7.176 BROADWAY ST. 12 AC 1960 334.39 57 7.176 BROADWAY ST. 12 AC 1960 350.14 57 7.176 BROADWAY ST. 12 AC 1960 334.39 57 7.176 BROADWAY ST. 12 AC 1960 334.39 57 7.176 BROADWAY ST. 4 CI 1970 728.73 47 3.342 E. THIRD AVE. 4 CI 1970 339.57 47 3.342 E. THIRD AVE. 4 CI 1970 344.85 47 3.342 E. THIRD AVE. 6 PVC-SCH40 1975 244.97 47 7.463 BROADWAY ST. 4 PVC-SCH40	BROADWAY		AC	1960	563.25	57	7.176	1969	7.0	7.176	50.23	HIGH	24.81%	\$	148,185 \$	41,843	843
E. FOURTH AVE. 4 AC 1960 360.00 57 7.176 COOK ST. 12 AC 1960 380.04 57 7.176 W. SIXTH AVE. 6 AC 1960 334.39 57 7.176 BROADWAY ST. 12 AC 1960 343.39 57 7.176 AMPLE ST. 6 CI 1960 345.30 57 7.176 MAPLE ST. 6 CI 1960 346.30 57 7.176 MAPLE ST. 12 AC 1960 345.30 57 7.176 MAPLE ST. 6 CI 1970 328.75 47 3.342 E. THIRD AVE. 4 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 PVC-SCH40 1975 244.97 47 3.342 M. SIXTH AVE. 6 PVC-SCH40 1975 244.97 47 7.463 R. PVC-SCH40 1975 244.97	BROADWAY		AC	1960	348.92	57	7.176	1969	7.0	7.176	50.23	HIGH	24.90%	\$	\$ 797,19	25,921	921
COOK ST. 12 AC 1960 192.64 57 7.176 W. SIXTH AVE. 6 AC 1960 334.39 57 7.176 BROADWAY ST. 12 AC 1960 545.30 57 7.176 MAPIE ST. 6 CI 1960 545.30 57 7.176 MAPIE ST. 6 CI 1970 728.75 47 3.342 E. THIRD AVE. 4 CI 1970 315.42 47 3.342 E. THIRD AVE. 4 CI 1970 344.85 47 3.342 E. THIRD AVE. 6 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 CI 1970 40.76 47 3.342 E. THIRD AVE. 6 PVC-SCH40 1975 244.97 47 3.342 E. THIRD AVE. 6 PVC-SCH40 1975 235.50 42 7.463 BROADWAY ST. 12 PVC-SCH40	E. FOURTH A		AC	1960	360.00	57	7.176	1981	7.0	7.176	50.23	HIGH	24.99%	\$	47,355 \$	13,3	13,372
W. SIXTH AVE. 6 AC 1960 334.39 57 7.176 BROADWAY ST. 12 AC 1960 350.14 57 7.176 BROADWAY ST. 12 AC 1960 545.30 57 7.176 MAPLE ST. 6 CI 1970 728.75 47 3.342 E. THIRD AVE. 4 CI 1970 315.42 47 3.342 E. THIRD AVE. 4 CI 1970 315.42 47 3.342 E. THIRD AVE. 4 CI 1970 338.57 47 3.342 W. SIXTH AVE. 6 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 PVC-SCH40 1975 244.97 42 7.463 RIVERSIDE DR. 4 PVC-SCH40 1975 235.50 42 7.463 BROADWAY ST. 12 PVC-SCH40 1992 501.93 25 3.253 COOK ST. 12 PV	COOK ST.		AC	1960	192.64	57	7.176	1969	7.0	7.176	50.23	нівн	25.04%	\$	50,681 \$	14,3	14,311
BROADWAY ST. 12 AC 1960 350.14 57 7.176 MAPLE ST. 8 AC 1960 545.30 57 7.176 MAPLE ST. 6 CI 1970 728.75 47 3.342 E. THIRD AVE. 4 CI 1970 315.42 47 3.342 E. THIRD AVE. 4 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 CI 1970 40.76 47 3.342 W. SIXTH AVE. 6 PVC-SCH40 1975 244.97 47 3.342 B N. SIXTH AVE. 6 PVC-SCH40 1975 244.97 42 7.463 B N. SIXTH AVE. 6 PVC-SCH40 1975 235.50 42 7.463 B ROADWAY ST. 12 PVC-SCH40 2000 603.92 17 1.831 COOK ST. 12	W. SIXTH AN		AC	1960	334.39	57	7.176	1970	7.0	7.176	50.23	HIGH	25.13%	\$	43,985 \$	12,4	12,420
R AC 1960 545.30 57 7.176 MAPLE ST. 6 CI 1970 728.75 47 3.342 E. THIRD AVE. 4 CI 1970 315.42 47 3.342 E. THIRD AVE. 4 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 CI 1970 40.76 47 3.342 W. SIXTH AVE. 6 PVC-SCH40 1975 244.97 47 3.342 W. SIXTH AVE. 6 PVC-SCH40 1975 244.97 47 3.342 R. V. SIXTH AVE. 6 PVC-SCH40 1975 244.97 47 3.342 R. SIXTH AVE. 6 PVC-SCH40 1975 244.97 47 3.342 R. SIXTH AVE. 6 PVC-SCH40 1975 235.50 42 7.463 R. SIXTH AVE. 6 PVC-SCH40 1992 501.93 25 3.253 R. SIXTH AVE. 12 PVC-S	BROADWAY		AC	1960	350.14	57	7.176	1969	7.0	7.176	50.23	нын	25.22%	\$	92,117 \$	26,0	26,011
MAPLE ST. 6 CI 1970 728.75 47 3:342 E. THIRD AVE. 4 CI 1970 315.42 47 3:342 E. THIRD AVE. 4 CI 1970 339.57 47 3:342 W. SIXTH AVE. 6 CI 1970 344.85 47 3:342 W. SIXTH AVE. 6 CI 1970 344.85 47 3:342 W. SIXTH AVE. 6 CI 1970 40.76 47 3:342 W. SIXTH AVE. 6 PVC-SCH40 1975 244.97 42 7.463 R. V. SIXTH AVE. 6 PVC-SCH40 1975 235.50 42 7.463 R. V. SIXTH AVE. 6 PVC-SCH40 1975 235.50 42 7.463 R. V. SIXTH AVE. 6 PVC-SCH40 1992 501.93 25 7.463 R. ROADWAY ST. 12 PVC-SCH40 2000 500.93 17 1.831 COOK ST. <t< td=""><td></td><td>∞</td><td>AC</td><td>1960</td><td>545.30</td><td>57</td><td>7.176</td><td>1999</td><td>7.0</td><td>7.176</td><td>50.23</td><td>нен</td><td>25.36%</td><td>\$</td><td>95,641 \$</td><td>27,0</td><td>27,006</td></t<>		∞	AC	1960	545.30	57	7.176	1999	7.0	7.176	50.23	нен	25.36%	\$	95,641 \$	27,0	27,006
E. THIRD AVE. 4 CI 1970 315.42 47 3.342 E. THIRD AVE. 4 CI 1970 339.57 47 3.342 W. SIXTH AVE. 6 CI 1970 344.85 47 3.342 W. SIXTH AVE. 6 CI 1970 40.76 47 3.342 W. SIXTH AVE. 6 PVC-SCH40 1975 244.97 42 7.463 E PVC-SCH40 1975 235.50 42 7.463 E PVC-SCH40 1975 235.50 42 7.463 E PVC-SCH40 1975 201.93 25 3.253 UPSON ST. 12 PVC-SCH40 2000 603.92 17 1.831 EROADWAY ST. 12 PVC-SCH40 2000 501.21 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 E PVC-SCH40 2000 501.21 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 E PVC-SCH40 2000 501.21 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 E PVC-SCH40 2000 739.11 17 1.831 E PVC-SCH40 2000 739.88 17 1.831 COOK ST. 12 PVC-SCH40 2000 730.88 17 1.831 COOK ST. 12 PVC-SCH40 2000 730.88 17 1.831	MAPLEST		D	1970	728.75	47	3.342	1971	6.0	8.342	50.05	нівн	25.54%	\$	\$ 658'56	63,8	63,826
E. THIRD AVE. 4 CI 1970 339.57 47 3:342 E. THIRD AVE. 4 CI 1970 344.85 47 3:342 W. SIXTH AVE. 6 PVC-SCH40 1975 244.97 42 7.463 AVE.SIXTH AVE. 6 PVC-SCH40 1975 235.50 42 7.463 AVE.SCH40 1975 235.50 42 7.463 BROADWAY ST. 4 PVC-SCH40 1992 501.93 25 3.253 BROADWAY ST. 12 PVC-SCH40 2000 603.92 17 1.831 COOK ST. 12 PVC-SCH40 2000 603.92 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 501.21 17 1.831 COOK ST. 12 PVC-SCH40 2000 520.01 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000		9	D	1970	315.42	47	3.342	2012	6.0	8.342	50.05	HIGH	25.62%	\$	41,490 \$	27,6	27,625
E. THIRD AVE. 4 CI 1970 344.85 47 3:342 W. SIXTH AVE. 6 CI 1970 40.76 47 3:342 M. SIXTH AVE. 6 PVC-SCH40 1975 244.97 42 7.463 FORDARIA 1975 235.50 42 7.463 7.463 FORDARIA 1975 235.50 42 7.463 RIVERSIDE DR. 4 PVC-SCH40 1975 501.93 25 7.463 BROADWAY ST. 4 PVC-SCH40 2000 603.92 17 1.831 COOK ST. 12 PVC-SCH40 2000 603.92 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 360.02 17 1.831 COOK ST. 12 PVC-SCH40 2000 550.01 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 <th< td=""><td>E. THIRD AV</td><td></td><td>IJ</td><td>1970</td><td>339.57</td><td>47</td><td>3.342</td><td>1995</td><td>6.0</td><td>8.342</td><td>50.05</td><td>нівн</td><td>25.71%</td><td>\$</td><td>\$ 44,667 \$</td><td>29,</td><td>29,741</td></th<>	E. THIRD AV		IJ	1970	339.57	47	3.342	1995	6.0	8.342	50.05	нівн	25.71%	\$	\$ 44,667 \$	29,	29,741
W. SIXTH AVE. 6 CI 1970 40.76 47 3.342 M. SIXTH AVE. 6 PVC-SCH40 1975 244.97 42 7.463 6 PVC-SCH40 1975 938.06 42 7.463 7 7 7 7.463 7.463 8 PVC-SCH40 1975 235.50 42 7.463 8 PVC-SCH40 1975 105.36 42 7.463 9 PVC-SCH40 1992 501.93 25 3.253 9 PVC-SCH40 2000 603.92 17 1831 9 PVC-SCH40 2000 1168.11 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 9 PVC-SCH40 2000 520.01 17 1.831 8 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831	E. THIRD AV		D	1970	344.85	. 47	3.342	1963	6.0	8.342	50.05	HIGH	25.80%	\$	45,361 \$	30,2	30,203
6 PVC-SCH40 1975 244.97 42 7.463 6 PVC-SCH40 1975 938.06 42 7.463 6 PVC-SCH40 1975 235.50 42 7.463 6 PVC-SCH40 1975 235.50 42 7.463 6 PVC-SCH40 1975 105.36 42 7.463 COPPRINT 4 PVC-SCH40 1992 501.93 25 3.253 BROADWAY ST. 12 PVC-SCH40 2000 603.92 17 1.831 COOK ST. 12 PVC-SCH40 2000 360.02 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 501.21 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 COOK ST. 12 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 COOK ST. 12 PVC-SCH40 2000 739.11 17 1.831	W. SIXTH A		D	1970	40.76	47	3.342	1991	6.0	8.342	50.05	HIGH	25.81%	\$	5,362 \$	3,5	3,570
6 PVC-SCH40 1975 938.06 42 7.463 6 PVC-SCH40 1975 235.50 42 7.463 RIVERSIDE DR. 4 PVC-SCH40 1975 105.36 42 7.463 BROADWAY ST. 4 PVC-SCH40 1992 501.93 25 3.253 BROADWAY ST. 4 PVC-SCH40 2000 603.92 17 1.831 COOK ST. 12 PVC-SCH40 2000 360.02 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 501.21 17 1.831 COOK ST. 12 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.18 17 1.831 2 PVC-SCH40 2000 739.18 37 2.446 <td< td=""><td></td><td>9</td><td>PVC-SCH40</td><td>1975</td><td>244.97</td><td>42</td><td>7.463</td><td>2012</td><td>4.0</td><td>12.463</td><td>49.85</td><td>нын</td><td>25.87%</td><td>\$</td><td>32,224 \$</td><td>8,</td><td>8,174</td></td<>		9	PVC-SCH40	1975	244.97	42	7.463	2012	4.0	12.463	49.85	нын	25.87%	\$	32,224 \$	8,	8,174
6 PVC-SCH40 1975 235.50 42 7.463 RIVERSIDE DR. 4 PVC-SCH40 1975 105.36 42 7.463 UPSON ST. 4 PVC-SCH40 1992 501.93 25 3.253 BROADWAY ST. 4 PVC-SCH40 2000 603.92 17 1.831 COOK ST. 12 PVC-SCH40 2000 360.02 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 501.21 17 1.831 2 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.18 17 1.831 2 PVC-SCH40 2000 739.18 37 2.446 3 2 PVC-SCH40 2000 730.88 17 1.831 <td></td> <td>9</td> <td>PVC-SCH40</td> <td>1975</td> <td>938.06</td> <td>42</td> <td>7.463</td> <td>2012</td> <td>4.0</td> <td>12.463</td> <td>49.85</td> <td>HIGH</td> <td>26.11%</td> <td>\$</td> <td>123,393 \$</td> <td>31,3</td> <td>31,300</td>		9	PVC-SCH40	1975	938.06	42	7.463	2012	4.0	12.463	49.85	HIGH	26.11%	\$	123,393 \$	31,3	31,300
6 PVC-SCH40 1975 105.36 42 7.463 RIVERSIDE DR. 4 PVC-SCH40 1992 501.93 25 3.253 UPSON ST. 12 PVC-SCH40 2000 603.92 17 1.831 COOK ST. 12 PVC-SCH40 2000 1168.11 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 501.21 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.18 17 1.831 2 PVC-SCH40 2000 739.18 17 1.831 COOK ST. 12 PVC-SCH40 2000 730.88 17 1.831		9	PVC-SCH40	1975	235.50	42	7.463	2012	4.0	12.463	49.85	HIGH	26.17%	\$	\$ 876,08	7,8	7,858
RIVERSIDE DR. 4 PVC-SCH40 1992 501.93 25 3.253 UPSON ST. 4 PVC-SCH40 2000 603.92 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 1168.11 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.18 17 1.831 2 PVC-SCH40 2000 730.88 17 1.831 2 PVC-SCH40 2000 730.88 17 1.831 2 PVC-SCH40 2000 730.88 17 1.831 3 CI 1980 86.48 37 2.446		9	PVC-SCH40	1975	105.36	42	7.463	2012	4.0	12.463	49.85	HIGH	26.20%	\$	13,859 \$	3,5	3,515
UPSON ST. 4 PVC-SCH40 2000 603.92 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 1168.11 17 1.831 COOK ST. 12 PVC-SCH40 2000 360.02 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 501.21 17 1.831 2 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 730.88 17 1.831 2 PVC-SCH40 2000 730.88 17 1.831 3 PVC-SCH40 2000 730.88 17 1.831 3 CI 1980 86.48 37 2.446	RIVERSIDE		PVC-SCH40	1992	501.93	25	3.253	2000	6.0	8.253	49.52	нвн	26.32%	\$	66,024 \$	44,	44,549
BROADWAY ST. 12 PVC-SCH40 2000 1168.11 17 1.831 COOK ST. 12 PVC-SCH40 2000 360.02 17 1.831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 730.88 17 1.831 2 PVC-SCH40 2000 730.88 17 1.831 3 12 CI 1980 86.48 37 2.446	TS NOSON ST		PVC-SCH40	2000	603.92	17	1.831	2001	7.0	6.831	47.82	HIGH	26.48%	\$	79,440 \$	64,8	64,895
COOK ST. 12 PVC-SCH40 2000 360.02 17 1831 COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.18 17 1.831 COOK ST. 12 CI 1980 86.48 37 2.446	BROADWAY		PVC-SCH40	2000	1168.11	17	1.831	1969	7.0	6.831	47.82	HIGH	26.78%	\$	307,319 \$	251,052	052
COOK ST. 12 PVC-SCH40 2000 501.21 17 1.831 BROADWAY ST. 12 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.18 17 1.831 2 PVC-SCH40 2000 739.88 17 1.831 COOK ST. 12 Cl 1980 86.48 37 2.446	COOK ST.		PVC-SCH40	2000	360.02	17	1.831	1969	7.0	6.831	47.82	нвн	26.87%	\$	94,718 \$,77,	77,376
BROADWAY ST. 12 PVC-SCH40 2000 520.01 17 1.831 2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 739.18 17 1.831 COOK ST. 12 Cl 1980 86.48 37 2.446	COOK ST.		PVC-SCH40	2000	501.21	17	1.831	1969	7.0	6.831	47.82	HIGH	27.00%	\$	131,864 \$	107,721	721
2 PVC-SCH40 2000 739.11 17 1.831 2 PVC-SCH40 2000 730.88 17 1.831 COOK ST. 12 CI 1980 86.48 37 2.446	BROADWAY		PVC-SCH40	2000	520.01	17	1.831	1969	7.0	6.831	47.82	HIGH	27.13%	\$.	136,809 \$	111,761	761
2 PVC-5CH40 2000 730.88 17 1.831 COOK ST. 12 Cl 1980 86.48 37 2.446		2	PVC-SCH40	2000	739.11	17	1.831	2001	7.0	6.831	47.82	HIGH	27.32%	\$	\$ 222,76	,67	79,422
COOK ST. 12 CI 1980 86.48 37 2.446		2	PVC-SCH40	2000	730.88	17	1.831	1995	7.0	6.831	47.82	HIGH	27.50%	\$	96,140 \$	78,	78,538
	COOK ST.		D	1980	86.48	37	2.446	1983	6.0	7.446	44.67	HIGH	27.53%	\$	22,752 \$	17,	17,188
12 Cl 1980 36.12 37 2.446	COOK ST.	. 12	ō	1980	36.12	37	2.446	1983	6.0	7.446	44.67	ндн	27.54%	\$	\$ 203'6	7,	7,179
22 ELM ST, 4 PVC-SCH40 1980 212.47 37 6.057 1978-198 ^a	ELM ST.		PVC-SCH40	1980	212.47	37	6.057	1978-1984	4.0	11.057	44.23	HIGH	27.59%	\$	27,949 \$	11,(11,020

					NISK SCOIIIIB OI WATER S		CILI DISCIDENCIOLI	שאינים שאינים	stem bistilbation of stem Assets and Estimated heptacement value/ carrent value	ישומרכוווכווור אם	ומכן כמווכוו	value		STREET, STREET		ができる。 では、 では、 では、 では、 では、 では、 では、 では、
GISID	DESCRIPTION	PIPE DIAMETE R	MATERIAL	YEAR INSTALLED	SHAPE	AGE (in 2017)	AGE_SCORE	HydrantAge	CONSEQUENCE	CONDITION	RISK	RISK_LEVEL	SUM TOTAL PERCENT (LENGTH)	Estima Replace (as d	Estimated Asset Replacement Value (as of 2015)	Estimated Asset Current Value (Based on
856	DATE ST.	9	AC	1960	335.88	57	7.176	1998	0.9	7.176	43.06	HIGH	30.67%	Ş	44,181 \$	12,476
861	E. THIRD AVE.	9	AC	1960	344.08	57	7.176	2013	6.0	7.176	43.06	HIGH	30.76%	\$	45,260 \$	12,780
863	MAGNOLIA ST.	4	AC	1960	721.01	57	7.176	1995	0.9	7.176	43.06	HIGH	30.95%	\$	94,842 \$	26,780
698	E. FIFTH AVE.	9	AC	1960	59.05	57	7.176	1993	6.0	7.176	43.06	HIGH	30.96%	\$	\$ 892'1	2,193
891	BROADWAY ST.	12	AC	1960	273.97	57	7.176	1983	6.0	7.176	43.06	HIGH	31.03%	\$	\$ 620,27	20,353
937	MAPLE ST.	9	AC	1960	38.29	57	7.176	1970	6.0	7.176	43.06	HIGH	31.04%	\$	\$ 75,037 \$	1,422
829		∞	PVC-SCH40	1991	174.72	26	3.455	1991	5.0	8.455	42.28	HIGH	31.09%	\$	30,645 \$	20,056
242	STEEL ST.	9	PVC-SCH40	1970	401.00	47	9.010	1976	3.0	14.010	42.03	HIGH	31.19%	\$	52,748 \$	5,223
625	E. THIRD AVE.	4	ō	1970	329.04	47	3.342	2009	5.0	8.342	41.71	HIGH	31.27%	\$	43,282 \$	28,818
633	W. SECOND AVE.	9	Ū	1970	78.51	47	3.342	2012	5.0	8.342	41.71	HIGH	31.29%	\$	10,327 \$	6,876
635	W. SIXTH AVE.	9	Ū	1970	298.62	47	3.342	1970	5.0	8.342	41.71	HIGH	31.37%	\$	\$ 082'68	26,154
643	E. SIXTH AVE.	9	D	1970	354.69	47	3.342	1991	5.0	8.342	41.71	HIGH	31.46%	\$	46,656 \$	31,065
645	W. THIRD AVE.	9	D	1970	143.37	47	3.342	1997	5.0	8.342	41.71	HIGH	31.50%	\$	18,858 \$	12,556
969	W. SIXTH AVE.	∞	D	1970	10.33	47	3.342	1970	5.0	8.342	41.71	HIGH	31.50%	\$	1,811 \$	1,206
831	E. SIXTH AVE.	9	D	1970	338.99	47 .	3.342	1991	5.0	8.342	41.71	HIGH	31.58%	\$	44,591 \$	29,690
325	RIVERSIDE DR.	9	PVC-SCH40	1992	405.60	25	3.253	1991	5.0	8.253	41.26	HIGH	31.69%	\$	\$ 83,353 \$	36,000
703	RIVERSIDE DR.	9	PVC-SCH40	1992	649.03	25	3.253	1991	5.0	8.253	41.26	HIGH	31.85%	\$	85,374 \$	909'25
916	PERCH ST.	9	PVC-SCH40	1992	1063.83	25	3.253	1991	5.0	8.253	41.26	нын	32.13%	\$	139,936 \$	94,422
54	RADIUM ST.	4	PVC-SCH40	2000	857.06	17	1.831	2005-2009	6.0	6.831	40.99	HIGH	32.34%	\$	112,737 \$	95,096
284	E. THIRD AVE.	9	PVC-SCH40	2000	348.48	17	1.831	1963	6.0	6.831	40.99	нівн	32.43%	\$	45,839 \$	37,446
382	W. SECOND AVE.	12	PVC-SCH40	2000	1303.41	17	1.831	2005	0.9	6.831	40.99	НЭІН	32.77%	\$	342,914 \$	280,130
096		0	PVC-SCH40	2000	3530.33	17	1.831	2003	6.0	6.831	40.99	HIGH	33.67%	\$	464,380 \$	379,356
935	E. SIXTH AVE.	9 .	ō	1990	35.69	27	1.650	1995	6.0	6.650	39.90	HIGH	33.68%	\$	4,695 \$	3,920
1112	<null></null>	∞	D	1991	1395.45	26	1.576	<null></null>	6.0	6.576	39.45	HIGH	34.03%	\$	244,748 \$	206,185
1113	<null></null>	∞	C	1991	183.08	26	1.576	<null></null>	0.9	6.576	39.45	HIGH	34.08%	\$	32,110 \$	27,050
1126	<null></null>	8	D	1991	189.62	26	1.576	<null></null>	0.9	6.576	39.45	НІВН	34.13%	\$	33,258 \$	28,018
1127	<null></null>	œ	ס	1991	354.47	26	1.576	<null></null>	6.0	6.576	39.45	HIGH	34.22%	\$	62,170 \$	52,375
365	DATE ST.	∞	PVC-SCH40	1970	137.95	47	9.010	1971	2.0	19.010	38.02	HIGH	34.25%	\$	24,196 \$	2,396
366	DATE ST.	8	PVC-SCH40	1970	159.95	47	9.010	1971	2.0	19.010	38.02	HIGH	34.29%	\$	28,054 \$	2,778
367	DATE ST.	8	PVC-SCH40	1970	95.40	47	9.010	1971	2.0	19.010	38.02	HIGH	34.32%	\$	16,732 \$	1,657
368	DATE ST.	8	PVC-SCH40	1970	296.37	47	9.010	1971	2.0	19.010	38.02	HIGH	34.39%	\$	51,981 \$	5,147
372	DATE ST.	8	PVC-SCH40	1970	142.98	47	9.010	1971	2.0	19.010	38.02	HIGH	34.43%	\$	\$ 820,25	2,483

isset lue	4,534	3,978	4,150	6,864	3,594	11,681	605'6	10,205	4,521	3,537	2,434	5,541	817	8,559	7,745	1,790	257	43,040	40,420	11,267	1,097	1,112	1,554	916	2,438	2,886	2,735	42,810	107,928	30,254	26,671	39,838
Estimated Assertant Value																Melan													10			
	45,793 \$	40,183 \$	41,916 \$	27,061 \$	14,170 \$	46,052 \$	72,557 \$	77,861 \$	34,497 \$	\$ 166,991	\$ 075,81	42,278 \$	6,232 \$	\$ 808,30	\$ 760,63	13,661 \$	911 \$	152,424 \$	143,144 \$	\$ 806'68	3,886 \$	3,937 \$	5,502 \$	3,242 \$	8,633 \$	\$ 022,01	4,316 \$	\$ 595'29	\$ 652,071	\$ 647,74	42,093 \$	62,874 \$
Estimated Asset Replacement Value	45	40	41	27	17	46	72	77	37	26	18	42	9	39	55	1		15.	143	36						10		.9	171	4.	4.	9
	\$	\$	\$	\$	\$	\$	\$	4	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$
SUM TOTAL PERCENT	34.50%	34.56%	34.62%	34.67%	34.70%	34.79%	34.89%	35.00%	35.05%	35.09%	35.12%	35.18%	35.19%	35.29%	35.37%	35.40%	35.40%	35.62%	35.83%	35.91%	35.92%	35.92%	35.93%	35.94%	35.96%	35.98%	35.99%	36.12%	36.45%	36.54%	36.62%	36.74%
RISK_LEVEL	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH
RISK	38.02	38.02	38.02	37.39	37.39	37.39	37.38	37.38	37.38	37.38	37.38	37.38	37.38	37.38	37.38	37.38	35.88	35.88	35.88	35.88	35.88	35.88	35.88	35.88	35.88	35.88	34.66	34.66	34.66	34.66	34.66	34.66
CONDITION	19.010	19.010	19.010	12.463	12.463	12.463	18.689	18.689	18.689	18.689	18.689	18.689	18.689	18.689	18.689	18.689	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	8.664	8.664	8.664	8.664	8.664	8.664
CONSEQUENCE	2.0	2.0	2.0	3.0	3.0	3.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	4.0	4.0	4.0	4.0	4.0	4.0
HydrantAge	1970	1971	1971	1975	1975	1975	1971	1971	1971	1971	1971	1971	1971	1971	1971	1971	1973	1984	1969	1998	1999	1970	1998	1998	1998	1998	1996	1995	1993	1994	1963	1995
AGE_SCORE	9.010	9.010	9.010	7.463	7.463	7.463	8.689	8.689	8.689	8.689	8.689	8.689	8.689	8.689	8.689	8,689	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	3.664	3.664	3.664	3.664	3.664	3.664
AGE (in 2017)	47	47	47	42	42	42	46	46	46	46	46	46	46	46	46	46	57	57	57	57	57	57	57	57	57	57	27	27	27	27	7.2	27
SHAPE	261.09	229.10	238.99	205.73	107.72	350.10	413.69	443.93	196.69	153.89	105.88	241.05	35.53	372.36	336.94	103.85	6.92	90.698	816.15	303.35	29.55	29.93	41.83	24.65	65.63	07.77	32.81	513.65	1294.96	363.00	320.00	477.98
YEAR USTALLED	1970	1970	1970	1975	1975	1975	1971	1971	1971	1971	1971	1971	1971	1971	1971	1971	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1990	1990	1990	1990	1990	1990
MATERIAL II	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40
PIPE DIAMETE R	8	8	8	9	9	9	8	8	8	8	8	8	8	8	8	4	9	8	8	9	4	9	9	4	4	4	4	4	4	9	9	9
PIPE VEAR DESCRIPTION DIAMETE MATERIAL INSTALLED R	DATE ST.	DATE ST.	DATE ST.	SILVER ST.	SILVER ST.		DATE ST.	DATE ST.	DATE ST.	DATE ST.	DATE ST.	DATE ST.	DATE ST.	DATE ST.	DATE ST.	OLIVO ST.	N. CEDAR ST.	PERSHING ST.	MORGAN ST.	DATE ST.	N. CEDAR ST.	W. SIXTH AVE.	DATE ST.	DATE ST.	DATE ST.	DATE ST.	W. FIFTH AVE.	TUNGSTEN ST.	STEEL ST.	FOCH ST.	E. FIRST AVE.	ALUMINUM ST.
GISTD	373	374	426	332	333	1052	360	361	362	363	364	422	089	681	682	884	260	651 F	723	847	849	850 V	857	668	006	901	۸ 9	57 T	63	193	200	229 A

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GIS ID	DESCRIPTION	PIPE DIAMETE R	MATERIAL	YEAR INSTALLED	SHAPE	AGE (in 2017)	AGE_SCORE	HydrantAge	CONSEQUENCE	CONDITION	RISK	RISK_LEVEL	PERCENT (LENGTH)	Estimat Replacem	Estimated Asset Replacement Value (as of 2015)	Estimated Asset Current Value (Based on	Asset 'alue on
230	VEATER ST.	9	PVC-SCH40	1990	180.14	27	3.664	1994	4.0	8.664	34.66	HIGH	36.79%	\$	\$ 569,65		15,013
231	TUNGSTEN ST.	9	PVC-SCH40	1990	563.00	27	3.664	1994	4.0	8.664	34.66	HIGH	36.93%	\$	74,057 \$		46,923
375	E. THIRD AVE.	9	PVC-SCH40	1990	321.13	7.2	3.664	1993	4.0	8.664	34.66	HIGH	37.02%	\$	42,241 \$		26,764
434	W. NINTH AVE.	9	PVC-SCH40	1990	3.26	27	3.664	1994	4.0	8.664	34.66	нен	37.02%	\$	429 \$		272
456	VEATER ST.	9	PVC-SCH40	1990	360.05	27	3.664	1994	4.0	8.664	34.66	HIGH	37.11%	\$	47,361 \$		30,008
619	FIR ST.	9	PVC-SCH40	1990	367.03	27	3.664	1997	4.0	8.664	34.66	HIGH	37.20%	\$	48,279 \$		30,590
989	W. NINTH AVE.	00	PVC-SCH40	1990	358.37	7.2	3.664	1994-1995	4.0	8.664	34.66	HIGH	37.29%	\$	62,854 \$		39,825
889	N. GRAPE ST.	9	PVC-SCH40	1990	464.93	27	3.664	1994	4.0	8.664	34.66	HIGH	37.41%	\$	61,156 \$		38,749
069	W. SIXTH AVE.	9	PVC-SCH40	1990	390.22	27	3.664	1993	4.0	8.664	34.66	HIGH	37.51%	\$	51,330 \$		32,523
756	N. GRAPE ST.	9	PVC-SCH40	1990	858.38	27	3.664	1994	4.0	8.664	34.66	нын	37.73%	\$	112,912 \$		71,542
810	ORE ST.	4	PVC-SCH40	1990	609.25	27	3.664	1976	4.0	8.664	34.66	HIGH	37.89%	\$	80,140 \$		50,778
812	KOPRA ST.	9	PVC-SCH40	1990	720.59	27	3.664	1984	4.0	8.664	34.66	HIGH	38.07%	\$	\$ 98,786 \$		850,09
1002		2	PVC-SCH40	1990	404.81	27	3.664	1995	4.0	8.664	34.66	HIGH	38.17%	\$	53,249 \$		33,739
1004		0.75	PVC-SCH40	1990	328.94	27	3.664	1998	4.0	8.664	34.66	нен	38.26%	\$	43,269 \$		27,415
1036		8	PVC-SCH40	1990	967.50	27	3.664	1998	4.0	8.664	34.66	HIGH	38.50%	\$	169,691 \$		107,518
511	DATE ST.	9	AC	1960	542.79	57	7.176	1970-1971	2.0	17.176	34.35	HIGH	38.64%	\$	71,399 \$		20,161
512	DATE ST.	9	AC	1960	514.32	57	7.176	1970-1971	2.0	17.176	34.35	HIGH	38.77%	\$	67,654 \$		19,103
1065		2	PVC-SCH40	2000	374.04	17	1.831	2004	5.0	6.831	34.15	HIGH	38.87%	\$	49,201 \$		40,193
262	MARSHALL ST.	9	ס	1970	188.52	47	3.342	1976	4.0	8.342	33.37	HBIH	38.92%	\$	24,797 \$		16,511
632	LANE ST.	9	IJ	1970	354.32	47	3.342	1997	4.0	8.342	33.37	HIGH	39.01%	\$	\$ 46,607 \$		31,033
639	E. SECOND AVE.	9	O	1970	227.21	47	3.342	2012	4.0	8.342	33.37	HIGH	39.07%	\$	\$ 788,62		19,900
640	E. SECOND AVE.	9	D	1970	330.07	47	3.342	2012	4.0	8.342	33.37	HIGH	39.15%	\$	43,418 \$		28,909
732	W. SECOND AVE.	9	ū	1970	342.99	47	3.342	2012	4.0	8.342	33.37	HIGH	39.24%	\$	45,116 \$	10	30,040
826	E. SIXTH AVE.	9	D	1970	351.78	47	3.342	1988	4.0	8.342	33.37	HIGH	39.33%	\$	46,273	\$	30,810
929	POST ST.	9	PVC-SCH40	2010	446.04	7	0.558	2010-2011	6.0	5.558	33.35	нівн	39.44%	\$	58,672	\$	55,399
240	ZINC ST.	9	PVC-SCH40	1980	440.51	37	6.057	1980	3.0	11.057	33.17	HIGH	39.55%	\$	\$ 57,945 \$	10	22,848
301	E. FIFTH AVE.	9	PVC-SCH40	1980	192.03	37	6.057	1981	3.0	11.057	33.17	HIGH	39.60%	\$	25,259 \$	\$	096'6
869	E. SIXTH AVE.	9	PVC-SCH40	1980	308.43	37	6.057	1988	3.0	11.057	33.17	HIGH	39.68%	\$	40,571 \$		15,998
721	VEATER ST.	9	PVC-SCH40	1980	214.36	37	6.057	1981	3.0	11.057	33.17	HIGH	39.74%	\$	28,197	\$	11,118
275	RIVERSIDE DR.	9	PVC-SCH40	1992	719.39	25	3.253	2000	4.0	8.253	33.01	HIGH	39.92%	\$	94,629	\$	63,851
276	RIVERSIDE DR.	9	PVC-SCH40	1992	51.92	25	3.253	2000	4.0	8.253	33.01	HIGH	39.93%	\$	6,829	\$	4,608
278	RIVERSIDE DR.	9	PVC-SCH40	1992	21.21	25	3.253	2000	4.0	8.253	33.01	HIGH	39.94%	\$	2,790 \$	\$	1,882

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6 AC 1860 652.18 57 7,176 1899 40 7,176 2871 HIGH 4407% 5 6 AC 1860 32.1 57 7,176 1896 71.7 28.1 HIGH 4417% 5 6 AC 1860 342.2 57 7,176 1896 71.7 28.1 HIGH 4417% 5 6 AC 1860 443.2 57 7,176 1891 71.7 28.1 HIGH 443.0% 5 6 AC 1860 443.2 57 7,176 1891 71.76 28.1 HIGH 443.0% 5 4 PUCSCHMO 1970 1978 20.0 170 28.0 140 71.76 28.1 HIGH 443.0% 5 4 PUCSCHMO 1970 1978 20.0 140.0 28.0 HIGH 443.0% 5 4 PUCSCHMO 1970 1978 <th></th> <th></th> <th>DIAMETE R</th> <th></th> <th>YEAR</th> <th>SHAPE</th> <th>AGE (in 2017)</th> <th>AGE_SCORE</th> <th>HydrantAge</th> <th>CONSEQUENCE</th> <th>CONDITION</th> <th>RISK SCORE</th> <th>RISK_LEVEL</th> <th>PERCENT (LENGTH)</th> <th>Replacemen (as of 20</th> <th>ne L</th> <th>Current Value (Based on</th>			DIAMETE R		YEAR	SHAPE	AGE (in 2017)	AGE_SCORE	HydrantAge	CONSEQUENCE	CONDITION	RISK SCORE	RISK_LEVEL	PERCENT (LENGTH)	Replacemen (as of 20	ne L	Current Value (Based on
6 AC 1990 52.17 57.7 11950 52.17 1176 1995-98 4.0 7176 287.1 HIGH 4407% 5 4 AC 1960 34436 57 7176 1991 37.1 HIGH 44.2% 5 4 AC 1960 34436 57 7176 1991 37.1 HIGH 44.2% 5 4 AC 1960 1403 57 7176 1991 40 7176 287.1 HIGH 44.2% 5 7 4 PVCSCH40 1970 3600 47 9010 1978 20 1400 280.7 HIGH 44.7% 5 7 4 PVCSCH40 1970 3010 1978 20 1400 280.7 HIGH 44.7% 5 7 4 PVCSCH40 1970 3010 1978 20 1400 280.7 HIGH 44.7% 5 7		MORGAN ST.	∞	AC	1960	652.18	57	7.176	1969	4.0	7.176	28.71	HIGH	44.05%		114,386 \$	32,299
6 AC 1960 414.26 57 7176 1996-1998 40 7176 2871 HIGH 44.17% 5 4 AC 1960 413.48 57 7176 1991 410 412.98 5 7 7176 1990 7176 1990 412.98 5 7 7176 1990 7176 2871 HIGH 447.7% 5 2 4 AC 1960 1950.20 57 7176 1998 40 7176 2871 HIGH 447.7% 5 2 4 PVCSCH40 1970 50.00 1978 2.0 1400 2802 HIGH 447.7% 5 2 4 PVCSCH40 1970 50.00 1978 2.0 1400 2802 HIGH 447.7% 5 2 4 PVCSCH40 1970 50.00 1978 2.0 1400 2802 HIGH 447.7% 5 2		HYDE ST.	9	AC	1960	52.17	57	7.176	1998	4.0	7.176	28.71	HIGH	44.07%	\$	\$ 898'9	1,938
4 AC 1960 33748 57 7176 1981 40 7175 2871 HIGH 44.26% 5 4 AC 1960 4373 57 7176 1989 40 7176 2871 HIGH 44.26% 5 4 AC 1960 1950 57 7176 2009 7176 2871 HIGH 44.26% 5 4 PVC-SCH40 1970 606-69 47 9010 1978 20 14010 28.02 HIGH 44.26% 5 4 PVC-SCH40 1970 606-69 47 9010 1978 20 14010 28.02 HIGH 45.03% 5 4 PVC-SCH40 1970 1978 20 14010 28.02 HIGH 45.03% 5 4 PVC-SCH40 1970 1972 20 14010 28.02 HIGH 45.03% 5 4 PVC-SCH40 1970 <		HYDE ST.	9	AC	1960	414.26	57	7.176	1996-1998	4.0	7.176	28.71	HIGH	44.17%	\$	54,492 \$	15,387
4 AC 1960 4103 57 7,176 2009 40 7,176 2801 140H 44,27% 5 4 AC 1960 1950,20 57 7,176 1984 40 7,176 28,20 14010 28,20 HIGH 44,87% 5 4 PVCSCH40 1970 696.60 47 9010 1978 20 14010 28,02 HIGH 44,87% 5 4 PVCSCH40 1970 696.60 47 9010 1978 20 14010 28,02 HIGH 45,02% 5 4 PVCSCH40 1970 914 47 9010 1978 20 14010 28,02 HIGH 45,03% 5 4 PVCSCH40 1970 914 47 9010 1975 20 14010 28,02 HIGH 45,13% 5 4 PVCSCH40 1970 314 9010 1975 20 14010		E. FIFTH AVE.	4	AC	1960	337.48	57	7.176	1981	4.0	7.176	28.71	HIGH	44.26%	\$	44,392 \$	12,535
6 AC 1960 AC 1716 1984 4 0 7176 2871 HIGH 4444% \$ 4 PVC-SCH40 1970 6866 47 9100 1978 2.0 14010 288.02 HIGH 4444% \$ 4 PVC-SCH40 1970 6866 47 9100 1978 2.0 14010 288.02 HIGH 45.08% \$ 4 PVC-SCH40 1970 226.31 47 9100 2013 2.0 14010 28.02 HIGH 45.08% \$ 4 PVC-SCH40 1970 2010 1978 2.0 14010 28.02 HIGH 45.08% \$ 4 PVC-SCH40 1970 2010 1978 2.0 14010 28.02 HIGH 45.14% \$ 4 PVC-SCH40 1970 2010 1978 2.0 14010 28.02 HIGH 45.04% \$ 4 PVC-SCH40 <		ASH ST.	4	AC	1960	41.03	57	7.176	2009	4.0	7.176	28.71	HIGH	44.27%	\$	\$ 268'5	1,524
4 PVC-SCH40 1970 306.07 47 9010 1978 20 14010 280.2 HIGH 4484% 5 4 PVC-SCH40 1970 666.9 47 9010 1978 2.0 14010 280.2 HIGH 4502% 5 4 PVC-SCH40 1970 50.2 47 9010 2013 2.0 14010 280.2 HIGH 4502% 5 4 PVC-SCH40 1970 30.2 47 9010 2013 2.0 14010 280.2 HIGH 45.00% 5 4 PVC-SCH40 1970 13.6 47 9010 1973 2.0 14010 280.2 HIGH 45.00% 5 4 PVC-SCH40 1970 19.0 1975 2.0 14010 280.2 HIGH 45.13% 5 4 PVC-SCH40 1970 1972 20.3 14010 280.2 HIGH 45.13% 5	A 150 (17 (17))	W. EIGHTH AVE.	9	AC	1960	1950.20	57	7.176	1984	4.0	7.176	28.71	HIGH	44.77%		\$ 085,930	72,436
4 PVCSCH40 1970 69669 47 9101 1978 20 14010 2802 HIGH 4502% 5 4 PVCSCH40 1970 265.31 47 9010 2013 20 14010 2802 HIGH 4508% 5 4 PVCSCH40 1970 1970 47 9010 2013 20 14010 2802 HIGH 4508% 5 4 PVCSCH40 1970 1914 47 9010 2013 20 14010 2802 HIGH 4513% 5 4 PVCSCH40 1970 1914 47 9010 1975 20 14010 2802 HIGH 4513% 5 4 PVCSCH40 1970 1916 1972 1901 1972 14010 2802 HIGH 4513% 5 4 PVCSCH40 1970 1972 1972 1971 1971 20 14010 2802 HIGH	* PRESIDENCE	E. SECOND AVE.	4	PVC-SCH40	1970	306.07	47	9.010	1978	2.0	14.010	28.02	HIGH	44.84%	\$	40,261 \$	3,986
4 PVCSCH40 1970 226.31 47 9010 2013 2.0 14.010 28.02 HIGH 45.08% \$ 4 PVCSCH40 1970 9.02 47 9010 1978 2.0 14010 28.02 HIGH 45.08% \$ 4 PVCSCH40 1970 13.66 47 9010 2013 2.0 14010 28.02 HIGH 45.12% \$ 4 PVCSCH40 1970 13.96 47 9010 1975 2.0 14010 28.02 HIGH 45.12% \$ 4 PVCSCH40 1970 13.96 47 9010 1975 2.0 14010 28.02 HIGH 45.12% \$ 4 PVCSCH40 1970 1378 2.0 14010 28.02 HIGH 45.13% \$ 4 PVCSCH40 1970 1378 2.0 14010 28.02 HIGH 45.13% \$ 4 <t< td=""><td>18 NOTES (1979)</td><td>LUCKY ST.</td><td>4</td><td>PVC-SCH40</td><td>1970</td><td>69.969</td><td>47</td><td>9.010</td><td>1978</td><td>2.0</td><td>14.010</td><td>28.02</td><td>HIGH</td><td>45.02%</td><td>\$</td><td>91,643 \$</td><td>9,074</td></t<>	18 NOTES (1979)	LUCKY ST.	4	PVC-SCH40	1970	69.969	47	9.010	1978	2.0	14.010	28.02	HIGH	45.02%	\$	91,643 \$	9,074
4 PVCSCH40 1370 9.02 47 9.010 1978 2.0 14.010 28.02 HIGH 45.08% \$ 4 PVCSCH40 1370 13.66 47 9.010 2013 2.0 14.010 28.02 HIGH 45.12% \$ 4 PVCSCH40 1370 71.34 47 9.010 2013 2.0 14.010 28.02 HIGH 45.13% \$ 4 PVCSCH40 1370 314.86 47 9.010 1375 2.0 14.010 28.02 HIGH 45.13% \$ 4 PVCSCH40 1370 314.86 47 9.010 1372 2.0 14.010 28.02 HIGH 45.13% \$ 4 PVCSCH40 1370 214.90 20.00 1378 2.0 14.010 28.02 HIGH 45.13% \$ 4 PVCSCH40 1370 21.00 1378 2.0 14.010 28.02 HIGH <	ALC: YES	CHERRY LN.	4	PVC-SCH40	1970	226.31	47	9.010	2013	2.0	14.010	28.02	HIGH	45.08%	\$	\$ 692,62	2,947
4 PVC-SCH40 1370 1366 47 9010 2013 20 14010 2802 HIGH 45.13% \$ 4 PVC-SCH40 1370 71.94 47 9010 2013 20 14010 2802 HIGH 45.13% \$ 4 PVC-SCH40 1370 31.48 47 9010 1372 20 14010 28.02 HIGH 45.13% \$ 4 PVC-SCH40 1370 314.86 47 9010 1372-1375 20 14010 28.02 HIGH 45.13% \$ 4 PVC-SCH40 1370 314.86 47 9010 1372-1375 20 14010 28.02 HIGH 45.13% \$ 4 PVC-SCH40 1370 314.86 47 9010 1378 20 14010 28.02 HIGH 45.13% \$ 4 PVC-SCH40 1370 21.40 30.10 1378 20 14010 28.0	100000	LUCKY ST.	4	PVC-SCH40	1970	9.02	47	9.010	1978	2.0	14.010	28.02	HIGH	45.08%	\$	1,187 \$	117
4 PVC-SCH40 1970 71.94 47 9010 2013 2.0 14.010 28.02 HIGH 45.13% \$ 4 PVC-SCH40 1970 9.14 47 9.010 1975 2.0 14.010 28.02 HIGH 45.13% \$ 4 PVC-SCH40 1970 314.86 47 9.010 1972 28.02 HIGH 45.13% \$ 4 PVC-SCH40 1970 314.86 47 9.010 1972 28.02 HIGH 45.13% \$ 4 PVC-SCH40 1970 234.90 47 9.010 1978 2.0 14.010 28.02 HIGH 45.23% \$ 4 PVC-SCH40 1970 24.9 9.010 1978 2.0 14.010 28.02 HIGH 45.33% \$ 4 PVC-SCH40 1970 218.9 47 9.010 1978 2.0 14.010 28.02 HIGH 45.34% \$ <td>Market St</td> <td>E. FOURTH AVE.</td> <td>4</td> <td>PVC-SCH40</td> <td>1970</td> <td>136.66</td> <td>47</td> <td>9.010</td> <td>2013</td> <td>2.0</td> <td>14.010</td> <td>28.02</td> <td>HIGH</td> <td>45.12%</td> <td>\$</td> <td>\$ 976,71</td> <td>1,780</td>	Market St	E. FOURTH AVE.	4	PVC-SCH40	1970	136.66	47	9.010	2013	2.0	14.010	28.02	HIGH	45.12%	\$	\$ 976,71	1,780
4 PVC-SCH40 1970 9.14 47 9.010 1975 2.0 14 010 28.02 HIGH 45.17% \$ 4 PVC-SCH40 1970 139.56 47 9.010 1972-1975 2.0 14.010 28.02 HIGH 45.17% \$ 4 PVC-SCH40 1970 33.48 47 9.010 1978-1975 2.0 14.010 28.02 HIGH 45.17% \$ 4 PVC-SCH40 1970 33.48 47 9.010 1978 2.0 14.010 28.02 HIGH 45.17% \$ 4 PVC-SCH40 1970 134.90 47 9.010 1978 2.0 14.010 28.02 HIGH 45.13% \$ 4 PVC-SCH40 1970 136.0 47 9.010 1978 2.0 14.010 28.02 HIGH 45.13% \$ 5 PVC-SCH40 1970 21.0 1978 2.0 14.010 28.02 </td <td>196/59</td> <td>CHERRY LN.</td> <td>4</td> <td>PVC-SCH40</td> <td>1970</td> <td>71.94</td> <td>47</td> <td>9.010</td> <td>2013</td> <td>2.0</td> <td>14.010</td> <td>28.02</td> <td>HIGH</td> <td>45.13%</td> <td>\$</td> <td>9,463 \$</td> <td>937</td>	196/59	CHERRY LN.	4	PVC-SCH40	1970	71.94	47	9.010	2013	2.0	14.010	28.02	HIGH	45.13%	\$	9,463 \$	937
4 PVCSCH40 1970 139.56 47 9.010 1975 2.0 14010 28.02 HIGH 45.17% \$ 4 PVCSCH40 1970 314.86 47 9.010 1972-1975 2.0 14010 28.02 HIGH 45.25% \$ 4 PVCSCH40 1970 33.48 47 9.010 1978 2.0 14.010 28.02 HIGH 45.25% \$ 4 PVCSCH40 1970 120.00 47 9.010 1978 2.0 14.010 28.02 HIGH 45.25% \$ 4 PVCSCH40 1970 120.00 47 9.010 1978 2.0 14.010 28.02 HIGH 45.23% \$ 5 PVCSCH40 1970 1978 2.0 14.010 28.02 HIGH 45.23% \$ 6 PVCSCH40 1970 1978 2.0 14.010 28.02 HIGH 45.23% \$ <td< td=""><td>97,055</td><td></td><td>4</td><td>PVC-SCH40</td><td>1970</td><td>9.14</td><td>47</td><td>9.010</td><td>1975</td><td>2.0</td><td>14.010</td><td>28.02</td><td>HIGH</td><td>45.14%</td><td>\$</td><td>1,202 \$</td><td>119</td></td<>	97,055		4	PVC-SCH40	1970	9.14	47	9.010	1975	2.0	14.010	28.02	HIGH	45.14%	\$	1,202 \$	119
4 PVC-SCH40 1970 314.86 47 9.010 1972-1975 2.0 14.010 28.02 HIGH 45.25% \$ 4 PVC-SCH40 1970 53.98 47 9.010 1972-1975 2.0 14.010 28.02 HIGH 45.27% \$ 4 PVC-SCH40 1970 136.60 47 9.010 1978 2.0 14.010 28.02 HIGH 45.33% \$ 4 PVC-SCH40 1970 116.60 47 9.010 2013 2.0 14.010 28.02 HIGH 45.33% \$ 4 PVC-SCH40 1970 126.60 47 9.010 1978 2.0 14.010 28.02 HIGH 45.33% \$ 5 PVC-SCH40 1970 126.60 47 9.010 1978 2.0 14.010 28.02 HIGH 45.23% \$ 6 PVC-SCH40 1970 21.0 1978 2.0 14.010 28.	13 5 50	E. FOURTH AVE.	4	PVC-SCH40	1970	139.56	47	9.010	1975	2.0	14.010	28.02	HIGH	45.17%	\$	18,357 \$	1,818
4 PVC-SCH40 1970 53.38 47 9,010 1978 2.0 14,010 28.02 HIGH 45.27% \$ 4 PVC-SCH40 1970 234.90 47 9,010 1978 2.0 14,010 28.02 HIGH 45.33% \$ 4 PVC-SCH40 1970 116.60 47 9,010 2013 2.0 14,010 28.02 HIGH 45.33% \$ 4 PVC-SCH40 1970 126.00 47 9,010 1978 2.0 14,010 28.02 HIGH 45.38% \$ 5 PVC-SCH40 1970 126.90 47 9,010 1978 2.0 14,010 28.02 HIGH 45.41% \$ 6 PVC-SCH40 1970 19.01 1978 2.0 14,010 28.02 HIGH 45.43% \$ 6 PVC-SCH40 1970 1972 1970 1970 1970 1970 1970 14,010	10700	GARNETT ST.	4	PVC-SCH40	1970	314.86	47	9.010	1972-1975	2.0	14.010	28.02	HIGH	45.25%	\$	41,417 \$	4,101
4 PVC-SCH40 1970 24.0 40.0 14.010 28.02 HIGH 45.33% \$ 4 PVC-SCH40 1970 116.60 47 9.010 2013 2.0 14.010 28.02 HIGH 45.36% \$ 4 PVC-SCH40 1970 116.60 47 9.010 1978 2.0 14.010 28.02 HIGH 45.33% \$ 4 PVC-SCH40 1970 22.0 14.010 28.02 HIGH 45.41% \$ 6 PVC-SCH40 1970 22.0 14.010 28.02 HIGH 45.43% \$ 6 PVC-SCH40 1970 218.0 1970 1975 2.0 14.010 28.02 HIGH 45.43% \$ 6 PVC-SCH40 1970 218.0 1970 1975 2.0 14.010 28.02 HIGH 45.43% \$ 7 PVC-SCH40 1970 218.0 14.010 28.02 HIGH <td>BOUGH B</td> <td>SILVER ST.</td> <td>4</td> <td>PVC-SCH40</td> <td>1970</td> <td>53.98</td> <td>47</td> <td>9.010</td> <td>1978</td> <td>2.0</td> <td>14.010</td> <td>28.02</td> <td>HIGH</td> <td>45.27%</td> <td>\$</td> <td>7,100 \$</td> <td>703</td>	BOUGH B	SILVER ST.	4	PVC-SCH40	1970	53.98	47	9.010	1978	2.0	14.010	28.02	HIGH	45.27%	\$	7,100 \$	703
4 PVC-SCH40 1970 116.60 47 9,010 2013 2.0 14,010 28.02 HIGH 45.38% 5 4 PVC-SCH40 1970 120.00 47 9,010 1978 2.0 14,010 28.02 HIGH 45.39% 5 6 PVC-SCH40 1970 72.69 47 9,010 1978 2.0 14,010 28.02 HIGH 45.43% 5 6 PVC-SCH40 1970 290.48 47 9,010 1978 2.0 14,010 28.02 HIGH 45.54% 5 6 PVC-SCH40 1970 290.48 47 9,010 1975 2.0 14,010 28.02 HIGH 45.54% 5 7 PVC-SCH40 1970 290.48 47 9,010 1978 2.0 14,010 28.02 HIGH 45.54% 5 8 PVC-SCH40 1970 290.48 47 9,010 2012 2.0	HE WED	SILVER ST.	4	PVC-SCH40	1970	234.90	47	9.010	1978	2.0	14.010	28.02	HIGH	45.33%	\$	\$ 868'08	3,059
4 PVC-SCH40 1970 120.00 47 9.010 1978 2.0 14.010 28.02 HIGH 45.39% \$ 4 PVC-SCH40 1970 72.69 47 9.010 1978 2.0 14.010 28.02 HIGH 45.43% \$ 6 PVC-SCH40 1970 290.48 47 9.010 1978 2.0 14.010 28.02 HIGH 45.49% \$ 6 PVC-SCH40 1970 290.48 47 9.010 1975 2.0 14.010 28.02 HIGH 45.49% \$ 7 PVC-SCH40 1970 268.98 47 9.010 1978 2.0 14.010 28.02 HIGH 45.54% \$ 8 PVC-SCH40 1970 268.99 47 9.010 2012 2.0 14.010 28.02 HIGH 45.49% \$ 8 PVC-SCH40 1971 26.1 20 14.010 28.02 HIGH		CHERRY LN.	4	PVC-SCH40	1970	116.60	47	9.010	2013	2.0	14.010	28.02	HIGH	45.36%	\$	15,338 \$	1,519
4 PVC-SCH40 1970 72.69 47 9.010 1978 2.0 14.010 28.02 HIGH 45.41% \$ 6 PVC-SCH40 1970 318.95 47 9.010 1978 2.0 14.010 28.02 HIGH 45.49% \$ 6 PVC-SCH40 1970 290.48 47 9.010 1975 2.0 14.010 28.02 HIGH 45.56% \$ 4 PVC-SCH40 1970 268.98 47 9.010 1978 2.0 14.010 28.02 HIGH 45.56% \$ 7 PVC-SCH40 1970 268.98 47 9.010 2012 2.0 14.010 28.02 HIGH 45.57% \$ 8 PVC-SCH40 1971 37.1.60 46 8.689 1971 2.0 14.010 28.02 HIGH 45.78% \$ 8 PVC-SCH40 1971 46 8.689 1971 2.0 13.689	TOTAL PROPERTY.	E. SECOND AVE.	4	PVC-SCH40	1970	120.00	47	9.010	1978	2.0	14.010	28.02	HIGH	45.39%	\$	15,785 \$	1,563
6 PVC-SCH40 1970 318.95 47 9.010 1978 2.0 14.010 28.02 HIGH 45.49% \$ 6 PVC-SCH40 1970 290.48 47 9.010 1975 2.0 14.010 28.02 HIGH 45.56% \$ 4 PVC-SCH40 1970 296.89 47 9.010 1978 2.0 14.010 28.02 HIGH 45.57% \$ 2 PVC-SCH40 1970 268.98 47 9.010 2012 2.0 14.010 28.02 HIGH 45.57% \$ 2 PVC-SCH40 1971 37.16 47 9.010 2012 2.0 14.010 28.02 HIGH 45.72% \$ 8 PVC-SCH40 1971 20.1 2.0 14.010 28.02 HIGH 45.84% \$ 8 PVC-SCH40 1971 2.0 2.0 14.010 28.02 HIGH 45.84% \$		SILVER ST.	4	PVC-SCH40	1970	72.69	47	9.010	1978	2.0	14.010	28.02	HIGH	45.41%	\$	9,561 \$	947
6 PVC-SCH40 1970 2010 1975 2.0 14.010 28.02 HIGH 45.56% \$ 4 PVC-SCH40 1970 19.55 47 9.010 1975 2.0 14.010 28.02 HIGH 45.57% \$ 2 PVC-SCH40 1970 268.98 47 9.010 2012 2.0 14.010 28.02 HIGH 45.64% \$ 2 PVC-SCH40 1970 348.43 47 9.010 2012 2.0 14.010 28.02 HIGH 45.64% \$ 8 PVC-SCH40 1971 2.0 14.010 28.02 HIGH 45.72% \$ 8 PVC-SCH40 1971 2.0 13.689 1971 2.0 13.689 27.38 HIGH 45.84% \$ 8 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.85% \$ 8 PVC-SCH40 1971 2.0 13.689		E. FOURTH AVE.	9	PVC-SCH40	1970	318.95	47	9.010	1978	2.0	14.010	28.02	HIGH	45.49%	\$	41,955 \$	4,154
6 PVC-SCH40 1970 1975 2.0 14,010 28.02 HIGH 45.57% \$ 4 PVC-SCH40 1970 268.98 47 9.010 1978 2.0 14,010 28.02 HIGH 45.64% \$ 2 PVC-SCH40 1970 348.43 47 9.010 2012 2.0 14,010 28.02 HIGH 45.64% \$ 8 PVC-SCH40 1971 371.60 46 8.689 1971 2.0 14,010 28.02 HIGH 45.75% \$ 8 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.75% \$ 8 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.85% \$ 8 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.85% \$ 8 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.92% \$ <	A REPORT OF	E. FOURTH AVE.	9	PVC-SCH40	1970	290.48	47	9.010	1975	2.0	14.010	28.02	HIGH	45.56%	\$	38,210 \$	3,783
4 PVC-SCH40 1970 268.98 47 9.010 1978 2.0 14.010 28.02 HIGH 45.64% \$ 2 PVC-SCH40 1970 348.43 47 9.010 2012 2.0 14.010 28.02 HIGH 45.72% \$ 8 PVC-SCH40 1970 95.46 47 9.010 2012 2.0 14.010 28.02 HIGH 45.72% \$ 8 PVC-SCH40 1971 28.3 1971 2.0 13.689 27.38 HIGH 45.85% \$ 8 PVC-SCH40 1971 28.689 1971 2.0 13.689 27.38 HIGH 45.85% \$ 8 PVC-SCH40 1971 253.26 46 8.689 1971 2.0 13.689 27.38 HIGH 45.92% \$ 8 PVC-SCH40 1971 253.26 46 8.689 1971 2.0 13.689 27.38 HIGH 45.94%	Bally and	E. FOURTH AVE.	9	PVC-SCH40	1970	19.55	47	9.010	1975	2.0	14.010	28.02	HIGH	45.57%	\$	2,571 \$	255
2 PVC-SCH40 1970 348.43 47 9.010 2012 2.0 14.010 28.02 HIGH 45.72% \$ 2 PVC-SCH40 1970 95.46 47 9.010 2012 2.0 14.010 28.02 HIGH 45.75% \$ 8 PVC-SCH40 1971 371.60 46 8.689 1971 2.0 13.689 27.38 HIGH 45.84% \$ 8 PVC-SCH40 1971 11.86 46 8.689 1971 2.0 13.689 27.38 HIGH 45.85% \$ 9 PVC-SCH40 1971 12.0 13.689 27.38 HIGH 45.85% \$ 9 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.85% \$ 9 PVC-SCH40 1971 103.42 46 8.689 1971 2.0 13.689 27.38 HIGH 45.92% \$ 9 PVC-SCH40	1000000	LUCKY ST.	4	PVC-SCH40	1970	268.98	47	9.010	1978	2.0	14.010	28.02	HIGH	45.64%	\$	35,381 \$	3,503
2 PVC-SCH40 1970 95.46 47 9.010 2012 2.0 14.010 28.02 HIGH 45.75% \$ 8 PVC-SCH40 1971 37.160 46 8.689 1971 2.0 13.689 27.38 HIGH 45.85% \$ 8 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.85% \$ 9 PVC-SCH40 1971 11.86 8.689 1971 2.0 13.689 27.38 HIGH 45.85% \$ 9 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.92% \$ 9 PVC-SCH40 1971 103.42 46 8.689 1971 2.0 13.689 27.38 HIGH 45.92% \$ 4 PVC-SCH40 1971 103.42 46 8.689 1971 2.0 13.689 27.38 HIGH 45.94% \$ 4 PVC-SCH40 1971	14/14/8/9		2	PVC-SCH40	1970	348.43	47	9.010	2012	2.0	14.010	28.02	HIGH	45.72%	\$	45,832 \$	4,538
8 PVC-SCH40 1971 371.60 46 8.689 1971 2.0 13.689 27.38 HIGH 45.84% \$ 8 PVC-SCH40 1971 28.34 46 8.689 1971 2.0 13.689 27.38 HIGH 45.85% \$ 8 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.85% \$ 9 PVC-SCH40 1971 2.0 13.689 27.38 HIGH 45.92% \$ 9 PVC-SCH40 1971 103.42 46 8.689 1971 2.0 13.689 27.38 HIGH 45.92% \$ 4 PVC-SCH40 1971 103.42 46 8.689 1971 2.0 13.689 27.38 HIGH 45.94% \$ 4 PVC-SCH40 1971 103.42 46.08 46.08 \$ \$			2	PVC-SCH40	1970	95.46	47	9.010	2012	2.0	14.010	28.02	HIGH	45.75%	\$	12,557 \$	1,243
8 PVC-SCH40 1971 28.34 46 8.689 1971 2.0 13.689 27.38 HIGH 45.85% \$ 8 PVC-SCH40 1971 11.86 46 8.689 1971 2.0 13.689 27.38 HIGH 45.85% \$ 7 PVC-SCH40 1971 253.26 46 8.689 1971 2.0 13.689 27.38 HIGH 45.92% \$ 8 PVC-SCH40 1971 103.42 46 8.689 1971 2.0 13.689 27.38 HIGH 45.92% \$ 4 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 7 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 18.31 2003 4.0 6.831 27.32 HIGH 46.08% \$ 8 PVC-SCH40 2000 549.43 17 18.31 2003 4.0 6	1883 23		∞	PVC-SCH40	1971	371.60	46	8.689	1971	2.0	13.689	27.38	HIGH	45.84%	\$	65,176 \$	8,542
8 PVC-SCH40 1971 11.86 46 8.689 1971 2.0 13.689 27.38 HIGH 45.85% \$ 8 PVC-SCH40 1971 253.26 46 8.689 1971 2.0 13.689 27.38 HIGH 45.92% \$ 9 PVC-SCH40 1971 103.42 46 8.689 1971 2.0 13.689 27.38 HIGH 45.92% \$ 14 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$	THE STATE OF		8	PVC-SCH40	1971	28.34	46	8.689	1971	2.0	13.689	27.38	HIGH	45.85%	\$	4,971 \$	651
8 PVC-SCH40 1971 253.26 46 8.689 1971 2.0 13.689 27.38 HIGH 45.92% \$ 8 PVC-SCH40 1971 103.42 46 8.689 1971 2.0 13.689 27.38 HIGH 45.94% \$ 4 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$	100000000000000000000000000000000000000		∞	PVC-SCH40	1971	11.86	46	8.689	1971	2.0	13.689	27.38	HIGH	45.85%	\$	2,080 \$	273
8 PVC-SCH40 1971 103.42 46 8.689 1971 2.0 13.689 27.38 HIGH 45.94% \$ 4 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$	* CONTRACTOR		8	PVC-SCH40	1971	253.26	46	8.689	1971	2.0	13.689	27.38	HIGH	45.92%	\$	44,418 \$	5,821
4 PVC-SCH40 2000 549.43 17 1.831 2003 4.0 6.831 27.32 HIGH 46.08% \$	No. OLG	DATE ST.	∞	PVC-SCH40	1971	103.42	46	8.689	1971	2.0	13.689	27.38	HIGH	45.94%	\$	18,138 \$	2,377
	A TRANSPORTER	LEAD ST.	4	PVC-SCH40	2000	549.43	17	1.831	2003	4.0	6.831	27.32	HIGH	46.08%	\$	72,272 \$	59,040

APPENDIX A Risk Scoring of Water System Distribution System Assets and Estimated Replacement Value/Current Value

YEAR		SHAPE AC	AGE (in	AGE SCADE	Wirdram+Ago	CONSEGUENCE	CONDITION	VICIN	DICK LEVEL	DEDCENIT	Poplacoment Value		Estimated Asset
	#		2017)	AGE_SCORE	nyuranınge	SCORE	SCORE	SCORE	NON_LEVEL	(LENGTH)	(as of 2015	2015)	(Based on
2000 5		545.00	17	1.831	2003	4.0	6.831	27.32	HIGH	46.22%	\$	71,690 \$	58,564
2000 14	<t< td=""><td>144.53</td><td>17</td><td>1.831</td><td>2003</td><td>4.0</td><td>6.831</td><td>27.32</td><td>HIGH</td><td>46.26%</td><td>\$</td><td>\$ 110,61</td><td>15,531</td></t<>	144.53	17	1.831	2003	4.0	6.831	27.32	HIGH	46.26%	\$	\$ 110,61	15,531
2000 224	24	224.20	17	1.831	2003	4.0	6.831	27.32	HIGH	46.32%	\$	29,492 \$	24,092
2000 361	51	361.00	17	1.831	2009	4.0	6.831	27.32	HIGH	46.41%	\$	47,486 \$	38,792
2000 545.20	45		17	1.831	1981	4.0	6.831	27.32	HIGH	46.55%	\$	71,716 \$	58,585
2000 611.60	11		17	1.831	2003	4.0	6.831	27.32	НЭІН	46.71%	\$	\$ 6449 \$	65,720
2000 88.38	8		17	1.831	2003	4.0	6.831	27.32	HIGH	46.73%	\$	11,626 \$	9,498
2000 94.60	4.		17	1.831	2003	4.0	6.831	27.32	HIGH	46.75%	\$	12,443 \$	10,165
2000 274.08	74.		17	1.831	2001	4.0	6.831	27.32	HIGH	46.82%	\$	36,053 \$	29,452
2000 269.48	2.69	81	17	1.831	2001	4.0	6.831	27.32	HIGH	46.89%	\$	35,447 \$	28,957
2000 546.95	16.9	5	17	1.831	2003	4.0	6.831	27.32	HIGH	47.03%	\$	71,945 \$	58,773
1970 677.05	77.0)5	47	3.342	1981	2.0	13.342	26.68	HIGH	47.20%	\$	\$ 650'68	59,298
1990 353.07	53.0	7	27	1.650	1996	4.0	6.650	26.60	HIGH	47.29%	\$	46,443 \$	38,781
1990 20.70	0.70		27	1.650	1995	4.0	6.650	26.60	HIGH	47.30%	\$	\$ 089'8	3,031
1990 323.26	23.2		27	3.664	1995	3.0	8.664	25.99	HIGH	47.38%	\$	42,521 \$	26,942
1935 703.07	03.07		82	12.989	1995	2.0	12.989	25.98	HIGH	47.56%	\$	123,311 \$	123,311
1935 170.04	70.04		82	12.989	2006	2.0	12.989	25.98	HIGH	47.60%	\$	\$ 823,62	29,823
1935 590.30	90.30		82	12.989	1995	2.0	12.989	25.98	HIGH	47.75%	\$	103,533 \$	103,533
1935 353.74	53.74		82	12.989	2012	2.0	12.989	25.98	HIGH	47.85%	₩.	46,532 \$	46,532
1980 3489.26	89.2	9	37	3.696	2005	7.0	3.696	25.87	HIGH	48.74%	\$	1,070,993 \$	675,122
1994 577.26	2.77		23	2.864	1993	2.0	12.864	25.73	HIGH	48.88%	\$	75,933 \$	54,189
1994 426.33	26.3		23	2.864	1993	2.0	12.864	25.73	HIGH	48.99%	\$	\$ 620,95	40,021
1994 459.09	59.	60	23	2.864	1993	2.0	12.864	25.73	HIGH	49.11%	\$	\$ 888,09	43,096
650.13	50.1	13	23	2.864	1995	2.0	12.864	25.73	HIGH	49.28%	\$	85,518 \$	61,030
1994 664.36	54.		23	2.864	1993	2.0	12.864	25.73	HIGH	49.44%	\$	\$ 068,78	62,366
421.12	21.	12	23	2.864	1984	2.0	12.864	25.73	HIGH	49.55%	\$	55,394 \$	39,532
438.59	38.5	6	23	2.864	1994	2.0	12.864	25.73	HIGH	49.66%	\$	\$ 869'25	41,172
1980 382.93	82.	93	37	3.696		0.9	3.696	22.18	HIGH	49.76%	\$	50,371 \$	31,752
1980 303.31	33.	31	37	6.057	1995	2.0	11.057	22.11	HIGH	49.84%	\$	\$ 268,68	15,732
1980 44.11	4		37	6.057	2011	2.0	11.057	22.11	HIGH	49.85%	\$	5,802 \$	2,288
351	51	351.73	37	6.057	1986	2.0	11.057	22.11	HIGH	49.94%	\$	46,267 \$	18,243

AGE (in AG 2017) 37 37 37 37 37 37 37 37 37	1984 1984 1986 1984 1986 2013 2011 2011 2011 1980 1971 1980 1981 1983 1983	CONSEQUENCE CONDITION SCORE 2.0 11.057	22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH HIGH HIGH HIGH HIGH HIGH	FACENT Referent Referent Referent Sci. 07% \$ 50.07% \$ 50.08% \$ 50.22% \$ 50.23% \$ 50.23% \$ 50.33% \$ 50.34% \$ 50.34% \$ 50.54% \$ 50.78% \$ 50.78% \$ 51.13% \$ 51.13% \$ 51.13% \$ 51.27% \$ 5	(as of 2015) 45,616 \$ 45,516 \$ 45,516 \$ 24,995 \$ 3,944 \$ 42,273 \$ 9,940 \$ 9,940 \$ 60,666 \$ 39,463 \$ 124,859 \$ 47,347 \$ 41,657 \$ 16,937 \$	Current Value (Based on 17,987 1,805 9,856 1,555 16,669 3,919 18,173 3,808 23,921 15,561 15,561 16,426 6,678
37 37 37 37 37 37 37 37 37 37 37			22.11 22.11	HIGH HIGH HIGH HIGH HIGH HIGH HIGH HIGH		45,616 4,577 24,995 3,944 42,273 9,940 46,088 9,657 60,666 124,859 124,859 124,859 124,859 124,859 14,7347 41,657	17,987 1,805 9,856 16,669 3,919 18,173 3,808 23,921 15,561 49,233 72,066 8,202 18,670
37 37 37 37 37 37 37 37 37 37 37			22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH HIGH HIGH HIGH HIGH HIGH		4,577 24,995 3,944 42,273 9,940 46,088 9,657 60,666 39,463 124,859 182,765 20,802 47,347 41,657	1,805 9,856 1,555 1,669 3,919 18,173 3,808 23,921 15,561 49,233 72,066 8,202 18,670
37 37 37 37 37 37 37 37 37 37			22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH HIGH HIGH HIGH HIGH HIGH		24,995 3,944 42,273 9,940 46,088 9,657 60,666 39,463 124,859 124,859 182,765 20,802 47,347 41,657	9,856 1,555 16,669 3,919 18,173 3,808 23,921 15,561 49,233 72,066 8,202 18,670
37 37 37 37 37 37 37 37 37			22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH HIGH HIGH HIGH HIGH HIGH		3,944 42,273 9,940 46,088 9,657 60,666 39,463 124,859 124,859 182,765 20,802 47,347 41,657	1,555 16,669 3,919 18,173 3,808 23,921 15,561 49,233 72,066 8,202 18,670 16,426
37 37 37 37 37 37 37 37			22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH HIGH HIGH HIGH HIGH		42,273 9,940 46,088 9,657 60,666 39,463 124,859 182,765 20,802 47,347 41,657	16,669 3,919 18,173 3,808 23,921 15,561 49,233 72,066 8,202 18,670
37 37 37 37 37 37 37			22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH HIGH HIGH HIGH HIGH		9,940 46,088 9,657 60,666 39,463 124,859 182,765 20,802 47,347 41,657	3,919 18,173 3,808 23,921 15,561 49,233 72,066 8,202 18,670 16,426
37 37 37 37 37 37			22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH HIGH HIGH HIGH		46,088 9,657 60,666 39,463 124,859 182,765 20,802 47,347 41,657	18,173 3,808 23,921 15,561 49,233 72,066 8,202 18,670 16,426
37 37 37 37 37			22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH HIGH		9,657 60,666 39,463 124,859 182,765 20,802 47,347 41,657	3,808 23,921 15,561 49,233 72,066 8,202 18,670 16,426
37 37 37 37			22.11 22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH HIGH		60,666 39,463 124,859 182,765 20,802 47,347 41,657	23,921 15,561 49,233 72,066 8,202 18,670 16,426
37			22.11 22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH HIGH		39,463 124,859 182,765 20,802 47,347 41,657	15,561 49,233 72,066 8,202 18,670 16,426
37			22.11 22.11 22.11 22.11 22.11 22.11	HIGH HIGH		124,859 182,765 20,802 47,347 41,657	49,233 72,066 8,202 18,670 16,426 6,678
37			22.11 22.11 22.11 22.11 22.11	HIGH		182,765 20,802 47,347 41,657	72,066 8,202 18,670 16,426 6,678
			22.11 22.11 22.11 22.11	HIGH		20,802 47,347 41,657 16,937	8,202 18,670 16,426
158.14 37 6.057			22.11			47,347	18,670 16,426 6,678
359.95 37 6.057			22.11	HIGH	THE PERSON NAMED BY ADDRESS OF	41,657	16,426
316.69 37 6.057			22.11	HIGH	51.35% \$	16 937	6.678
128.76 37 6.057				HIGH	51.38% \$	10,337	2000
360.00 37 6.057			22.11	HIGH	51.47% \$	47,354 \$	18,672
46.26 37 6.057	7 1989	2.0 11.057	22.11	HIGH	51.48% \$	\$ 580'9	2,399
188.83 37 6.057	7 1983	2.0 11.057	22.11	HIGH	51.53% \$	24,838 \$	9,794
18.32 37 6.057	7 2013	2.0 11.057	22.11	HIGH	51.54% \$	2,410 \$	950
255.34 37 6.057	7 1991	2.0 11.057	22.11	HIGH	\$1.60% \$	33,588 \$	13,244
132.65 37 6.057	7 2013	2.0 11.057	22.11	HIGH	51.64% \$	17,449 \$	6,880
504.25 37 6.057	7 1970	2.0 11.057	22.11	HIGH	51.76% \$	\$ 66,329 \$	26,154
64.76 37 6.057	7 1971	2.0 11.057	22.11	HIGH	51.78% \$	11,358 \$	4,479
27.43 37 6.057	7 1971	2.0 11.057	22.11	HIGH	\$1.79% \$	4,811 \$	1,897
697.05 37 6.057	7 2013	2.0 11.057	22.11	HIGH	\$ 1.97% \$	\$ 01,690 \$	36,154
82.60 37 6.057	7 1983	2.0 11.057	22.11	HIGH	\$ 1.99% \$	10,865 \$	4,284
360.04 37 6.057	7 1993	2.0 11.057	22.11	HIGH	\$ 22.08% \$	47,360 \$	18,675
294.91 37 6.057	7 1971	2.0 11.057	22.11	HIGH	52.15% \$	38,792 \$	15,296
361.74 37 6.057	7 1963	2.0 11.057	22.11	HIGH	52.25% \$	47,584 \$	18,763
39.43 70 5.782	2 1995	2.0 10.782	21.56	HIGH	\$ 22.26% \$	6,916 \$	2,917
581.89 70 5.782	2 2003	2.0 10.782	21.56	HIGH	52.41% \$	102,058 \$	43,048

l Asset Value on	14,215	13,743	10,607	12,658	30,920	94,572	70,558	2,009	4,689	7,277	2,475	42,019	5,744	13,500	11,149	1,531	5,888	7,757	10,291	18,202	7,804	13,306	7,272	17,180	30,397	13,330	12,019	15,189	2,365	17,503	4,461	2,443
Estimated Asset Current Value (Based on	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$
Estimated Asset Replacement Value (as of 2015)	50,341	48,671	37,563	44,826	40,398	115,768	86,371	20,289	47,355	22,995	24,994	68,636	5,744	53,222	43,953	6,035	23,211	30,582	40,570	71,758	30,765	52,454	28,668	27,114	47,974	21,039	18,969	23,972	3,732	27,624	7,040	3,856
Estimal Replacen	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$
SUM TOTAL PERCENT (LENGTH)	52.50%	52.60%	52.67%	52.74%	52.81%	53.04%	53.21%	53.25%	53.34%	53.38%	53.43%	53.56%	53.57%	53.68%	53.76%	53.78%	53.82%	53.88%	23.96%	54.10%	54.16%	54.26%	54.31%	54.37%	54.46%	54.50%	54.54%	54.58%	54.59%	54.65%	54.66%	54.67%
RISK_LEVEL	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	нен	HIGH	нен	HIGH	HIGH	нівн	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH
RISK SCORE	21.53	21.53	21.53	21.53	21.12	20.49	20.49	19.01	19.01	19.01	19.01	17.76	17.52	17.46	17.46	17.46	17.46	17.46	17.46	17.46	17.46	17.46	17.46	17.33	17.33	17.33	17.33	17.33	17.33	17.33	17.33	17.33
SCORE	7.176	7.176	7.176	7.176	2.346	6.831	6.831	19.010	19.010	19.010	19.010	8.878	17.523	17.463	17.463	17.463	17.463	17.463	17.463	17.463	17.463	17.463	17.463	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664
CONSEQUENCE SCORE	3.0	3.0	3.0	3.0	9.0	3.0	3.0	1.0	1.0	1.0	1.0	2.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
HydrantAge	1989	1995	1988	1969	1993	2001	2005	1971	1971	1983	2005	1989	1988	1983	1975	2010	2010	2010	2010	2010	2010	2010	2010	1996	1969	1995	1995	1995	1995	1995	1995	1995
AGE_SCORE	7.176	7.176	7.176	7.176	2.346	1.831	1.831	9.010	9.010	9.010	9.010	3.878	12.523	7.463	7.463	7.463	7.463	7.463	7.463	7.463	7.463	7.463	7.463	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664
AGE (in 2017)	57	57	57	57	27	17	17	47	47	47	47	28	57	42	42	42	42	42	42	42	42	42	42	27	7.7	7.2	7.2	72	27	27	27	27
SHAPE	382.70	370.01	285.56	255.58	307.12	880.10	656.62	154.24	360.00	174.82	190.01	521.78	43.67	404.61	334.14	45.88	176.46	232.49	308.42	545.53	233.88	398.77	217.94	206.13	364.71	159.94	144.21	182.24	28.37	210.00	53.52	29.32
YEAR	1960	1960	1960	1960	1990	2000	2000	1970	1970	1970	1970	1989	1960	1975	1975	1975	1975	1975	1975	1975	1975	1975	1975	1990	1990	1990	1990	1990	1990	1990	1990	1990
PIPE YEAR DIAMETE MATERIAL INSTALLED R	AC	AC	AC	AC	COPPER	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40
PIPE DIAMETE R	4	9	9	8	2	4	9	4	4	4	4	9	9	9	9	9	9	9	9	9	9	9	9	4	4	4	4	4	4	2	4	9
DESCRIPTION	E. EIGHTH AVE.	W. EIGHTH AVE.	MAPLE ST.	MORGAN ST.		BROAD ST.		E. NINTH AVE.	E. NINTH AVE.	E. SEVENTH AVE.	E. NINTH AVE.	J. MESCALERO DR				SILVER ST.		W. FIFTH AVE.	BELLE ST.	HILLCREST ST.												
GIS 1D	486	525	542	893	1008	15	1056	116	129	158 E	167	912	939	1059	1098	320	321	322	323	330	336	344	822	7	72	99	29	89	69	70	184	249

	Ž.	0	LENGTH 2017)	ED LENGTH 2017)	INSTALLED LENGTH 2017)
3.664	00054	7.2		277.11	1990 277.11
3.664	THE REAL PROPERTY.	77		72.03	1990 72.03
3.664		27	165.31 27		165.31
3.664	NO.	7.7	271.11 27		271.11
3.664	17.3	7.2	19.54 27		19.54
3.664	155000	77	326.49 27		326.49
3.664	Sec. 25. 10 F	27	314.48 27		314.48
3.664	SHOW	7.7	210.59 27		210.59
3.664	E SHIP	27	437.16 27		437.16
3.664	0.00	27	212.67 27		212.67
3.664	5 15 4	27	62.48 27		62.48
3.664		27	58.46 27		58.46
3.664		27	1237.66 27		1237.66
3.664		7.7	141.56 27		141.56
3.664	NA.	72	535.86 27		535.86
3.664		72	483.74 27		483.74
3.664		7.7	152.79 27		152.79
3.664		27	227.35 27		227.35
3.664		7.2		686.81	1990 686.81
3.664		27	268.01 27		268.01
3.664		77	595.63 27		595.63
3.664	200	27	335.75 27		335.75
3.664	5-4	7.2	263.07 27		263.07
7.176	1000	57	365.91 57		365.91
7.176		57	360.00 57		360.00
3.455		26	177.03 26		177.03
3.455		26	260.28 26		260.28
3.455	17 P. 28 P.	26	466.63 26		466.63
3.455		26	1703.44 26		1703.44
		76	508.00 26		508.00
3.455	100	27			

		DIDE			was acoling of water of		stem bistingation of stem Assets and Estimated Nephacement Value/Current Value	System Assets a	ild Latiniated In	placellelle va	מכו במוו בייי	Value	TO THE RESIDENCE OF THE PARTY O				
פוציט	DESCRIPTION DI	DIAMETE NANTCOIAL		YEAR	SHAPE	AGE (in			CONSEQUENCE	CONDITION	RISK		SUM TOTAL	Estimated Asser		Estimated Asser	Asset
		R		STALLED	LENGTH	2017)	AGESCORE	HydrantAge	SCORE	SCORE	SCORE	RISK_LEVEL	PERCENT	Replacement Value	ent Value	Current Value	/alue on
623	E. THIRD AVE.	4	Cl	1970	358.79	47	3.342	1963	2.0	8.342	16.68	HIGH	57.72%	\$	47,195 \$		31,424
731	W. SECOND AVE.	∞	ū	1970	162.22	47	3.342	2012	2.0	8.342	16.68	HIGH	57.76%	\$	28,451 \$		18.944
151	E. SEVENTH AVE.	4 PV	PVC-SCH40	1980	140.00	37	6.057	1983	1.0	16.057	16.06	HIGH	57.80%	. 5			7,262
1060		6 PV	PVC-SCH40	1980	204.05	37	6.057	1991	1.0	16.057	16.06	HIGH	57.85%	\$			10,583
356	N. INTERCHANGE	6 PV	PVC-SCH40	1996	509.12	21	2.497	1996-1998	2.0	7.497	14.99	HIGH	57.98%	\$	\$ 026,99		50,248
1095	N. INTERCHANGE	6 PV	PVC-SCH40	1996	655.44	21	2.497	1996-1998	2.0	7.497	14.99	HIGH	58.15%	\$	86,217 \$		64,689
48	SIMPSON ST.	4	AC	1960	380.01	57	7.176	1980	2.0	7.176	14.35	HIGH	58.25%	\$			14,115
223	MYRTLE ST.	9	AC	1960	369.00	57	7.176	1997	2.0	7.176	14.35	HIGH	58.34%	\$			13,706
351	W. FOURTH AVE.	∞	AC	1960	330.59	57	7.176	1998	2.0	7.176	14.35	HIGH	58.42%	\$	\$ \$86,75		16,373
379	W. FOURTH AVE.	8	AC	1960	203.60	57	7.176	1998	2.0	7.176	14.35	HIGH	58.48%	\$			10,083
454	IRON ST.	9	AC	1960	403.64	57	7.176	1969	2.0	7.176	14.35	HIGH	58.58%	\$	\$ 3,095 \$		14,992
471	N. CEDAR ST.	4	AC	1960	686.83	57	7.176	1999	2.0	7.176	14.35	HIGH	58.75%	\$	90,345 \$		25,511
472	N. CEDAR ST.	4	AC	1960	36.00	57	7.176	1999	2.0	7.176	14.35	HIGH	58.76%	\$	4,735 \$		1,337
473	E. FOURTH AVE.	4	AC	1960	313.24	57	7.176	1981	2.0	7.176	14.35	HIGH	58.84%	\$	41,204 \$		11,635
481	GOLD ST.	4	AC	1960	717.29	57	7.176	1995	2.0	7.176	14.35	HIGH	59.03%	\$	94,352 \$		26,642
503	CUCHILLO ST.	4	AC	1960	545.55	57	7.176	1972	2.0	7.176	14.35	HIGH	59.17%	\$	71,761 \$		20,263
504	WADE ST.	4	AC	1960	286.05	57	7.176	1972-1975	2.0	7.176	14.35	HIGH	59.24%	\$	37,627 \$		10,625
524	W. EIGHTH AVE.	9	AC	1960	314.31	57	7.176	1995	2.0	7.176	14.35	HIGH	59.32%	\$	41,344 \$		11,674
526	MARIE ST.	9	AC	1960	329.98	57	7.176	1970	2.0	7.176	14.35	HIGH	59.40%	\$	43,406 \$		12,256
534	MARIE ST.	9	AC	1960	78.12	57	7.176	1970	2.0	7.176	14.35	HIGH	59.42%	\$	10,276 \$		2,902
551	SILVER ST.	9	AC	1960	1025.09	57	7.176	1992	2.0	7.176	14.35	HIGH	29.69%	\$	134,840 \$		38,075
552	SILVER ST.	9	AC	1960	46.03	57	7.176	1992	2.0	7.176	14.35	HIGH	29.70%	\$	\$ 550'9		1,710
559	N. CEDAR ST.	9	AC	1960	3.63	57	7.176	1998	2.0	7.176	14.35	HIGH	29.70%	₩.	478 \$		135
562	DATE ST.	9	AC	1960	170.25	57	7.176	1995	2.0	7.176	14.35	HIGH	59.74%	\$	22,394 \$		6,324
268	CORBETT ST.	9	AC	1960	27.50	57	7.176	1989	2.0	7.176	14.35	HIGH	89.75%	\$	3,617 \$		1,021
571	CABALLO RD.	9	AC	1960	830.03	57	7.176	2013	2.0	7.176	14.35	HIGH	29.96%	\$	\$ 281,601		30,830
573	E. THIRD AVE.	9	AC	1960	171.97	57	7.176	1975	2.0	7.176	14.35	HIGH	%00.09	\$	\$ 22,622 \$		6,388
576	E. THIRD AVE.	9	AC	1960	43.96	57	7.176	1975	2.0	7.176	14.35	HIGH	60.02%	\$	5,782 \$		1,633
577	E. FIFTH AVE.	9	AC	1960	63.39	57	7.176	1993	2.0	7.176	14.35	HIGH	60.03%	\$	\$,338 \$		2,355
578	E. FOURTH AVE.	9	AC	1960	360.02	57	7.176	1975	2.0	7.176	14.35	HIGH	60.12%	\$	47,357 \$		13,372
579	ARROWHEAD RD.	9	AC	1960	361.00	57	7.176	1976	2.0	7.176	14.35	HIGH	60.22%	\$	47,486 \$		13,409
586	OSBORN LN.	9	AC	1960	356.86	57	7.176	1975	2.0	7.176	14.35	HIGH	60.31%	\$	46,941 \$		13,255

Estimated Asset	Current Value (Based on	13,372	26,373	9,298	12,876	12,617	3,938	4,825	29,961	6,853	6,159	7,074	6,354	33,549	29,782	24,683	16,646	2,561	16,674	1,016	2,341	1,209	346	409	804	5,730	2,086	1,650	538	1,775	4,549	522	3,116
	Replacement Value Curr (as of 2015) (B:	47,355 \$	\$ 868'86	32,927 \$	45,601 \$	44,683 \$	13,946 \$	17,086 \$	106,105 \$	24,270 \$	21,813 \$	25,053 \$	22,504 \$	118,813 \$	105,470 \$	32,250 \$	168,129 \$	25,863 \$	168,406 \$	10,262 \$	23,645 \$	12,210 \$	3,492 \$	4,136 \$	8,119 \$	57,871 \$	21,065 \$	16,661 \$	5,433 \$	17,928 \$	45,946 \$	5,268 \$	31,470 \$
Estimat	Replacen (as o	\$	\$	\$	\$	\$	\$	\$	\$	\$	S	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$
SUM TOTAL.	PERCENT (LENGTH)	60.40%	60.58%	60.64%	60.71%	%08.09	60.82%	%98.09	61.06%	61.11%	61.14%	61.18%	61.21%	61.38%	61.59%	61.65%	61.98%	62.03%	62.35%	62.37%	62.42%	62.44%	62.45%	62.46%	62.47%	62.59%	62.63%	62.66%	62.67%	62.71%	62.79%	62.80%	62.87%
	RISK_LEVEL	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	нен	HIGH	HIGH	HIGH	нюн	нівн	HIGH	HIGH	нівн	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH
RISK	SCORE	14.35	14.35	14.35	14.35	14.35	14.35	14.35	14.35	14.35	14.35	14.35	14.35	14.35	14.35	14.08	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14 01
NOILION	SCORE	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	2.346	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010
CONSECUENCE	SCORE	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	6.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
	HydrantAge	1995	1989	2013	1969	1995	1993	1993	2003	1996-1998	1999	1999	1999	1992-1999	1975	1981	1977	1977	1977	1976	1976	1976	1976	1976	1978	1978	1978	1978	1972	1972	1975	1975	1972
	AGE_SCORE	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	2.346	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010
AGE fin	2017)	57	57	57	57	57	57	57	57	57	57	57	57	57	57	27	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47
MISK SCOUMS	LENGTH	360.00	710.03	250.32	260.00	339.69	106.02	129.89	806.64	184.51	124.37	142.84	128.31	677.42	801.81	245.17	1278.16	196.62	1280.26	78.01	179.75	92.82	26.55	31.44	61.72	439.95	160.14	126.66	41.30	136.30	349.29	40.05	72021
VEAD	INSTALLED	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1990	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1070
	MATERIAL	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	COPPER	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	טוני כרוואט
PIPE	DIAMETE	9	4	9	∞	4	9	9	9	9	∞	∞	∞	∞	9	0.75	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	4	,
	DESCRIPTION	W. EIGHTH AVE.	CORBETT ST.	CABALLO RD.	MORGAN ST.	GOLD ST.	E. FIFTH AVE.	E. FIFTH AVE.	W. BARTON AVE.	HYDE ST.							HACKBERRY LN.	OTTONWOOD RE	MERCURY ST.	ARROWHEAD RD.	SILVER ST.	SILVER ST.	E. FIRST AVE.	E. FIRST AVE.	CABALLO RD.	ARROYO ST.	OSBORN LN.	OSBORN LN.	00018080				
	GIS ID	687	701	711	725	860	870	871			972	973	974	975	1001	1087	09									104	113	115	135	136	148	149	7.70

Asset alue	4,689	2,436	312	4,950	1,718	5,975	4,632	2,841	728	4,746	7,111	3,576	2,831	6,225	4,038	9,471	11,264	11,305	902	3,758	3,664	4,914	2,475	4,285	3,741	1,411	3,311	266	2,529	2,532	4,879	6,136
Estimated Assel Current Value (Based on											100			16		\$	10	\$		\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$
	47,357 \$	24,608 \$	3,153 \$	\$ 866'64	17,356 \$	60,344 \$	\$ 622.9	\$ 169,82	7,354 \$	47,930 \$	71,821 \$	36,114 \$	28,588 \$	62,876 \$	40,781 \$	\$ 259'56	\$ 692,811	114,184 \$	9,109 \$	37,952 \$	37,007 \$	49,628	24,993	43,281	37,782	14,252	33,439	2,689 \$	25,539	25,578	49,274	61,971
Estimated Asset Replacement Value (as of 2015)	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$
SUM TOTAL PERCENT (LENGTH)	62.96%	63.01%	63.01%	63.11%	63.14%	63.26%	63.35%	63.41%	63.42%	63.51%	63.65%	63.72%	63.78%	63.90%	63.98%	64.17%	64.39%	64.61%	64.63%	64.70%	64.77%	64.87%	64.92%	%00.59	%20.59	65.10%	65.17%	65.17%	65.22%	65.27%	65.37%	65.49%
RISK_LEVEL	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	HIGH	нівн	нівн	нівн	HIGH	нын	HIGH	нен	нівн	HIGH	HIGH	HIGH	HIGH
RISK RI SCORE	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01	14.01
CONDITION	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010	14.010
CONSEQUENCE C SCORE	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
HydrantAge	1971	1975	1983	1983	1972	1972-1975	1972-1975	1978	1978	1973	1971	2013	2013	1978	1978	1978	1978	1978	2013	2013	2013	1971	1977	1979	1979	2013	1997	1979	1972-1975	1972-1975	2013	1977
AGE_SCORE	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010	9.010
AGE (in 2017)	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47	47
SHAPE	360.02	187.08	23.97	380.10	131.94	458.75	355.62	218.12	55.90	364.38	546.00	274.55	217.34	478.00	310.03	727.17	864.90	868.06	69.25	288.52	281.33	377.28	190.00	329.03	287.23	108.35	254.21	20.44	194.15	194.45	374.60	471.12
YEAR INSTALLED	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970
PIPE DIAMETE MATERIAL IN R	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40
PIPE JAMETE R	4	4	4	4	4	4	4	4	4	9	9	9	9	9	9	9	9	9	9	9	9	9	9	4	9	9	9	4	2	4	2	2
DESCRIPTION D	E. SIXTH AVE.	E. SIXTH AVE.	E. SEVENTH AVE.	LUCKY ST.	ARROYO ST.	GARNETT ST.	GIBSON ST.	E. FIRST AVE.	E. SECOND AVE.	N. CEDAR ST.	TIN ST.	CHERRY LN.	CHERRY LN.	YUCCA ST.	W. EIGHTH AVE.	SIERRA VISTA DR.	PALO VERDE ST.	YUCCA ST.	CHERRY LN.	CHERRY LN.	MARIE ST.	TIN ST.	VEATER ST.	AUSTIN AVE.	AUSTIN AVE.	CHERRY LN.	W. THIRD AVE.	E. SECOND AVE.				
GIS 1D	154	155	156	157	162	163	164	175	176	204	259	264	281	291	292	293 S	296	297	313	314	337	439	447	466	467	708	754	878	786	886	266	1031

GIS ID DESCRIPTION PAMERIAL WATERIAL 1050 2 PVC-SCH40 1970 1071 0.75 PVC-SCH40 1970 1127 E. SEVENTH AVE. 4 PVC-SCH40 1990 11075 0.75 PVC-SCH40 1990 11076 0.75 PVC-SCH40 1990 52 HENSON ST. 8 PVC-SCH40 2000 75 NORTH ST. 4 PVC-SCH40 2000 84 NICKEL ST. 4 PVC-SCH40 2000 85 LEAD ST. 4 PVC-SCH40 2000 87 CITY ST. 4 PVC-SCH40 2000 87 LEAD ST. 4 PVC-SCH40 2000 87 COPPER ST. 4 PVC-SCH40 2000 894 NORTH ST. 4 PVC-SCH40 2000 895 BRIGHTON ST. 6 PVC-SCH40 2000 896 BRIGHTON ST. 4 PVC-SCH40 20	YEAR NSTALLED 1970	SHAPE	AGE (in	AGE COOPE	Undergat And	CONSEQUENCE	CONDITION	RISK	The second	SUM IOIAL	ESTIMA		Estimated Asset
2 6.75 E. SEVENTH AVE. 4 0.75 HENSON ST. 8 CITY ST. 6 NORTH ST. 4 LEAD ST. 4 LEAD ST. 4 COPPER ST. 4 COPPER ST. 4 SIMPSON ST. 4 MARSHALL ST. 6 MARSHALL ST. 6 MARSHALL ST. 6 REGHTON ST. 6 8 WYONA AVE. 4 CHARLES AVE. 4 CHARLES AVE. 4 WYONA AVE. 4 CHARLES AVE. 6 CHARLES AVE. 4 CHARLES AVE. 4 CHARLES AVE. 6 CHARLES AVE. 4 CHARLES AVE. 6 CHARLES AVE. 7 CHAR	1970		2017)	שמר שכחור	nydiality ge	SCORE	SCORE	SCORE	RISK_LEVEL	PERCENT	Replacen	Replacement Value	Current Value
6.75 E. SEVENTH AVE. 4 0.75 HENSON ST. 8 CITY ST. 6 NORTH ST. 4 ILEAD ST. 4 ILEAD ST. 4 SIMPSON ST. 4 SIMPSON ST. 4 SIMPSON ST. 6 NORTH ST. 6 NORTH ST. 6 WYONA AVE. 4 CHARLES AVE. 6 RIVERSIDE DR. 4		187.90	47	9.010	1971	1.0	14.010	14.01	HIGH	65.53%	\$	24,717 \$	2,447
E. SEVENTH AVE. 4 0.75 HENSON ST. 8 CITY ST. 6 NORTH ST. 4 ILEAD ST. 4 ILEAD ST. 4 ILEAD ST. 4 SIMPSON ST. 4 SIMPSON ST. 4 SIMPSON ST. 6 NORTH ST. 6 NORTH ST. 6 WYONA AVE. 6 GRAY ST. 4 GRAY ST. 6 CLANCY ST. 6 CLANCY ST. 6	1970	353.77	47	9.010	1997	1.0	14.010	14.01	HIGH	65.62%	\$	46,534 \$	4,607
0.75 HENSON ST. 8 CITY ST. 6 NORTH ST. 4 LEAD ST. 4 LEAD ST. 4 LEAD ST. 4 COPPER ST. 4 SIMPSON ST. 4 SIMPSON ST. 4 SIMPSON ST. 6 MARSHALL ST. 6 NORTH ST. 6 REGHTON ST. 6 CHARLES AVE. 4 CHARLES AVE. 4 CHARLES AVE. 4 CHARLES AVE. 4 CHARLES DR. 4 CHARLES DR. 4	1990	223.70	27	3.664	1995	1.0	13.664	13.66	HIGH	65.68%	\$	29,425 \$	18,644
0.75 HENSON ST. 8 CITY ST. 6 NORTH ST. 4 LEAD ST. 4 LEAD ST. 4 COPPER ST. 4 COPPER ST. 4 SIMPSON ST. 4 MARSHALL ST. 6 MARSHALL ST. 6 NORTH ST. 6 REGHTON ST. 6 CHARLES AVE. 4 CHARLES DR. 4	1990	174.70	27	3.664	1999	1.0	13.664	13.66	HIGH	65.73%	\$	\$ 086,22	14,560
HENSON ST. 8 CITY ST. 6 NORTH ST. 4 LEAD ST. 4 LEAD ST. 4 COPPER ST. 4 SIMPSON ST. 4 SIMPSON ST. 4 MARSHALL ST. 6 NORTH ST. 6 WYONA AVE. 4 CHARLES AVE. 6 CHARLES AVE. 6 CHARLES AVE. 6 CHARLES AVE. 6 CHARLES AVE. 7 CHARLES	1990	186.19	77	3.664	1999	1.0	13.664	13.66	HIGH	65.77%	\$	24,492 \$	15,518
CITY ST. 6 NORTH ST. 4 NICKEL ST. 4 LEAD ST. 4 COPPER ST. 4 SIMPSON ST. 4 SIMPSON ST. 4 SIMPSON ST. 6 NORTH ST. 6 NORTH ST. 6 REIGHTON ST. 6 CHARLES AVE. 4 CHARLES AVE. 6 CHARLES AVE. 6 CHARLES AVE. 6 CHARLES AVE. 6 CHARLES AVE. 7 CHARLES AVE. 7 CHARLES AVE. 7 CHARLES AVE. 6 CHARLES AVE. 7 CHARLE	2000	367.00	17	1.831	2003	2.0	6.831	13.66	HIGH	65.87%	\$	64,368 \$	52,583
NORTH ST. 4 NICKEL ST. 4 LEAD ST. 4 COPPER ST. 4 SIMPSON ST. 4 SIMPSON ST. 4 SIMPSON ST. 6 MARSHALL ST. 6 NORTH ST. 6 NORTH ST. 6 CHARLES AVE. 4 CHARLES AVE. 6 CCLANCY ST. 6 CLANCY ST. 6	2000	360.11	17	1.831	2005-New	2.0	6.831	13.66	HIGH	%96.59	\$	47,369 \$	38,696
NICKEL ST. LEAD ST. COPPER ST. SIMPSON ST. SIMPSON ST. MARSHALL ST. MARSHALL ST. NORTH ST. BRIGHTON ST. BRIGHTON ST. CHARLES AVE. WYONA AVE. GRAY ST. GRAY ST. CLANCY ST. 6 6 8 8 WYONA AVE. 4 CHARLES AVE. 4 CHARLES AVE. 6 CHARLES ST. 7 CHARL	2000	252.38	17	1.831	2001	2.0	6.831	13.66	HIGH	66.02%	s	33,198 \$	27,120
LEAD ST.	2000	731.03	17	1.831	1986	2.0	6.831	13.66	HBIH	66.21%	\$	96,160 \$	78,554
COPPER ST. 4 SIMPSON ST. 4 SIMPSON ST. 4 MARSHALL ST. 6 NORTH ST. 6 BRIGHTON ST. 6 8 WYONA AVE. 4 CHARLES AVE. 4 GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	2000	348.34	17	1.831	1986	2.0	6.831	13.66	HIGH	66.30%	\$	45,820 \$	37,431
SIMPSON ST. 4 SIMPSON ST. 4 MARSHALL ST. 6 NORTH ST. 6 BRIGHTON ST. 6 8 WYONA AVE. 4 CHARLES AVE. 4 CHARLES AVE. 4 GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	2000	734.19	17	1.831	1986	2.0	6.831	13.66	HIGH	66.49%	\$	\$ 925'96	78,894
SIMPSON ST. 4 MARSHALL ST. 6 NORTH ST. 4 BRIGHTON ST. 6 8 WYONA AVE. 4 CHARLES AVE. 4 WYONA AVE. 4 CHARLES AVE. 4 CHARLES AVE. 4 CHARLES AVE. 4 CHARLES AVE. 6 CLANCY ST. 6	2000	365.90	17	1.831	1986	2.0	6.831	13.66	HIGH	%85'99	\$	48,131 \$	39,318
MARSHALL ST. 6 NORTH ST. 4 BRIGHTON ST. 6 8 WYONA AVE. 4 CHARLES AVE. 4 WYONA AVE. 4 GRAY ST. 6 CLANCY ST. 6	2000	364.28	17	1.831	1986	2.0	6.831	13.66	HIGH	%29.99	\$	47,917 \$	39,144
NORTH ST. 4 BRIGHTON ST. 6 8 WYONA AVE. 4 CHARLES AVE. 4 WYONA AVE. 4 GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	2000	285.06	. 17	1.831	1995	2.0	6.831	13.66	HIGH	66.75%	\$	37,496 \$	30,631
BRIGHTON ST. 6 8 WYONA AVE. 4 CHARLES AVE. 4 WYONA AVE. 4 GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	2000	361.89	17	1.831	1969	2.0	6.831	13.66	HIGH	66.84%	\$	47,603 \$	38,888
6 WYONA AVE. 4 CHARLES AVE. 4 WYONA AVE. 4 GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	2000	359.39	17	1.831	2001	2.0	6.831	13.66	HIGH	66.93%	\$	47,274 \$	38,619
8 WYONA AVE. 4 CHARLES AVE. 4 WYONA AVE. 4 GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	2000	63.77	17	1.831	2006	2.0	6.831	13.66	HIGH	%56.99	\$	\$ 688'8	6,853
WYONA AVE. 4 CHARLES AVE. 4 WYONA AVE. 4 GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	1990	39.40	27	1.650	1995	2.0	0.650	13.30	HIGH	%96.99	\$	6,911 \$	5,771
CHARLES AVE. 4 WYONA AVE. 4 GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	1935	450.01	82	12.989	1983	1.0	12.989	12.99	HIGH	%20.79	\$	59,194 \$	59,194
WYONA AVE. 4 GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	1935	899.72	82	12.989	1978	1.0	12.989	12.99	HIGH	67.30%	\$	118,350 \$	118,350
GRAY ST. 4 RIVERSIDE DR. 4 CLANCY ST. 6	1935	450.02	82	12.989	1983	1.0	12.989	12.99	HIGH	67.41%	\$	\$ 59,195 \$	59,195
RIVERSIDE DR. 4 CLANCY ST. 6	1935	489.18	82	12.989	1983	1.0	12.989	12.99	HIGH	67.54%	\$	64,347 \$	64,347
CLANCY ST. 6	1935	819.39	82	12.989	1983	1.0	12.989	12.99	HIGH	67.75%	\$	107,782 \$	107,782
	1935	325.37	82	12.989	1983-1999	1.0	12.989	12.99	HIGH	67.83%	\$	42,800 \$	42,800
497 FOCH ST. 4 AC	1935	318.99	82	12.989	1970	1.0	12.989	12.99	HIGH	67.91%	\$	41,960 \$	41,960
499 MARR AVE. 4 AC	1935	32.39	82	12.989	1998	1.0	12.989	12.99	HIGH	67.92%	\$	4,261 \$	4,261
505 AUSTIN AVE. 4 AC	1935	450.00	82	12.989	1993	1.0	12.989	12.99	HIGH	68.04%	\$	59,193 \$	59,193
507 BIRCH ST. 4 AC	1935	359.00	82	12.989	1981	1.0	12.989	12.99	HIGH	68.13%	\$	47,223 \$	47,223
532 BROADWAY ST. 6 AC	1935	17.34	82	12.989	1997	1.0	12.989	12.99	HIGH	68.13%	\$	2,281 \$	2,281
535 MIMS ST. 6 AC	1935	172.77	82	12.989	1963	1.0	12.989	12.99	HIGH	68.18%	\$	\$ 727,22	727,22
536 MIMS ST. 6 AC	1935	134.84	82	12.989	1963	1.0	12.989	12.99	HIGH	68.21%	\$	17,736 \$	17,736
537 MCADOO ST. 6 AC	1935	410.27	82	12.989	1991	1.0	12.989	12.99	HIGH	68.32%	\$	\$ 296'85	53,967

APPENDIX A Recoring of Water System Distribution System Assets and Estimated Replacement Value/Current Value

					RISK SCOTING OF Water System Distribution System Assets and Estimated Replacement Value, Current Value	Water Syst	cili bisti batioli	aystelli Assets a	וומ בזנוווומנכמ וויי	na allamandi	ine fee		A SERVICE OF THE PROPERTY OF T	STATE OF STA	0.00	政化のいかのかのとのはあることはあると
GISID	DESCRIPTION	PIPE	MATERIAL	YEAR	SHAPE	AGE (in	AGE SCORE	HvdrantAge	CONSEQUENCE	CONDITION	RISK	RISK LEVEL	SUM TOTAL PERCENT	Estima Replace	Estimated Asset F Replacement Value	Estimated Asset Current Value
		R		INSTALLED	LENGTH	(7102			SCORE	SCORE	SCORE		(LENGTH)	(as c	of 2015)	(Based on
926	JACKSON ST.	9	PVC-SCH40	1994	814.36	23	2.864	1993	1.0	12.864	12.86	HIGH	71.31%	\$	107,121 \$	76,447
927	HAGEN ST.	9	PVC-SCH40	1994	536.36	23	2.864	1993	1.0	12.864	12.86	HIGH	71.45%	\$	70,552 \$	50,350
928	GLENN ST.	9	PVC-SCH40	1994	562.32	23	2.864	1993	1.0	12.864	12.86	HIGH	71.59%	\$	\$ 896'82	52,787
930	GLENN ST.	9	PVC-SCH40	1994	267.45	23	2.864	1995	1.0	12.864	12.86	HIGH	71.66%	\$	35,181 \$	25,107
1101		9	PVC-SCH40	1994	277.85	23	2.864	1994	1.0	12.864	12.86	HIGH	71.73%	\$	36,548 \$	26,083
964		10	PVC-C900	2000	1113.81	17	1.831	2006-2007	7.0	1.831	12.82	HIGH	72.01%	\$	244,191 \$	199,482
965		12	PVC-C900	2000	616.79	17	1.831	2011	7.0	1.831	12.82	HIGH	72.17%	\$	162,272 \$	132,562
996		12	PVC-C900	2000	42.77	17	1.831	2011	7.0	1.831	12.82	HIGH	72.18%	\$	11,252 \$	9,192
904	SMITH AVE.	9	PVC-SCH40	1975	319.08	42	7.463	1997	1.0	12.463	12.46	HIGH	72.26%	\$	41,972 \$	10,647
329	SILVER ST.	9	PVC-SCH40	1975	335.41	42	7.463	1975	1.0	12.463	12.46	HIGH	72.35%	\$	44,120 \$	11,191
334	SILVER ST.	9	PVC-SCH40	1975	186.02	42	7.463	1975	1.0	12.463	12.46	HIGH	72.40%	\$	24,468 \$	6,207
335	SILVER ST.	9	PVC-SCH40	1975	306.36	42	7.463	1975	1.0	12.463	12.46	HIGH	72.47%	\$	40,298 \$	10,222
160	LUCKY ST.	4	PVC-SCH40	1975	523.82	42	7.463	1983	1.0	12.463	12.46	HIGH	72.61%	\$	\$ 406'89	17,478
317	E. AURORA AVE.	9	PVC-SCH40	1975	33.25	42	7.463	2010	1.0	12.463	12.46	HIGH	72.62%	\$	4,374 \$	1,109
318	E. AURORA AVE.	9	PVC-SCH40	1975	36.47	42	7.463	2010	1.0	12.463	12.46	HIGH	72.63%	\$	4,798 \$	1,217
319	E. AURORA AVE.	9	PVC-SCH40	1975	120.01	42	7.463	2010	1.0	12.463	12.46	HIGH	72.66%	\$	15,786 \$	4,004
1074		1	POLY	1990	226.87	27	2.346	1999	1.0	12.346	12.35	HIGH	72.71%	\$	29,843 \$	22,841
963		10	PVC-C900	2006	611.73	11	1.000	1971	12.0	1.000	12.00	MEDIUM	72.87%	\$	134,117 \$	120,706
970		10	PVC-C900	2006	2089.03	11	1.000	1971	12.0	1.000	12.00	MEDIUM	73.40%	\$	\$ 866,734	412,203
128	PINE ST.	4	PVC-SCH40	2000	551.09	17	1.831	2006	1.0	11.831	11.83	MEDIUM	73.54%	\$	72,491 \$	59,218
1066	はあれている。	2	PVC-SCH40	2000	202.33	17	1.831	2005	1.0	11.831	11.83	MEDIUM	73.60%	\$	26,614 \$	21,741
1097		9	PVC-SCH40	2009	130.88	∞	0.660	2006	2.0	2.660	11.32	MEDIUM	73.63%	\$	17,216 \$	16,080
12	COLEMAN ST.	4	PVC-SCH40	2010	366.88	7	0.558	2013	2.0	5.558	11.12	MEDIUM	73.72%	\$	48,259 \$	45,567
993		0.75	PVC-SCH40	2010	229.66	7	0.558	2013	2.0	5.558	11.12	MEDIUM	73.78%	\$	30,210 \$	28,524
994		0.75	PVC-SCH40	2010	294.19	7	0.558	2013	2.0	5.558	11.12	MEDIUM	73.86%	\$	\$ 869'88	36,539
995		0.75	PVC-SCH40	2010	188.52	7	0.558	1975	2.0	5.558	11.12	MEDIUM	73.91%	\$	24,797 \$	23,414
6	LOCUST ST.	4	PVC-SCH40	1980	290.85	37	6.057	1984	1.0	11.057	11.06	MEDIUM	73.98%	\$	38,259 \$	15,086
10	MAGNOLIA ST.	4	PVC-SCH40	1980	63.17	37.	6.057	1995	1.0	11.057	11.06	MEDIUM	74.00%	\$	\$ 608'8	3,276
14	E. FIRST AVE.	4	PVC-SCH40	1980	322.92	37	6.057	1995	1.0	11.057	11.06	MEDIUM	74.08%	\$	42,477 \$	16,749
16	E. FIRST AVE.	4	PVC-SCH40	1980	27.53	37	6.057	1995	1.0	11.057	11.06	MEDIUM	74.09%	\$	3,621 \$	1,428
18	ELM ST.	4	PVC-SCH40	1980	186.51	37	6.057	1978-1984	1.0	11.057	11.06	MEDIUM	74.13%	\$	24,533 \$	9,674
19	ELM ST.	4	PVC-SCH40	1980	274.69	37	6.057	1978-1984	1.0	11.057	11.06	MEDIUM	74.20%	\$	36,133 \$	14,248

APPENDIX A Risk Scoring of Water System Distribution System Assets and Estimated Replacement Value/Current Value

Property of the Section of the	Control of the second distribution of the second	SECRETARY OF SECULOR	Control of the lateral description of	STATE OF STREET STATE OF STREET	nish acuiling of	or water sys	יבווו חוצרו ומתרומו	ii Distribution System Assets and Estimated		replacement value/current	ine/carrent	, value			AND PROBLEMS	
GISTD	DESCRIPTION	PIPE DIAMETE R	YEAR E MATERIAL INSTALLED	YEAR INSTALLED	SHAPE	AGE (in 2017)	AGE_SCORE	HydrantAge	CONSEQUENCE SCORE	CONDITION	RISK SCORE	RISK_LEVEL	SUM TOTAL PERCENT (LENGTH)	Estimated Asset Replacement Value (as of 2015)		Estimated Asset Current Value (Based on
20	ELM ST.	4	PVC-SCH40	1980	122.73	37	6.057	1978-1984	1.0	11.057	11.06	MEDIUM	74.23%	\$ 1	16,144 \$	998'9
21	ELM ST.	4	PVC-SCH40	1980	59.96	37	6.057	1978-1984	1.0	11.057	11.06	MEDIUM	74.25%	\$	\$ 1887	3,110
72	HILLCREST ST.	4	PVC-SCH40	1980	152.49	37	6.057	1981	1.0	11.057	11.06	MEDIUM	74.29%	\$ 2	\$ 650'02	606'2
77	CORONA AVE.	4	PVC-SCH40	1980	52.26	37	6.057	2011	1.0	11.057	11.06	MEDIUM	74.30%	\$	6,874 \$	2,710
81	LEAD ST.	4	PVC-SCH40	1980	180.48	37	6.057	1986	1.0	11,057	11.06	MEDIUM	74.35%	\$ 2	23,740 \$	9,361
88	PLATINUM ST.	4	PVC-SCH40	1980	379.38	37	6.057	1981	1.0	11.057	11.06	MEDIUM	74.44%	\$ 4	\$ 406,64	19,678
89	VEATER ST.	4	PVC-SCH40	1980	177.48	37	6.057	1981-1984	1.0	11.057	11.06	MEDIUM	74.49%	\$ 2	23,346 \$	9,206
06	COAL ST.	4	PVC-SCH40	1980	556.21	37	6.057	1971	1.0	11.057	11.06	MEDIUM	74.63%	5 7	73,163 \$	28,849
91	MARSHALL ST.	4	PVC-SCH40	1980	TT2.77	37	6.057	1971	1.0	11.057	11.06	MEDIUM	74.68%	\$ 2	22,726 \$	8,961
92	COAL ST.	4	PVC-SCH40	1980	301.69	37	6.057	1971	1.0	11.057	11.06	MEDIUM	74.75%	\$ 3	39,684 \$	15,648
93	MARSHALL ST.	4	PVC-SCH40	1980	179.27	37	6.057	1971	1.0	11.057	11.06	MEDIUM	74.80%	\$ 2	23,581 \$	9,298
96	E. SECOND AVE.	4	PVC-SCH40	1980	279.17	37	6.057	1988	1.0	11.057	11.06	MEDIUM	74.87%	\$	36,722 \$	14,480
105	TURTLEVIEW AVE	4	PVC-SCH40	1980	13.71	37	6.057	1983	1.0	11.057	11.06	MEDIUM	74.87%	\$	1,803 \$	711
106	TURTLEVIEW AVE	4	PVC-SCH40	1980	163.98	37	6.057	1978	1.0	11.057	11.06	MEDIUM.	74.92%	\$ 2	21,571 \$	8,505
107	TURTLEVIEW AVE	4	PVC-SCH40	1980	7.65	37	6.057	1978	1.0	11.057	11.06	MEDIUM	74.92%	\$	1,007 \$	397
108	GOLD ST.	4	PVC-SCH40	1980	207.70	37	6.057	1978	1.0	11.057	11.06	MEDIUM	74.97%	\$ 2	27,321 \$	10,773
109	JOFFRE ST.	4	PVC-SCH40	1980	360.45	37	6.057	1995	1.0	11.057	11.06	MEDIUM	75.06%	\$	47,414 \$	18,696
110	MAGNOLIA ST.	4	PVC-SCH40	1980	239.29	37	6.057	1995	1.0	11.057	11.06	MEDIUM	75.12%	\$	31,477 \$	12,412
112	E. SECOND AVE.	4	PVC-SCH40	1980	215.29	37	6.057	1995	1.0	11.057	11.06	MEDIUM	75.18%	\$	28,319 \$	11,166
152	OAK ST.	9	PVC-SCH40	1980	360.00	37	6.057	1976	1.0	11.057	11.06	MEDIUM	75.27%	\$ \$	47,354 \$	18,672
153	OAK ST.	4	PVC-SCH40	1980	360.00	37	6.057	1986	1.0	11.057	11.06	MEDIUM	75.36%	\$	47,354 \$	18,672
159	LUCKY ST.	4	PVC-SCH40	1980	348.55	37	6.057	1983	1.0	11.057	11.06	MEDIUM	75,45%	\$	45,848 \$	18,078
169	PERSHING ST.	9	PVC-SCH40	1980	353.30	37	6.057	1989	1.0	11.057	11.06	MEDIUM	75.54%	\$	46,473 \$	18,325
170	LOCUST ST.	4	PVC-SCH40	1980	278.10	37	6.057	1984-1990	1.0	11.057	11.06	MEDIUM	75.61%	\$	36,581 \$	14,424
173	E. SEVENTH AVE.	4	PVC-SCH40	1980	12.06	37	6.057	1983	1.0	11.057	11.06	MEDIUM	75.62%	\$	1,587 \$	979
180	MAPLE ST.	4	PVC-SCH40	1980	290.11	37	6.057	1981	1.0	11.057	11.06	MEDIUM	75.69%	\$	38,161 \$	15,047
181	E. FIRST AVE.	4	PVC-SCH40	1980	360.01	37	6.057	1995	1.0	11.057	11.06	MEDIUM	75.78%	\$	47,356 \$	18,673
182	TURTLEVIEW AVE	4	PVC-SCH40	1980	135.69	37	6.057	1978	1.0	11.057	11.06	MEDIUM	75.82%	\$	17,849 \$	7,038
189	VEATER ST.	4	PVC-SCH40	1980	683.02	37	6.057	1984	1.0	11.057	11.06	MEDIUM	75,99%	\$	\$ 9,844 \$	35,426
198	POPLAR ST.	9	PVC-SCH40	1980	433.42	37	6.057	1980	1.0	11.057	11.06	MEDIUM	76.10%	\$	57,012 \$	22,480
239	SIMPSON ST.	9	PVC-SCH40	1980	360.20	37	6.057	1980	1.0	11.057	11.06	MEDIUM	76.19%	\$	47,381 \$	18,683
241	SIMPSON ST.	9	PVC-SCH40	1980	360.50	37	6.057	1980	1.0	11.057	11.06	MEDIUM	76.28%	\$	47,420 \$	18,698

set	49,092	006'9	18,655	16,311	4,214	865	19,344	854	11,671	6,743	15,719	885	15,499	18,091	4,034	26,570	18,393	7,097	35,021	13,385	26,995	20,731	201,151	40,296	25,268	2,243	25,897	26,926	33,046	996	3,321	10,951
Estimated Asset Current Value (Based on	45	9	18	16	7		19		1		15		1	18	7	2(18		3.	1.	2(2(20	4(2:		2.	2(3.			1(
	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	<>→	\$	\$	\$	\$	\$	\$
Estimated Asset Replacement Value (as of 2015)	124,502	17,500	47,310	41,365	10,687	2,194	49,058	2,167	29,600	17,101	39,865	2,245	39,307	45,881	10,230	67,384	46,647	17,997	88,817	33,946	68,462	52,576	246,234	95,534	906'65	5,317	61,396	63,837	78,344	2,291	7,873	25,962
. Est	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$
SUM TOTAL PERCENT (LENGTH)	76.53%	76.56%	76.65%	76.73%	76.75%	%91.91	76.85%	76.86%	76.91%	76.95%	77.02%	77.03%	77.11%	77.19%	77.21%	77.35%	77.44%	77.47%	77.64%	77.71%	77.84%	77.94%	78.30%	78.49%	78.60%	78.61%	78.70%	78.80%	78.91%	78.91%	78.93%	78.96%
RISK_LEVEL	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM
RISK	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	11.06	10.99	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.78
CONDITION SCORE	11.057	11.057	11.057	11.057	11.057	11.057	11.057	11.057	11.057	11.057	11.057	11.057	11.057	11.057	11.057	11,057	11.057	11.057	11.057	11.057	11.057	11.057	1.831	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.782
CONSEQUENCE	1.0	1,0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	6.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
HydrantAge	1980-1997	1980-1997	1980	1981	1988	1988	1984	1984	1981	1980	1981	1981	1981	1984		. 1980	1986	1983	1981	1971	1980	1983	2003	1989	1995	2003	1963	2013	2003	2003	1993	1995
AGE_SCORE	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	6.057	1.831	5.782	5.782	5.782	5.782	5.782	5.782	5.782	5.782	5.782
AGE (in 2017)	37	37	37	37	37	37	37	37	37	37	37	37	37	37	37	37	37	37	37	37	37	37	17	70	70	70	70	70	70	70	70	70
SHAPE	946.49	133.04	359.66	314.47	81.25	16.68	372.95	16.47	225.02	130.00	303.06	17.06	298.85	348.80	TT.TT	512.27	354.62	136.82	675.21	258.06	520.47	399.70	1403.92	726.27	455.42	40.42	350.05	363.97	446.69	13.06	44.89	148.02
YEAR	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	1980	2000	1947	1947	1947	1947	1947	1947	1947	1947	1947
PIPE DIAMETE MATERIAL R	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-C900	10	D	D	O	D	CI	CI	Cl	CI
PIPE DIAMETE R	9	9	9	9	9	9	9	9	9	9	9	9	9	9	4	9	9	9	2	4	9	2	00	4	4	9	∞	∞	8	8	∞	∞
DESCRIPTION	SAMINO DE CIELC	CAMINO DE CIELC	POPLAR ST.	PINE ST.	E. SIXTH AVE.	E. SIXTH AVE.	MARIE ST.	MARIE ST.	E. FIFTH AVE.	CORONA AVE.	E. FIFTH AVE.	E. SECOND AVE.	E. SECOND AVE.	MARIE ST.	PERSHING ST.	CORONA AVE.	HOLDEN RD.	E. FIFTH AVE.						CORBETT ST.	FOCH ST.	PERSHING ST.	W. NINTH AVE.	W. NINTH AVE.	PERSHING ST.	PH EDWARDS PA	E. EIGHTH AVE.	E. EIGHTH AVE.
GISID	255	256	285	300	302	303	339	340	345	348	429	637	638	739	761	787	206	953	966	1038	1051	1090	958	995	621	641	652	653	959	. F59	662	663

APPENDIX A Risk Scoring of Water System Distribution System Assets and Estimated Replacement Value/Current Value

sset	25,732	26,634	11,097	26,894	3,162	25,109	20,119	20,692	19,310	21,711	46,751	26,373	25,839	10,555	81,979	43,187	18,239	33,383	24,129	8,471	11,322	12,176	12,434	10,084	22,306	30,004	30,007	29,173	30,005	30,411	5,300	2,783
Estimated Asser Current Value	2	2		2		2	2	2	1	2	4	2	2		w	7																
	5 \$	3 \$	\$ 6	\$ 0	\$ \$	\$ 6	\$ \$	\$ 9	\$ 0	2 \$	5 1	4 \$	\$ 0	4 \$	3 \$	\$ 1	3 \$	5 1	4 \$	\$ 0	5 \$	4 \$	\$ 6	\$ 0	7 \$	4 \$	\$ 6	2 \$	5 \$	7 \$	4 \$	2 \$
Estimated Asset Replacement Valu	61,006	63,143	26,309	63,760	7,498	59,529	47,698	49,056	45,780	51,472	110,837	62,524	61,260	25,024	86,823	94,577	39,943	43,617	42,614	14,960	19,995	21,504	21,959	17,810	36,437	47,354	47,359	46,042	47,355	47,997	8,364	4,392
Es Rep	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	4	\$	\$	\$	\$	\$	\$	4	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$
SUM TOTAL PERCENT	79.05%	79.14%	79.18%	79.27%	79.29%	79.37%	79.46%	79.56%	79.65%	79.75%	79.96%	80.08%	80.14%	80.18%	80.35%	80.53%	80.61%	%69.08	80.78%	80.81%	80.85%	80.89%	80.93%	80.96%	81.03%	81.13%	81.22%	81.31%	81.40%	81.49%	81.51%	81.52%
RISK_LEVEL	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM
RISK	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.78	10.56	10.43	10.43	9.39	9.34	9.34	9.34	9.34	9.34	9.34	8.88	99.8	99.8	99.8	99.8	99.8	99.8	99.8
CONDITION SCORE	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.782	10.558	10.434	10.434	2.346	9.338	9.338	9.338	9.338	9.338	9.338	8.878	8.664	8.664	8.664	8.664	8.664	8.664	8.664
CONSEQUENCE	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	4.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
HydrantAge	1989	1981	1993	1981	1995	1995	1963	2013	2013	2013	1971	1993	1998	2003	2010	1971	2005	2009	1979	1983	1993	1993	1993	1993	1989	1996	1996	1991	1997	1995	1995	1991
AGE_SCORE	5.782	5.782	5.782	5.782	5.782	5.782	5.782	5.782	5.782	5.782	5.782	5.782	5.782	5.782	0.558	5.434	5.434	2.346	4.338	4.338	4.338	4.338	4.338	4.338	3.878	3.664	3.664	3.664	3.664	3.664	3.664	3.664
AGE (in 2017)	70	70	70	70	70	70	70	70	70	70	70	70	70	70	7	29	29	27	57	57	57	57	57	57	28	27	27	27	27	27	27	27
SHAPE	347.83	360.01	150.00	363.53	42.75	339.41	362.61	372.93	348.03	391.31	842.61	356.48	349.28	142.67	660.05	719.00	303.66	331.59	323.96	113.73	152.01	163.48	166.94	135.39	277.00	360.00	360.03	350.02	360.01	364.88	63.59	33.39
YEAR	1947	1947	1947	1947	1947	1947	1947	1947	1947	1947	1947	1947	1947	1947	2010	1950	1950	1990	1960	1960	1960	1960	1960	1960	1989	1990	1990	1990	1990	1990	1990	1990
PIPE OJAMETE MATERIAL R	CI	Cl	CI	CI	CI	D	CI	CI	O	CI	CI	CI	CI	CI	PVC-C900	D	CI	COPPER	CI	CI	O	CI	CI	Cl	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40
PIPE DIAMETE R	8	8	∞	8	8	8	9	9	9	9	9	8	8	8	9	9	4	0.75	4	4	4	4	4	4	9	4	4	9	4	4	4	4
DESCRIPTION	E. EIGHTH AVE.	W. NINTH AVE.	W. NINTH AVE.	W. NINTH AVE.	W. NINTH AVE.	SPRUCE ST.	E. EIGHTH AVE.	E. EIGHTH AVE.	PERSHING ST.	SILVER ST.	COLEMAN ST.	IVY ST.		MARR AVE.	RIVERSIDE DR.	PERSHING ST.	PERSHING ST.	PERSHING ST.	PERSHING ST.	E COCHISE WAY	W. FIFTH AVE.	W. FIFTH AVE.	CITY ST.	W. CARTER ST.	TUNGSTEN ST.	MAGNOLIA ST.						
GIS ID	664	999	299	899	712	713	715	736	737	738	813	823	872	875	343	644	1103	1080	94	493	498	692	770	816	911	4	5	49	51	58	102	118

cet	e	28,617	1,851	1,809	28,431	29,805	2,941	856	29,807	39,790	1,443	25,508	14,964	23,900	31,672	57,514	15,618	26,427	30,879	14,465	8,479	30,005	2,240	60,924	30,004	30,304	17,506	30,012	30,266	35,801	76,114	20,854	32,414
Estimated Asset	Current Value (Based on	28	1	1	28	29	2		29	36		25	14	23	31	57	15	26	30	17		30)9	3(3(17	30	3(35	76	20	37
Estin	Ja D	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	43
Asset	Value 15)	45,166	2,921	2,855	44,871	47,040	4,642	1,512	47,044	62,798	2,277	40,258	23,617	37,720	49,987	90,772	24,649	41,709	48,734	22,829	13,381	47,356	3,535	96,153	47,354	47,827	27,629	47,367	47,768	56,504	120,127	32,913	51,158
Estimated Asset	Replacement Value (as of 2015)																														1		
Esti	Repla (a	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$
OTAL	ENT GTH)	51%	51%	.62%	%0,	%6,	%08	31%	%06	32%	35%	%01	82.15%	22%	82.32%	82.49%	82.54%	52%	72%	82.76%	82.79%	82.88%	%68	83.07%	83.17%	%97	83.31%	83.40%	83.50%	83.61%	83.84%	83.90%	84.00%
SUM TOTAL	PERCENT (LENGTH)	81.61%	81.61%	81.6	81.70%	81.79%	81.80%	81.81%	81.90%	82.02%	82.02%	82.10%	82.	82.22%	82.3	82.4	82.	82.62%	82.72%	82.	82.	82.8	82.89%	83.0	83.	83.26%	83.	83.4	83.	83.(83.8	83.0	84.0
	VEL	M	Σ	M	M	M	MI	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M	M
	RISK_LEVEI	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM
DICK	2	99'8	99.8	8.66	99.8	99'8	8.66	8.66	8.66	8.66	99.8	99.8	8.66	8.66	99.8	99.8	99.8	99.8	99.8	8.66	99.8	99.8	99.8	99.8	99.8	8.66	8.66	99.8	99.8	99.8	99.8	99	99.8
		80	8.	8	8.	8	∞	8	80	8.	8	8.	8.		89	8.	8.	8	8	oó.	8.	8	∞.	∞.	8	∞	89	∞	8	8	89	89	∞i
CONDITION	SCORE	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664	8.664
CONCEDITENCE	SCORE	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
CONICE	SC																																
	ntAge	1992	1995	1995	1995	1995	1995	1995	1978	1984	1984	2006	1963	1995	1995	1995-2003	1980	1995	1995	1980	1969	1995	2006	1993	1994	1995	1995	1995	1992	1990	1997	1997	1991
	HydrantAge	19	19	19	19	19	19	19	19	19	19	20	19	19	19	1995-	19	19	19	19	19	19	20	19	19	19	19	19	19	19	19	19	19
	CORE	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64	64
	AGE_SCORE	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664	3.664
re (In		7	7	7	7	7	7	7	7	1	7	7	1	7	2	7	7	1	7	1	1	7	7	1	7	1	7	7	7	7	7	7	7
	2017)	27	77	27	77	77	27	27	27	77	27	27	27	7.7	27	27	77	27	27	27	27	27	77	27	27	27	27	27	27	27	27	27	7.7
n n		.36	21	70	.12	.61	29	50	.64	.41	31	.05	.54	.76	.01	.07	.39	80.	.49	.55	.73	.01	87	.98	00.	.60	.04	60:	.14	.55	.23	.21	.92
	LENGTH	343.36	22.21	21.70	341.12	357.61	35.29	11.50	357.64	477.41	17.31	306.05	179.54	286.76	380.01	70.069	187.39	317.08	370.49	173.55	101.73	360.01	26.87	730.98	360.00	363.60	210.04	360.09	363.14	429.55	913.23	250.21	388.92
0 % 1	INSTALLED	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990	1990
																		10,3			0.00												
	MATERIAL	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40	PVC-SCH40
ų.																																	
PIPE	DIAMETE	4	4	4	4	4	4	4	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	9	. 4	. 4	4	4	9	9	9
	PTION	ST.	EIGHTH AVE.	EIGHTH AVE.	ST.	E ST.	ST.	SEVENTH AVE.	ST.	A ST.	E ST.	E ST.	AR ST.	T AVE.	R ST.	R ST.	ON ST.	IRY ST.	S ST.	ON ST.	IRY ST.	H AVE.	E ST.	ST.	HST.	FEN ST.	SECOND AVE	ND AVE	ST.	ST ST.	AY DR.		N AVE.
	DESCRIPTION	ELM ST	E. EIGHT	E. EIGHT	ELM ST	MAPLE ST	ELM ST	. SEVEN	IVY ST.	KOPRA ST	MARIE ST	MARIE ST.	N. CEDAR ST	E. FIRST AVE.	VEATER ST	VEATER ST.	SIMPSON ST	MERCURY ST	BRASS ST	SIMPSON ST	MERCURY ST	E. FIFTH AVE	MARIE ST	PINE ST.	FOCH ST	TUNGSTEN ST.	W. SECO	W. SECOND AVE	ELM ST.	LOCUST ST.	PARWAY DR		LINCOLN AVE.
	2	2			5	9	1	12 E.	1	2	9	7	1	9	0	7	2	3	00	5	7	2	6	4	5				6	0	5	9	
	GIS ID	122	123	124	125	126	171	172	191	192	196	197	201	203	220	227	232	233	238	245	247	282	299	304	435	455	460	615	669	740	745	746	774

ののないのである	行を続くてはなどは球を指揮を続	THE REAL PROPERTY.	のの情報は、記れるないないという	前のでから 記述の 流さ	0 0 000			CONTRACTOR AND		THE REAL PROPERTY.		A CHICAGO	AND COMPANY OF THE PARTY OF THE			
GIS ID	DESCRIPTION	DIAMETE R	MATERIAL	YEAR INSTALLED	SHAPE LENGTH	AGE (in 2017)	AGE_SCORE	HydrantAge	CONSEQUENCE	CONDITION	RISK SCORE	RISK_LEVEL	PERCENT	Estimal Replacer	Estimated Asset E Replacement Value	Estimated Asset Current Value
824	ELM ST.	4	PVC-SCH40	1990	338.83	27	3.664	1995	1.0	8.664	8.66	MEDIUM	84.09%	\$	44,570 \$	28,240
851	FIR ST.	9	PVC-SCH40	1990	366.00	27	3.664	1998	1.0	8.664	8.66	MEDIUM	84.18%	\$	48,144 \$	30,504
867	ELM ST.	4	PVC-SCH40	1990	14.50	27	3.664	1992	1.0	8.664	99.8	MEDIUM	84.19%	\$	1,907 \$	1,208
944	PINE ST.	9	PVC-SCH40	1990	253.31	27	3.664	1995	1.0	8.664	99.8	MEDIUM	84.25%	\$	33,321 \$	21,112
945	E. AURORA AVE.	9	PVC-SCH40	1990	55.11	27	3.664	1995	1.0	8.664	99'8	MEDIUM	84.26%	\$	7,250 \$	4,593
946	E. AURORA AVE.	9	PVC-SCH40	1990	54.70	27	3.664	1995	1.0	8.664	99.8	MEDIUM	84.28%	\$	7,195 \$	4,559
947	E. AURORA AVE.	9	PVC-SCH40	1990	58.46	27	3.664	1995	1.0	8.664	99.8	MEDIUM	84.29%	\$	\$ 689'2	4,872
979		8	PVC-SCH40	1990	1035.98	27	3.664	1991	1.0	8.664	99.8	MEDIUM	84.56%	\$	181,701 \$	115,128
866		2	PVC-SCH40	1990	301.46	27	3.664	1991	1.0	8.664	99.8	MEDIUM	84.64%	\$	39,654 \$	25,125
1005		0.75	PVC-SCH40	1990	262.91	27	3.664	1998	1.0	8.664	99.8	MEDIUM	84.70%	\$	34,583 \$	21,912
1020		0.75	PVC-SCH40	1990	459.22	27	3.664	1984	1.0	8.664	8.66	MEDIUM	84.82%	\$	\$ 90,409	38,274
1021		1	PVC-SCH40	1990	166.54	27	3.664	1984	1.0	8.664	8.66	MEDIUM	84.86%	\$	\$ 706,12	13,881
1022		2	PVC-SCH40	1990	1080.98	27	3.664	1984	1.0	8.664	99.8	MEDIUM	85.14%	\$	142,192 \$	90,094
1037		1	PVC-SCH40	1990	206.68	27	3.664	1969	1.0	8.664	99.8	MEDIUM	85.19%	\$	27,187 \$	17,226
1041		2	PVC-SCH40	1990	89.75	77	3.664	1976	1.0	8.664	8.66	MEDIUM	85.21%	\$	11,806 \$	7,480
1064		0.75	PVC-SCH40	1990	269.78	27	3.664	1994	1.0	8.664	99.8	MEDIUM	85.28%	\$	35,487 \$	22,485
1077		2	PVC-SCH40	1990	685.42	27	3.664	1986	1.0	8.664	99.8	MEDIUM	85.46%	\$	\$ 091,06	57,126
1078		0.75	PVC-SCH40	1990	208.96	27	3.664	1995	1.0	8.664	99'8	MEDIUM	85.51%	\$	27,487 \$	17,416
1081		0.75	PVC-SCH40	1990	221.81	27	3.664	1993	1.0	8.664	99.8	MEDIUM	85.57%	\$	29,177 \$	18,487
1082		4	PVC-SCH40	1990	199.32	7.7	3.664	1995	1.0	8.664	99.8	MEDIUM	85.62%	\$	26,219 \$	16,612
1083		2	PVC-SCH40	1990	216.62	77	3.664	1995	1.0	8.664	99.8	MEDIUM	85.67%	\$	\$ 494 \$	18,054
1084		0.75	PVC-SCH40	1990	275.35	27	3.664	1995	1.0	8.664	99.8	MEDIUM	85.74%	\$	36,219 \$	22,949
1085		1	PVC-SCH40	1990	249.33	27	3.664	1995	1.0	8.664	99.8	MEDIUM	85.81%	\$	32,797 \$	20,780
1086		0.75	PVC-SCH40	1990	95.02	27	3.664	1995	1.0	8.664	99.8	MEDIUM	85.83%	\$	12,499 \$	7,920
1091		4	PVC-SCH40	1990	604.39	77	3.664	2009	1.0	8.664	99.8	MEDIUM	85.99%	\$	79,502 \$	50,373
355		∞	PVC-SCH40	1991	146.72	26	3.455	1991	1.0	8.455	8.46	MEDIUM	86.02%	\$	25,733 \$	16,841
413		∞	PVC-SCH40	1991	117.02	26	3.455	1991	1.0	8.455	8.46	MEDIUM	86.05%	\$	\$ 20,525 \$	13,433
414		00	PVC-SCH40	1991	216.84	26	3.455	1991	1.0	8.455	8.46	MEDIUM	86.11%	\$	38,032 \$	24,891
416		00	PVC-SCH40	1991	160.01	26	3.455	1991	1.0	8.455	8.46	MEDIUM	86.15%	\$	28,064 \$	18,367
910	N BOSQUE DR.	9	PVC-SCH40	1991	551.33	26	3.455	1999	1.0	8,455	8.46	MEDIUM	86.29%	\$	72,521 \$	47,462
442	MARSHALL ST.	9	CI	1970	159.05	47	3.342	1976	1.0	8.342	8.34	MEDIUM	86.33%	\$	20,921 \$	13,930
443	MARSHALL ST.	9	CI	1970	163.96	47	3.342	1976	1.0	8.342	8.34	MEDIUM	86.37%	\$	21,567 \$	14,360

を 場合を できる と は と から	Current Value (Based on	28,520	14,577	28,942	11,315	31,596	31,452	30,876	30,783	30,977	13,258	30,472	31,553	30,361	31,160	15,837	24,502	3,656	6,650	4,067	3,291	45,529	13,372	15,986	17,606	18,233	7,561	13,770	32,389	31,982	747	12,299	
	Curr (B	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$ (\$ \$	\$	\$ (\$	\$ 6	\$	\$	\$	\$ 9	\$,	
	Estimated Asset Replacement Value (as of 2015)	42,833	21,893	43,467	16,994	47,454	47,237	46,373	46,232	46,524	19,912	45,766	47,390	45,599	46,799	23,785	36,799	5,418	8,803	5,384	4,357	60,269	47,356	56,612	62,350	64,572	26,779	48,765	114,703	113,263	2,646	43,557	
经股份股份债务公司	Estima Replacei	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	
等 对非常是 形 医脐结	PERCENT (LENGTH)	86.46%	86.50%	86.58%	86.62%	86.71%	%08.98	%68.98	%86.98	87.07%	87.11%	87.20%	87.29%	87.38%	87.47%	87.50%	87.56%	87.57%	82.78%	87.59%	87.59%	87.71%	87.80%	87.91%	88.03%	88.16%	88.21%	88.31%	88.53%	88.75%	88.75%	88.84%	
Value	RISK_LEVEL	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MEDIUM	MOT	TOW	MOT	NOT	MOT	TOW	TOW	TOW	LOW	MOT	LOW	LOW	TOW	TOW	
iej callelli	RISK	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.34	8.25	7.45	7.45	7.45	7.45	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	Control of the Contro
placellielit vait	CONDITION	8.342	8.342	8.342	8.342	8,342	8.342	8.342	8.342	8.342	8.342	8.342	8.342	8.342	8.342	8.342	8.342	8.253	7.446	7.446	7.446	7.446	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	CALIFORNIA PROPERTY AND ADDRESS OF THE PARTY A
id Estimated he	CONSEQUENCE	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	というないというないないないないのできょう とうないしん
System Assets a	HydrantAge	1976	1997	1971	1997	1995	1976	1976	1999	2012	1976	1992	1986	1992	1995	2012	2012	2000	1983	1989	1989	1984	2006	1991	1991-1997	1978	1981	1995	1963	1984	1993	1993	Company of the State of the Sta
elli Distribution	AGE_SCORE	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.342	3.253	2.446	2.446	2.446	2.446	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	
i water syst	AGE (in 2017)	47	47	- 47	47	47	47	47	47	47	47	47	47	47	47	47	47	25	37	37	37	37	57	57	57	57	57	57	57	57	57	57	OF STREET OF STREET
RISK SCOFING OF WATER S	SHAPE	325.63	166.43	330.45	129.20	360.76	359.10	352.54	351.47	353.69	151.37	347.92	360.27	346.66	355.78	135.61	209.81	41.19	50.19	30.70	24.84	458.18	360.01	430.38	474.00	490.89	203.58	370.72	872.00	861.05	20.11	331.13	
	YEAR INSTALLED	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1970	1992	1980	1980	1980	1980	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	
	MATERIAL	CI	O	CI	CI	CI	O	Cl	C	Cl	ō	CI	O	CI	CI	ō	O	PVC-SCH40	ō	ō	ō	ō	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	
	PIPE DIAMETE MATERIAL R	9	4	4	9	9	9	9	9	9	9	9	9	9	9	80	8	9	8	8	8	9	9	9	9	4	9	9	4	4	4	4	
	DESCRIPTION	MARSHALL ST.	W. THIRD AVE.	COLEMAN ST.	LANE ST.	E. SIXTH AVE.	MARSHALL ST.	MARSHALL ST.	IVY ST.		MARSHALL ST.	E. SIXTH AVE.		E. SIXTH AVE.	E. SIXTH AVE.			RIVERSIDE DR.	COOK ST.	E. EIGHTH AVE.	KRUGER ST.	JUNIPER ST.	MARIE ST.	MYRTLE ST.	MYRTLE ST.	PALO VERDE ST.	VEATER ST.	W. SECOND AVE.	IVY ST.	LOCUST ST.	PINE ST.	PINE ST.	TO THE RESIDENCE OF THE PARTY O
	GIS ID	444	617	626	631	699	718	719	753	764	809	827	828	829	830	666	1023	77.2	655	999	672	750	194	221	222	432	441		461	462	479	480	Contraction of the Contraction o

	Estimated Asset Current Value (Based on	12,616	8,019	9,574	863	3,522	2,338	1,993	1,083	12,037	13,334	13,000	26,745	13,001	31,888	13,052	13,186	13,493	25,598	25,782	1,330	26,706	17,836	33,727	11,010	13,371	25,603	11,518	17,829	16,693		10,707
		44,679 \$	28,400 \$	33,907 \$	3,056 \$	12,473 \$	8,281 \$	\$ 090'2	3,834 \$	42,629 \$	47,223 \$	46,039 \$	94,716 \$	46,042 \$	112,931 \$	46,223 \$	\$ 769,697	47,785 \$	\$ 859'06	91,305 \$	4,709 \$	94,578 \$	63,165 \$	119,443 \$	38,992 \$	47,354 \$	90,671 \$	40,790 \$	63,141 \$	\$ 9,119 \$		37,920 \$
	Estimated Asset Replacement Value (as of 2015)	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	4	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	\$	Ų	t,
	SUM TOTAL PERCENT (LENGTH)	89.16%	89.22%	89.29%	89.29%	89.32%	89.33%	89.35%	89.35%	89.44%	89.53%	89.62%	89.80%	89.89%	90.11%	90.20%	90.29%	90.38%	%95'06	90.74%	90.74%	90.93%	91.02%	91.19%	91.27%	91.36%	91.54%	91.62%	91.71%	91.82%	91 90%	07070
value	RISK_LEVEL	LOW	LOW	LOW	LOW	TOW	ГОМ	LOW	LOW	LOW	LOW	MOT	NOT	LOW	NOT	TOW	TOW	TOW	TOW	MOT	TOW	MOT	MOT	MOT	MOT	NOT	MOT	MOT	MOT	MOT	MOI	:
rej cui leilt	RISK	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	7.18	
מרכוווכוור אמונ	CONDITION SCORE	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	
na Estimated Nepi	CONSEQUENCE SCORE	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
שליבווו שישבוים מ	HydrantAge	1972	1972	1972	1962	1972	2003	2003	2003	2005	1996	1993	2005	1998	2013	1976	1981	2005	1995	1981	1999	1993	1999	1989	1978	1998	1999	1972	2009	2005	1978	CONTRACTOR
III DISCIEDACIOII	AGE_SCORE	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	7.176	
or state of other	AGE (in 2017)	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	57	
00	SHAPE	339,66	215.91	77.73	23.23	94.82	62.95	53.67	29.14	324.08	359.00	350.00	720.06	350.02	858.53	351.40	355.00	363.27	689.17	694.13	35.80	719.01	360.14	681.01	296.43	360.00	689.31	310.10	360.00	449.44	288.27	
THE REAL PROPERTY AND ADDRESS OF THE PERSON NAMED IN COLUMN TWO PERSONS AND ADDRESS OF THE PERSON NAMED IN COLUMN TWO PERSONS AND ADDRESS OF THE PERSON NAMED IN COLUMN TWO PERSON NAMED IN COLUMN TRANSPORT NAMED IN COLUMN TWO PERSON NAMED IN COLUMN TRANSPORT NAMED IN COLUMN TWO PERSON NAMED	YEAR INSTALLED	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	1960	
	PIPE DIAMETE MATERIAL R	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	AC	
CA SERVICE AND	PIPE DIAMETE R	4	4	4	4	4	9	9	9	9	9	9	9	9	9	9	. 9	9	9	9	9	9	∞	00	9	9	9	4	8	4	9	
	DESCRIPTION	CABALLO RD.	CABALLO RD.	DUNN ST.	BIRCH ST.	CABALLO RD.	PERSHING ST.	PERSHING ST.	PERSHING ST.	W. NINTH AVE.	N. GRAPE ST.	W. FOURTH AVE.	W. EIGHTH AVE.	W. EIGHTH AVE.	POPLAR ST.	SILVER ST.	GOLD ST.	MAGNOLIA ST.	MAGNOLIA ST.	GOLD ST.	SILVER ST.	KRUGER ST.	W. FOURTH AVE.	E. EIGHTH AVE.	W. NINTH AVE.	W. EIGHTH AVE.	SILVER ST.	WADE ST.	W. FOURTH AVE.	SIERRA VISTA DR.	W. NINTH AVE.	
が原状にはは他のないので	GIS ID	200	501	502	909	509	514	515	516	517	519	520	528	529	530	544	545	547	548	549	550	570	597	601	642	689	200	702	730	733	735	(1) ののでは、これ

PIPE DIAMETE	MATERIAL	YEAR	SHAPE	AGE (in	AGE_SCORE	HydrantAge	CONSEQUENCE	CONDITION	RISK	RISK_LEVEL	PERCENT	Replacement Value		Current Value
											(LENGTH)	(as of 2015		(Based on
	AC	1960	863.79	57	7.176	2001-2005	1.0	7.176	7.18	TOW	92.42%	\$	113,623 \$	32,084
	AC	1960	330.01	57	7.176	1981	1.0	7.176	7.18	TOW	92.51%	\$	43,410 \$	12,258
	AC	1960	360.09	57	7.176	2009	1.0	7.176	7.18	TOW	95.60%	\$	63,157 \$	17,834
Barry.	AC	1960	324.75	57	7.176	1995	1.0	7.176	7.18	TOW	95.68%	\$	42,718 \$	12,062
1355	AC	1960	39.53	57	7.176	1995	1.0	7.176	7.18	TOW	95.69%	\$	5,200 \$	1,468
	AC	1960	388.46	57	7.176	1972	1.0	7.176	7.18	MOT	92.79%	\$	\$ 860,15	14,429
	AC	1960	82.76	57	7.176	1981	1.0	7.176	7.18	LOW	92.81%	\$	\$ 788,01	3,074
	AC	1960	176.66	57	7.176	1980	1.0	7.176	7.18	LOW	92.86%	\$	23,238 \$	6,562
	AC	1960	63.02	57	7.176	1998	1.0	7.176	7.18	row	92.87%	\$	\$ 062'8	2,341
100000	AC	1960	809.45	57	7.176	1997	1.0	7.176	7.18	LOW	93.08%	\$	106,474 \$	30,065
10000	AC	1960	45.44	57	7.176	1988	1.0	7.176	7.18	LOW	93.09%	\$	\$ 776,5	1,688
CHE!	AC	1960	195.55	57	7.176	1983	1.0	7.176	7.18	TOW	93.14%	\$	34,297 \$	9,684
	AC	1960	218.22	57	7.176	1983	1.0	7.176	7.18	LOW	93.20%	\$	38,274 \$	10,807
1300	AC	1960	153.10	57	7.176	1983	1.0	7.176	7.18	row	93.23%	\$	\$ 853 \$	7,582
100	PVC-SCH40	2000	33.02	17	1.831	2005	1.0	6.831	6.83	LOW	93.24%	\$	4,344 \$	3,549
	PVC-SCH40	2000	76.06	17	1.831	2005	1.0	6.831	6.83	TOW	93.27%	\$	11,966 \$	9,775
	PVC-SCH40	2000	28.38	17	1.831	2005	1.0	6.831	6.83	LOW	93.27%	\$	3,733 \$	3,050
	PVC-SCH40	2000	476.10	17	1.831	2003	1.0	6.831	6.83	LOW	93.39%	\$	83,504 \$	68,215
	PVC-SCH40	2000	743.88	17	1.831	1969	1.0	6.831	6.83	MOT	93.58%	\$	\$ 058'26	79,935
	PVC-SCH40	2000	384.58	17	1.831	1986	1.0	6.831	6.83	MOT	93.68%	\$	\$ 885'05	41,326
	PVC-SCH40	2000	513.95	17	1.831	1989	1.0	6.831	6.83	row	93.81%	\$	67,604 \$	55,227
	PVC-SCH40	2000	16.83	17	1.831	2005	1.0	6.831	6.83	TOW	93.82%	\$	2,214 \$	1,809
	PVC-SCH40	2000	360.03	17	1.831	1977	1.0	6.831	6.83	TOW	93.91%	\$	47,359 \$	38,688
	PVC-SCH40	2000	182.52	17	1.831	1997	1.0	6.831	6.83	LOW	93.96%	\$	24,009 \$	19,613
	PVC-SCH40	2000	315.04	17	1.831	2009	1.0	6.831	6.83	LOW	94.04%	\$	41,440 \$	33,853
	PVC-SCH40	2000	333.59	17	1.831	2005	1.0	6.831	6.83	LOW	94.12%	\$	43,881 \$	35,847
	PVC-SCH40	2000	316.53	17	1.831	2005-2009	1.0	6.831	6.83	MOT	94.20%	\$	41,636 \$	34,013
	PVC-SCH40	2000	350.25	17	1.831	2000	1.0	6.831	6.83	TOW	94.29%	\$	46,072 \$	37,636
	PVC-SCH40	2000	236.32	17	1.831	1976	1.0	6.831	6.83	TOW	94.35%	\$	31,085 \$	25,394
	PVC-SCH40	2000	1138.88	17	1.831	2000	1.0	6.831	6.83	LOW	94.64%	\$	149,809 \$	122,380
	PVC-SCH40	2000	314.16	17	1.831	1983	1.0	6.831	6.83	TOW	94.72%	٠,	41,325 \$	33,759
	PVC-SCH40	2000	450.15	17	1.831	2010	1.0	6.831	6.83	LOW	94.84%	\$	59,212 \$	48,371

GISID	DESCRIPTION	PIPE DIAMETE	PIPE MATERIAL I	YEAR	SHAPE	AGE (in 2017)	AGE_SCORE	HydrantAge	CONSEQUENCE	CONDITION SCORE	RISK SCORE	RISK_LEVEL	SUM TOTAL PERCENT (LENGTH)	Estimated Asset Replacement Value (as of 2015)		Estimated Asset Current Value (Based on
195	KOPRA ST.	9	D	1990	880.61	27	1.650	1984	1.0	6.650	6.65	LOW	92.06%	\$ 1	115,835 \$	96,726
338	MARIE ST.	9	O	1990	245.64	27	1.650	1984	1.0	6.650	6.65	LOW	95.13%	\$	32,312 \$	26,982
341	MARIE ST.	9	D	1990	45.31	27	1.650	2006	1.0	6.650	6.65	row	95.14%	\$	5,961 \$	4,977
685	MARIE ST.	9	D	1990	335.35	27	1.650	2006	1.0	6.650	6.65	LOW	95.22%	\$	44,112 \$	36,835
747	MARIE ST.	9	D	1990	135.62	27	1.650	2006	1.0	6.650	6.65	MOT	95.26%	\$	17,840 \$	14,897
1019		0.75	COPPER	2000	995.79	17	1.256	2006	5.0	1.256	6.28	LOW	95.51%	\$	\$ 986'081	114,530
622	MARIE ST.	4	D	2000	14.45	17	0.954	2006	1.0	5.954	5.95	MOT	95.52%	\$	1,900 \$	1,719
714	JUNIPER ST.	4	D	2000	866.99	17	0.954	2006	1.0	5.954	5:95	MOT	95.74%	\$	114,044 \$	103,168
294	SPRUCE ST.	9	PVC-SCH40	2010	857.33	7	0.558	2013	1.0	5.558	5.56	LOW	95.96%	\$	112,773 \$	106,482
295	MARIE ST.	9	PVC-SCH40	2010	360.00	7	0.558	2013	1.0	5.558	5.56	MOT	%50.96	\$	47,355 \$	44,713
298	MARIE ST.	9	PVC-SCH40	2010	300.03	7	0.558	2013	1.0	5.558	5.56	LOW	96.12%	\$	39,466 \$	37,264
686		9	PVC-SCH40	2010	22.71	7	0.558	2013	1.0	5.558	5.56	TOW	96.13%	\$	2,987 \$	2,820
066		9	PVC-SCH40	2010	391.49	7	0.558	2013	1.0	5.558	5.56	LOW	96.23%	\$	51,497 \$	48,624
955		8	PVC-C900	1996	267.93	21	2.497	1996	2.0	2.497	4.99	LOW	%08.36	\$	46,993 \$	35,259
926		00	PVC-C900	1996	215.87	21	2.497	1996	2.0	2.497	4.99	MOT	96.35%	\$	37,862 \$	28,408
957		8	PVC-C900	1996	527.85	21	2.497	1998	2.0	2.497	4.99	MOT	96.49%	\$	\$ 62,26	69,463
696		9	PVC-C900	2009	303.79	8	0.660	2006	7.0	0.660	4.62	MOT	%25.96	\$	\$ 096'68	37,323
691	JUNIPER ST.	9	PVC-C900	2005	136.57	12	1.124	2000	4.0	1.124	4.50	LOW	%09'96	\$	17,964 \$	15,944
693	IVY ST.	4	PVC-C900	2005	373.57	12	1.124	2005	4.0	1.124	4.50	LOW	%02'96	\$	49,140 \$	43,614
744	IVY ST.	9	PVC-C900	2005	722.00	12	1.124	1999	4.0	1.124	4.50	TOW	%88.96	\$	94,972 \$	84,294
749	JUNIPER ST.	9	PVC-C900	2005	382.88	12	1.124	1999	4.0	1.124	4.50	MOT	%86'96	\$	50,364 \$	44,701
296		10	PVC-C900	2010	324.31	7	0.558	2011	7.0	0.558	3.91	MOT	%90'.26	\$	71,101 \$	67,134
896		10	PVC-C900	2010	693.39	7	0.558	2011	7.0	0.558	3.91	MOT	97.24%	\$	152,018 \$	143,537
1047		12	PVC-C900	2010	47.69	7	0.558	2011	7.0	0.558	3.91	LOW	97.25%	\$	12,546 \$	11,846
1109		0	Unknown	1980	1267.71	37	3.696		1.0	3.696	3.70	row	97.57%	\$	166,755 \$	105,117
961		9	PVC-C900	2000	1138.02	17	1.831	2003	2.0	1.831	3.66	row	898.76		149,695 \$	122,287
1094		9	PVC-C900	2000	203.91	17	1.831	2006	2.0	1.831	3.66	TOW	97.92%	\$	\$ 228,92	21,911
991		9	PVC-C900	2013	796.76	4	0.285	2013	9.0	0.285	2.57	LOW	98.12%	\$	104,806 \$	101,817
1049		0.75	COPPER	2010	751.72	7	0.426	2010-2011	6.0	0.426	2.56	LOW	98.31%	\$	\$ 188,881	94,666
1055		0	Unknown	2005	177.48	12	0.809	2005	3.0	0.809	2.43	LOW	98.36%	\$	23,345 \$	21,457
1088	H4 11 11 11 11 11 11 11 11 11 11 11 11 11	1	POLY	1990	157.43	27	2.346	1993	1.0	2.346	2.35	TOW	98.40%	\$	\$ 802,02	15,849
1089		1	POLY	1990	245.54	27	2.346	1993	1.0	2.346	2.35	TOW	98.46%	\$	32,298 \$	24,720

APPENDIX A

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GIS ID	PIPE DESCRIPTION DIAMETE MATERIAL R	PIPE AMETE M/ R		YEAR INSTALLED	SHAPE	AGE (in 2017)	AGE_SCORE	HydrantAge	CONSEQUENCE SCORE	CONDITION	RISK	RISK_LEVEL	SUM TOTAL PERCENT	Estimated Asset Replacement Value		Estimated Asset Current Value
959		0 PV	PVC-C900	2000	481.63	17	1.831	2003	1.0	1.831	1.83	WOI	98 58%	\$ 63 353	~	51 754
982		6 PV	PVC-C900	2000	1028.91	17	1.831	1994	1.0	1.831	1.83	LOW	98.85%			110 563
1092		2 PV	PVC-C900	2000	383.86	17	1.831	1999	1.0	1.831	1.83	LOW	98.94%	\$ 50.497	* 0	41 248
1099	SILVER ST.	6 PV	PVC-C900	2000	1173.58	17	1.831	2010	1.0	1.831	1.83	LOW	99.24%	\$ 154.373		126.109
1010	0	0.75 C	COPPER	2000	359.86	17	1.256	2005	1.0	1.256	1.26	LOW	99.33%	\$ 47.336		41.389
1017		2 CI	COPPER	2000	62.38	17	1.256	2006	1.0	1.256	1.26	TOW	99.35%	\$ 8.205		7175
1018		1 C	COPPER	2000	354.97	17	1.256	2006	1.0	1.256	1.26	NOT	99 44%	\$ 46.697		40.876
, 616	JUNIPER ST.	9 PV	PVC-C900	2005	354.14	12	1.124	2004	1.0	1.124	1.12	MOI	99 53%	\$ 46 584		71 346
618	JUNIPER ST.	4 PV	PVC-C900	2005	193.46	12	1.124	1999	1.0	1.124	1.12	MOI	%85.66	\$ 75,448	· ·	27.586
624	JUNIPER ST.	4 PV	PVC-C900	2005	587.87	12	1.124	2000	1.0	1.124	1.12	TOW	99 73%	\$ 77.328		68 633
811	JUNIPER ST.	9 PV	PVC-C900	2005	331.39	12	1.124	1984	1.0	1.124	1.12	LOW	99.82%	\$ 43.591	× ×	38.690
1068		4 PV	PVC-C900	2005	361.00	12	1.124	2004	1.0	1.124	1.12	LOW	99.91%	\$ 47.486		42.147
1073		8 PV	PVC-C900	2005	360.02	12	1.124	1999	1.0	1.124	1.12	TOW	100.00%	\$ 63.143	. 5	56.043
										Shippe of a comment of a comment	Suntill Market Spirit State				CONTRACTOR CONTRACTOR	The second second second

APPENDIX B

Inventory, Risk Scoring, and Estimated Replacement Value/Current Value of Water System Well, Pump System, Buildings/Structures, and Storage Tank Assets

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	Notes/Comments	NOTES COMM	Well Pump Characteristics: Submersible; 30 HP, capacity of 450 GPM; set at depth of 220 feet. Info Sources: 2007 IMED Sanitary Survey and 1991 OSE Well Record, Capacity setlmated from 2007 IMRID Survey.	Well Pump Characteristics: Turbine; 25 HP; capacity of 500 GPM; set at depth of 150 feet, and 150 feet at 150 feet. Capacity of 150 feet at 150 feet a	Well Pump Characteristics: Submersible; 30 HP, capacity of 240 GPM; set at depth of 220 feet. Info Source: 2007 NMED Sanitary Survey.	Pump house replaced in 1999, but well was not replaced/rehabbed. Well Pump Characteristics: Turbine; 60 HP; capacity of 600 GPW; set into Source; 2007 NMED Sanitary Survey.	Well Pump Characteristics: Turbine; 75 HP; capacity of 900 GPM; set at depth of 260 feet. Info Source; 2007 NMED Sanitary Survey and Operator Interview.	Well Pump Characteristics: Turbine; 60 HP; capacity of 640 GPM; set at depth of 280 feet. Info Source: 2007 NMED Sanitary Survey.
	Estimated Asset Value (Replacement	ASSET_VALUE	\$25,000	\$25,000	\$25,000	\$30,000	\$35,000	\$35,000
	Driller	DRILLER	Larjon Orilling Co.	Layne Texas Co.	Johnson Drilling Co.	Rayford Guffey	Roger & Co., Inc.	Roger & Co., Inc.
	Replacement/ Rehab Cost	RPC_RHB_CT					\$58,000	
	Year Replaced/ Rehabbed	T YR RPC RHB					2014/2015	
	Initial	INIT_COST						
	Year installed	YR_INSTL	1990	1945	1958	1976	1999	1999
	atic Water Casing Casing Depth Casing Screen Depth Weels (Inch)	SCRN_DP_FT			N/A	230-507		
8	Casing	CAS_MAT	Steel	Steel	Steel	Steej	Steel	Steel
APPENDIX B	Casing Depth (Feet)	CAS_DP_FT		420	235	511	009	009
to the state of th	Casing Diameter (Inch)	CAS DIA IN	16	12	16	12	12	12
JAN CAO F	2 -2	SWL_FT	80	08	8	8	08	08
	Well Depth (Feet)	WELL DP_FT	395	420	404	511	009	009
	Top of Well Elevation (Feet)	TOW EL FT					4238	4264
;	Capacity/ Production (GPM)	CAPTY_GPM	375	280	240	009	006	640
	SEO Well	SEO_NO	HS-11-5-2	HS-11	HS-11-S-5	HS-11-5-4	HS-11-S-10	HS-11-S-9
	System Well No.	WELL NO	Well #1	Well #2	Weil#4	Well #6	Well #7	Wel) #8
	Location Description	LOC_DESC	Cook St. Pump Station (Water Treatment Facility)	South side of Cook St. about 600 feet from Cook St. Water Treatment Facility	Approx 250 southwest of Intersection of Ridge Rd.and Central Ave.	Approx. 380 feet NW of the Intersectin of S Broadway St. and Rio Grande Ave.	NE corner of the Intersection of Veater St. and Hyde Ave.	NW corner of City Utility Yard Property approx 300 due west of Michigan St:
	Easting	EASTING	772105.1	771994.8	771309.4	771116.1	770842.9	771774.1
	Northing	NORTHING	2890347.9	2889685.8	2887377.1	2889366.5	2890720.6	2888356.8
	Asset ID (GIS Point No./ID)	GE.	0		2	e e	4	v

APPENDIX B
T or C Water System Inventory: Pumps Systems - Pumps

			and O&M	and O&M	ch rated to 600 3) -Distributor (2014.	common (common (pprox. 2012 and	pprox. 2012 and	r interview and	from the 1970s(coord has 1961. , Operator	or interview and	bbed and n. Info Sources: (erator interview	As-builts,	r 2014. Info	ach rated to 600g 03) -Distributor
	Notes/Comments		Pomp in very good condition Pumps observed producing 100 PSI in pipe manifold. Infro source: Site Visit, Oper, Interview, submittel date, and ORAM Manual	Pump In very good condition Pumps observed producing 100 PSI in pipe manifold. Info source: Sire Visit, Oper. Interview, submittal data, and O&M Manual	Duplex Booster Skid-Mounted System with 2 pumps each rated to 600 ⁵ GPM, 604P (Model # TDH-1200-90/System ID # 07-0203) -Distributor ₍ IC&H, Pump in very good condition - Impeller was replaced in 2014.	Small package system with two pumps, connected in a common (manifold, all housed in a small fiberglass dome. Pump No. 1 in good condition. (info sources: 2007 NMED Sankary Survey and Operator interview and Site Visit.	Small padage system with two pumps, connected in a common manifold, all housed in a small fiberglass dome. I manifold, all housed in a small fiberglass dome. I may No. 1 in good condition. Info Sources 2007 NMED Sanitary Survey and Operator interview and Site Visit.	REPURPOSED - pumps were removed from station in approx. 2012 and put into storage at the WWTP site.	REPURPOSED - pumps were removed from station in approx. 2012 and put into storage at the WWTP site.	Pump replace in 2001 and in good condition. You cobserved at 500 GPM using new flow mater. Info Sources: 2007 NMED Sanitary Survey and Operator interview and Sire Visit.	Pump in fair to proor condition. Operator believes it is from the 19708(visual inspection indicates it is at least that old, OSE Record, Deerator info Sources; 2007 NMED Sanitary Survey, OSE Record, Operator (Interview.	Pump replaced in approx. 2001 and in good condition interview and info Sources: 2007 NMED Sanitary Survey and Operator interview and Site Visit.	Flow observed at 850 GPM at 30 PSI. Purry in good condition. **Purny was Salvaged/Rehabbed and reinstalled in 1999 as part of new building construction. Info Sources: Coop NuRID Sanitary Survey, IH 1998-99 As-builts, Operator Interview and Site Visit.	Pump in good condition. Info Sources: 2007 NMED Santary Survey, LH 1998-99 As-bullts, Operator interview and Site Visit.	*Pump in fair condition - Bearing to be replaced winter 2014, info Sources: 2007 NNRD Sanitary Survey, LH 1998-99 As-builts, Operator (Interview and Site Visit.	Ouplex Booster Skid-Mounted System with 2 pumps each rated to SDQE GPM, GHP (Model # TDH-1200-90/System ID # 07-0203) -Distributor JCBH. JCBH. Pump in very good condition
	Estimated Asset Value (Replacement Value)	ASSET_VALUE	\$120,000	\$120,000	\$25,000	\$1,000	\$500	\$25,000	\$25,000	\$18,000	\$15,000	\$15,000	\$25,000	000′0ε\$	\$25,000	\$25,000
	Project No./ System No./ Job No., Etc.	PROJECT_NO	Factory Order #8G6- 10241/P.O. # 204513	Factory Order #8G6- 10241/P.O. # 204513	System ID # 07-0203			Job# 96-065; PO# 204514 (JC&H)								System ID # 07-0203
	Serial No.	SERIAL_NO	96-10241-1	96-10241-2												
	Model No.	MOD_NO			TDH-1200-90											TDH-1200-90
i of C water system inventory, rumps systems - rumps	Manufacturer	MANUFACT	Verti-Line	Verti-Line	Pumps: Aurora Pump; System: Canariis Corp.			Aurora Pump	Aurora Pump	Red Jacket	Layne and Bowler, Inc.		Goulds Pumps, Inc.			Pumps: Aurora Pump; System: Canariis Corp.
entory: rumps	Replacement/ Rehab Cost	RPC_RHB_CT			\$3,000								\$1400**			N/A
er system my	Year Replaced/ Rehabbed	YR_RPC_RHB		- "	2012					2001	1961	2001	1999	,	2014-2015*	A/A
ו סגר אאמ	Initial Cost	INIT_COST														
	Year	YR_INST	1996	1996	2007	5006	5006	1996	1992	1991	1945	1958	1976	1999	1999	2007
	Pump Setting Depth (Feet)	PMP DP	N/A	N/A	A/A	N/A	N/A	Ϋ́ A	N/A	220	150	270	220	260	580	N/A
	Duty Pt. Motor Horsepower (HP)	MTR HP	225	225	45	m	Ħ	25	25	. 54	20	24	49	09	49	42
	Duty Pt. Capacity/ Flowrate (GPM)	CAPTY GPM	3000	3000	009			850	850	450*	450	240	*005	650	200	009
	Pump Type	PIMP TYPE	Vertical Turbine	Vertical Turbine	Aurora Horizontal Split Case Model 411 (Duplex System)	Vertical Split Case	Vertical Split Case	Aurora Horizontal Split Case HSC-410	Aurora Horizontal Split Case HSC-410	Submersible	Vertical Turbine	Submersible	Vertical Turbine	Vertical Turbine	Vertical Turbine	Aurora Horizontal Split Case Model 411 (Duplex System)
	Location/Description	TOC DESC	Cook St, Treatment Facility on Cook St; Pump No. 1	Cook St. Treatment Facility on Cook St; Pump No. 2	Booster Pump Station #2 (Morgan PS); Pump No. 1	Cleio Vista Booster Pump System at the end of Camino De Cleio; Pump No. 1	Cielo Vista Booster Pump System at the end of Camino De Cielo; Pump No. 2	Pershing St. Booster Pump Station at SE corner of N Pershing St. and W Eighth Ave.	Pershing St. Booster Pump Station at SE corner of N Pershing St. and W Eighth Ave,	Well #1 at Cook St Water Treatment Facility (Southern part of Property)	Well #2 Pump House (Inside) at Cook St. about 600 ft west of Water Treatment Facility	Well #4 Pump Shed (Inside) at Approx. 250 feet SW of the Intersection of Ridge Rd and Central Ave.	Well #6 Pump Building (inside) at Approx. 380 feet NW of the Intersection of S Broadway St. and Rio Grande Ave.	Well #7 Pump Building (inside) at NE corner of the Intersection of Veater St. and Hyde Ave.	Well #8 Pump Building (inside) at NW corner of City Utility Yard Property approx 300 due west of Michigan St.	Booster Pump Station #2 (Morgan PS); Pump No. 2
	Easting		772209.7	772211.0	774978.8	774996.1	774948.4	778224.5	778207.1	772121,0	772281.6	771213.6	770884.1	770850.2	771776.5	774978.8
	Northing		2890327.4	2890361.3	2897282.9	2897491.2	2897484.7	2901216.7	2901215.3	2890346.2	2890382.7	2887196.2	2889348.7	2890775.8	2888378.2	2897282.9
	Asset ID (GIS Point No./ID)		0	1	2	m	4	v,	9	7	90	6	10	п	12	£1

Item					SIMIA		Spartan Constructors of Texas, Inc	tors of lexas, inc	Smithco Con	Smithco Construction, Inc.	File	File Construction		J29 Enterprises, LLC	rises, LLC.	Highland	Highland Enterprises, LLC	ses, LLC
No.	Description	Unit	Est. Qty.	Unit Cost	t Total Cost	ost	Unit Cost	Total Cost	Unit Cost	Total Cost	Unit Cost	Total Cost	Cost	Unit Cost	Total Cost	Unit Cost	P.	Total Cost
	Furnish full-coverage pre and post construction video documentation of the entire construction site (DVD format)	SI	н	\$ 3,600.00	v	3,600.000 \$	1,695.38 \$	\$ 1,695.38	\$ 2,500.00	\$ 2,500.00	\$ 10,303.30	٠,	10,303.30 \$	2,687.00	\$ 2,687.00	\$ 369.89	\$ 68	369.89
	Material Testing Allowance Traffic Control	ALLOW	1 1	\$ 10,000.00	\$ \$	10,000.00 \$	10,000.00 \$	10,000.00	\$ 10,000.00	\$ 10,000.00	\$ 10,000.00	w w	10,000.00 \$	10,000.00	\$ 10,000.00	\$ 10,000.00	00 \$	10,000.00
	Preparation, Implementation and Maintenance of Dona Ana County Storm Water Pollution Prevention Plan (SWPPP) (Includes BMP and all related appurtenances not otherwise Included on Bid Form)	SI	Ţ	00'000'6 \$	v	\$ 00:000'6	9,607.13 \$		000.000	\$ 5,000.00	· v	S	18,880.20 \$	5,745.00	745	· v	the factor of the same	6,719.75
	Furnish and install NMDOT-approved seeding material along maximum disturbed area width of 20-feet in accordance with NMDOT permit. (Include all labor, materials, and related appurtenances not separately listed on Bid Form),	Acres	4	\$ 7,000.00	v	28,000.00 \$	7,911.75 \$	31,647.00	\$ 6,000.00	\$ 24,000.00	\$ 8,153,10	·s	32,612.40 \$	8,900.00	\$ 35,600.00	\$ 6,164.91	\$ 16	24,659.64
	Utility Relocation (location and utility invoices must be approved by Engineer and Owner prior to any relocation)	Allow.	1	\$ 10,000.00	4	10,000.00 \$	10,000.00 \$	10,000.00	\$ 10,000.00	\$ 10,000.00	\$ 10,000.00	S	10,000.00 \$	10,000.00	\$ 10,000.00	\$ 10,000.00	\$ 00	10,000.00
-	furnish and install Ginch C300 PVC DR-18 Waterline, (Indudes all material, labor, potholing, trenching, type D bedding, removal of waste excavation, joint restraints, fittings, warming tape, tracer wire, above ground pipe marker, 2 backfulling, compaction, distriction, six erestoration, hydrostatic pressure testing and all related appurtenances not separately listed on the bid form), CIP	T.	578	\$ 20.00	00 \$ 11,560.00	\$ 00.00	29.592 \$	17,104.18	\$ 20.00	\$ 11,560.00	v	26.30 \$ 15.	15,201.40 \$	18.00	\$ 10,404.00	\$ 58.85	\$ 55	34,015.30
	Furnish and Install 10-inch C900 PVC DR-18 Waterline, [Includes all material, labor, potholing, trenching, type D bedding, ermoval of waste excavation, joint restraints, fittings, warning tape, tracer wire, above ground pine markers, backfilling, compaction, disinfection, site restoration, hydrostatic pressure testing and all related apportrenances not separately listed on the bild form), CIP	LF.	7984	\$ 30.00	00 \$ 239,520.00	\$ 0000	32.921 \$	262,841.26	\$ 25.00	\$ 199,600.00	\$ 30.10	s	240,318.40 \$	34.80	\$ 277,843.20	\$ 57.85	•	461,874.40
	Furnish and Install 12-inch C900 PVC DR-18 Waterline, [Includes all material, Jabor, potholing, trenching, type D bedding, arming tape, tracer wire, abour gound pipe makers, backfilling, comparation, idsinfection, site restoration, hydrostatic pressure testing and all related appurtenances not separately listed on the bird form), CIP	LF	7943	\$ 35.00	00 \$ 278,005.00	\$ 65.00	38.007	301,889.60	30.00	\$ 238,290.00	\$ 37.90	v	301,039.70 \$	31.00	\$ 246,233.00	\$ 65.00	v.	516,295.00
	Furnish and Install 12-Inch HOPE Waterline, (Includes all matteria). Babe, potholing, trenching, type D. 2 bedding, tremonal of waste accuration, joint restraints, fittings, warning tape, tracer wire, above ground piper markers, backlining compaction, distinfaction, site restoration, hydrostatic pressure testing and all related appurtenances not separately listed on the bid form), CIP	5	450	\$ 40.00	00 \$ 18,000.00	\$ 00.00	52.762 \$	23,742.90	0000	\$ 27,000.00	\$ 55.10	v	\$ 24,795.00 \$	45.75	\$ 20,587,50	\$ 61.62	\$ 22	27,729.00
/ 24 2 5	Furths and Install 10-inch Ductile fron Waterline, (Includes all material, labor, potholing, trenching, type D-2 bedding, termoval of whose caevastion, join trestraints, fittings, warning tape, tracer wire, above ground pipe markers, backfilling, compaction, disification, site restoration, hydrostatic pressure testing and all related appurtenances	LF	746	\$ 40.00	00 \$ 29,840.00	0.00 \$	54.175 \$	40,114.55	\$ 65.00	\$ 48,490.00	\$ 60.20	v	44,909.20 \$	53.85	\$ 40,172.10	\$ 65.92	\$ 2	49,176.32
s	Furnish and Install 6-inch Gate Valve in Cast Iron Valve Box, (Includes all labor, materials and related appurtenances not separately listed on Bid Form), CIP	EA	1	\$ 1,500.00	00 \$ 1,500.00	\$ 00.00	1,361.44 \$	1,361.44	\$ 1,500.00	\$ 1,500.00	\$ 1,175.30	v,	1,175.30 \$	1,345.00	\$ 1,345.00	\$ 1,350.12	2 \$	1,350.12
	Furnish and Install 10-inch Gate Valve in Cast Iron Valve Box, (Includes all labor, materials and related appurtenances not separately listed on Bid Form), CIP	Ā	o	\$ 2,600.00	00 \$ 23,400.00	\$ 00.0	2,493.742 \$	22,443.68	\$ 2,500.00 \$	\$ 22,500.00	\$ 2,084.70	v.	18,762.30 \$	2,464.00	\$ 22,176.00	\$ 2,615.15	\$	23,536.35



L						SMA		Spa.	rtan Constru	Spartan Constructors of Texas, Inc		Smithco Cor	Smithco Construction, Inc.	Inc.	File	File Construction	uc	129 E	J29 Enterprises, LLC.	, LLC.	Highla	Highland Enterprises, LLC	Jrises, LLC
	27	Furniss and mixtal by operance of zero asset asset asset as tastion 65-80 on Highway 28 (Includes backfill and compaction, carrier pipe, seals, spacers and all other appurtnances required for a complete working installation), CIP	5	45	ν. 	200.00	9,000.00	s,	153.303	\$ 6,89	6,898.64 \$	150.00	vs.	6,750.00 \$	\$ 315.90	•	14,215.50	\$ 143.72	\$ 27.	6,467.40	v	128.31 \$	5,773.95
	28	Furnish and install by Open-cut 24-Inch steel casing at station 73-45 on Highway 28 (Includes backfill and compaction, carrier pipe, seals, spacers and all other appurtenances required for a complete working installation), (QI	5	18	\$	200.00 \$	3,600.00	v,	158.954	\$ 2,86	2,861.17 \$	150.00	\$ 2,	2,700.00 \$	\$ 369.50	\$ 05	6,651.00	\$ 137.80	\$ 08.	2,480.40	vs.	135.77 \$	2,443.86
	29	Furnish and install by Open-cut 24-inch steel casing at station B1410 on Highway 28 (Includes backfill and compactor, carrier pipe, seals, spacers and all other appurteances required for a complete working installation), (IP	J.	17	۰ د	200.00 \$	3,400.00	vs.	170.257	\$ 2,85	2,894.37 \$	150.00	\$ 2,5	2,550.00 \$	436.30	\$ 08	7,417.10	\$ 136.15	\$ 51.	2,314.55	υ,	143.75 \$	2,443.75
	30	Furnish and install by Open-cut 24-inch steel casing at the intersection of Lara Road and Highway 28 (Includes backfill and compaction, carrier pipe, seals, spacers and all other appurteances required for a complete working installation), CIP	LF .	22	× ×	200.00 \$	4,400.00	ν.	158.955	3,49	3,497.01 \$	150.00	v	3,300.00 \$	354.80	\$ 08	7,805.60	\$ 149.00	\$ 00:	3,278.00	v.	139.83 \$	3,076.26
	31	Furnish and install by trenchless methods 14-inch steel casing for the Grinth waterline stubout at Sta. 2+10 (Includes trenching, backfill and compaction, carrier pipe, seals, spacers and all other appurtenances required for a complete working installation), CIP	Ð	63	\$	265.00 \$	16,695.00	v	466.958	\$ 29,41	29,418.35 \$	250.00	vs.	15,750.00 \$	347.10	•	21,867.30	\$ 533.30	30 \$	33,597.90	v.	355.69 \$	22,408.47
	32	Furnish and Install Three-Way Fire Hydrant Assembly, flindudes tee on mainline, all pipe from tee to hydrant, fittings, rises, gate valve, hydrant, restrained joints, drain rock, trenching, labor, backfill and site restoration), cip	EA	2	0′5 \$	5,000.00 \$	35,000.00	۰,	5,100.511	\$ 35,703.58	3.58 \$	6,500.00	\$ 45,5	45,500.00 \$	4,826.40	v	33,784.80	\$ 5,214.00	\$ 00	36,498.00	\$ 6,807.29	7.29 \$	47,651.03
*	33	Furnish and Install welded steel tank, (Includes cathodic protection and all appurtenance not Included separately on bid form for a complete installation), CIP	SI	1	\$ 370,0	370,000.00	370,000.00	·s	437,119.05	\$ 437,119.05	w	475,000.00	\$ 475,0	475,000.000 \$	450,000.00	·s	450,000.00	\$ 487,046.00	~ (8)	487,046.00	\$ 428,295.44	5.44 \$	428,295.44
	34	New well Building (Includes concrete foundation, interior walls, electrical, HVAC, pulmbing, wash sinks, shower, plumbing, valves, doors, etc.). As Shown on Plans and detailed in Technical Specifications, CIP	EA	1	\$ 120,000.00		\$ 120,000.00		124,327.60	\$ 124,327.60	vs.	210,000.00	\$ 210,0	210,000.00 \$	165,000.00	v	165,000.00	\$ 181,346.00	v	181,346.00	\$ 123,494.24	1.24 \$	123,494.24
	35	Furnish and Install G-inch Gate Valve with handwheel, (Includes all labor, materials and related appurtenances not separately listed on Bid Form), CIP	EA	1	\$ 2,0	2,000.00 \$	2,000.00	vs.	1,136.42	\$ 1,13	1,136.42 \$	1,500.00	\$ 1,5	1,500.00 \$	1,103.00	\$ 00	1,103.00	\$ 937.00	\$ 00	937.00	v	\$ 920.76 \$	850.76
	36	Furnish and install well house piping, (Includes air relief volves, gauges, pressure transducer, sample ports, hose bibs, pipe supports and related appurtenances not separately listed on Bid Form), Cip	SI	н	\$ 30,0	30,000.00 \$	30,000.00	w	51,169.50	\$ 51,169.50	\$ 9.50	85,000.00	\$ 85,0	\$ 5,000.00 \$	43,000.00	S	43,000.00	\$ 20,702.00	\$ 00	20,702.00	\$ 63,418.43	3.43 \$	63,418.43
X	37	Chlorination Station (include chlorine drum, (2) pumps, tubing, 1 full 55 gallon barrel, water supply, (2) injection ports diffuser, spill containment unit and all related appurteanness required for a complete working installation), CIP	รา	1	\$ 30,0	30,000.00 \$	30,000.00	v	23,776.35	\$ 23,776.35	6.35 \$	15,000.00	\$ 15,0	15,000.00 \$	13,500.00	v	13,500.00	\$ 26,827.00	\$ 00	26,827.00	\$ 15,523.24	3.24 \$	15,523.24
	38	Furnish and Install Eight Foot High Chain Link Fence (Includes vehicle gate, pedestrian gate, removal and disposal of existing fence, concrete footings and all related appurtenances), CIP	J.	327	\$	23.00 \$	7,521.00	v	74.597	\$ 24,393.22	3.22 \$	70.00	\$ 22,8	22,890.00 \$	74.00	•	24,198.00	\$ 79.1	\$ 05	25,996.50	ss.	\$ 57.97	25,097.25
~	39	Utility 3-phase power extension to the site (includes coordination with El Paso Electric Company and utility involves must be approved by Engineer and Owner prior commencing any electrical extension work)	Allow	н	\$ 270,0	270,000.00 \$	270,000.00	\$ 5.	270,000.00	\$ 270,000.00	·v>	270,000.00	\$ 270,0	270,000.00 \$	270,000.00	·ss	270,000.00	\$ 270,000.00	v	270,000.00	\$ 270,000.00	\$ 0000	270,000.00
	40	Furnish and Install New 4-inch Magnetic flow meter (Includes fittings, and all required appurtenances not otherwise listed in the Bid Form), CIP	EA	1	\$ 10,0	10,000.00 \$	10,000.00	٠,	3,966.15	96'8 \$	3,966.15 \$	4,000.00	\$ 4,0	4,000.00 \$	6,800.00	\$ 00	6,800.00	\$ 3,875.00	\$ 00	3,875.00	\$ 9,243.	\$ 79.8	9,243.67
	41	Furnish and Install New 6-inch Flow Control valve (Includes fittings, and all required appurtenances not otherwise listed in the Bid Form), CIP	EA	2	\$ 10,01	10,000.00	20,000.00	\$	26,057.40	\$ 52,114.80	4.80 \$	14,000.00	\$ 28,0	28,000.00 \$	14,200.00	vs	28,400.00	\$ 10,475.00	\$ 00	20,950.00	\$ 13,328.54	3.54 \$	26,657.08
	43	Mobilization of the well drilling Equipment Demobilization of the well drilling equipment	SI	1	\$ 10,01	10,000.00 \$	10,000.00	SS	2,825.63	\$ 2,82	2,825.63 \$	45,000.00	\$ 45,000.	45,000.00 \$	40,400.00	SS	40,400.00	\$ 49,765.00	\$ 00	49,765.00	\$ 44,011.19	38 \$	44,011.19

6 2615 C



4. Significant control of the contro						INIC	A	Spartan Construe	Spartan Constructors of Texas, Inc	Smithco Construction,	struction, Inc.	FIIE	File Construction	iction	129	enterpris	es, LLC.	Highlan	Highland Enterprises, LLC	rises, LL
Particular Par	44	drill cutting samples every 10 feet		40	s.		15,000.00	141.281					SULPHONE.	6,000.00	\$ 140		5,616.80		13 \$	4,965.20
Particular control c	45	Furnish and Install 18-inch diameter steel conductor casing, CIP		42	45	-		\$ 40.689	\$ 1,708.94	\$ 200.00	\$ 8,400.00		1000000	1,806.00	\$ 210	\$ 09.0	8,845.20		20 \$	7,820.40
Control cont	46	Drill 10-inch pilot borehole from bottom of surface casing to		610	s			\$ 56.513	\$ 34,472.93	\$ 70.00	\$ 42,700.00			35,990.00		5 00.1	45,140.00		45 \$	39,924.50
Participation annities a section to taking a	47	Geophysical Logging of borehole	ALLOW	1		14,000.00	3 14,000.00	\$ 14,000.00	\$ 14,000.00	\$ 14,000.00	\$ 14,000.00		-	14,000.00	5 2	-	14.000.00	\$ 14.000	\$ 00	14.000.00
Section 2019 Control 2019 Control 2 Control 2 Control 2 Control 3	48	Discrete interval sampling as specified in technical specifications	EA	2	*		15,400.00		\$ 3,955.88	1000			-	4,200.00	S		39,574.00	1000	63 \$	34,983.26
the desired between the foreign of the control of t	49	Water Quality Testing	ALLOW	1	s	6,000.00	\$ 6,000.00	\$ 6,000.00	\$ 6,000.00	\$ 6,000.00	\$ 6,000.00		\$ 00	6,000,00	\$ 6.000	\$ 00.0	6.000.00	\$ 6.000	\$ 00	6 000 0
Second and the seco	20	Plug and Abandon pilot boring with grout if completion of production well not justified	F)	059	w		4,875.00		\$ 3,952.65		\$ 3,900.00		10000000	4,550.00		38 \$	4,147.00		64 \$	3,666.00
Protection and and and and and and and and and an	51	Ream pilot borehole to 16-inch production borehole from bottom of surface casing to total deoth	- Fi	610	s.		54,900.00	158.235		80.00	48,800.00		C Chicago	98,210.00		The State of the S	53,728.80		87 \$	47,500.70
Purple and the late of the control and purple and the late of the control and the late of the late o	52	Furnish and Install 10-inch i.d. Blank Casing, CIP	F.	290	S	100.00	\$ 29,000.00	\$ 46.566	\$ 13,504.14	\$ 75.00	\$ 21,750.00		\$ 00	13,920.00	\$ 82	\$ 86.	24,064.20	\$ 73	35 \$	21.271.50
Furnish and state from the control of the control	23	Furnish and Install 10-inch i.d. dielectric metal adaptors for welding screen to casing, CIP	EA	m	45		12,000.00	1,158.507			-	1,	10000000	3,642.00	6,3	\$ 06:	19,148.70	\$ 5,642	46 \$	16,927.38
Experimentary containing the containing and conta	54	Furnish and Install 10-inch i.d. Well Screen, CIP	T.	360	45	250.00 \$	00'000'06	3 248.655	\$ 89,515.80	\$ 150.00	\$ 54,000.00	\$ 260.	\$ 00	93,600.00	S	\$ 56.	59,742.00	\$ 146	70 \$	52,812.00
Figure 1 Figure 2 Figure 3	25	Furnish and install centralizers Furnish and install filter pack (assumed to be 410 linear ft of	ш	9	s.		1,200.00	254.307			1000			1,602.00	S	\$ 05.	651.00	\$ 95		575.52
Particular and trained proportion branch of country and trained proportion branch of country and trained proportion branch of country and trained proportion branch and trained proportion proportion branch and proportion proportion proportion branch and proportion propor	20	8-16 silica sand filter pack)	S	13	s		13,000.00	957.322						12,662.00		-	13,724.75		\$ 92	12,132.38
Formation that the recent growth of the control growth of the co	22	Furnish and install pea gravel between bentonite seals of screened intervals		2	s	_	4,250.00	378.634			1,750.00			1,925.00		\$ 86.	1,914.90		\$ 55	1,692.75
Will Undergrowth with the standing part of the standing of t	28	Furnish and Install Bentonite Seal and cement grout, CIP		12	*	850.00 \$	_	1,057.914		\$ 1,000.00				12,900.00		-	14,093.40		21 \$	12.458.52
Extractivated Objects Representative 185 20 5 2000 5	59	Well Development	HRS	24	S	350.00 \$	8,400.00	\$ 226.05	\$ 5,425.20	\$ 675.00	1000			5,520.00	\$ 740	\$ 00	17,760.00		-	15,708.7.
National to the region of th	9	Test pumping of well	HRS	20	s	370.00 \$		226.05		\$ 225.00	\$ 11,250.00	\$ 235.	\$ 00	11,750.00	\$ 248	.93 \$	12,446.50	\$ 220	\$ 90	11,003.00
Furnish and testal state of the first and testal state of the firs	61	Standby at the request of Owners Representative	HRS	∞ .	\$	300.00		310.819		\$ 375.00	\$ 3,000.00	\$ 325.	\$ 00	2,600.00	\$ 383	\$ 00.	3,064.00	\$ 338.	\$ \$	2,708.4
Particular devicability control of the form of the f	63	wen usin rection and bacteriological testing Furnish and Install 1-inch sounding line (Includes 1-inch breather pipe with screen, sounding tube with lock and all	o 5	1 410	n 0			23 12 32 47	2	3,00	3,000.00	5,20	CONTRACTOR	5,200.00	3,15	534 (45)	3,190.00	2,87	23 \$	2,821.23
Furtish and interail Filteres Adaptive, Lift Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete pad around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, & 4y4 concrete, and page around well-less diagraph; If Pipe, page around and interailistic or significant, while page around page around and interailistic and and interailistic page around and interailistic and around and interailistic page	64	other appurtenances not included on bid form), CIP Furnish and install 4-inch i.d. Dron nine. CIP	4	430	v	00 85	00 000 00	150 01	6 0 00 6	00 10										
Figure F	5	de la company de		420	2	200.00	24,340.00	177.61	\$ 8,286.53	\$ 72.00	\$ 10,750.00	\$ 20.	\$ 00	8,600.00	\$ 25	.28 \$	10,870.40	\$ 22.	34 \$	9,606.20
Construct wellhead finduces concrete, sampling port with building concrete, sampling port with building port with building port with building port with building port with and total 30 HP Submersible Pump w/ Controls, Exp. 1 S 20,000.00 S 25,000.00 S 25,533.48 S 12,533.48 S 12,000.00 S 15,000.00 S 15,000.00 S 15,330.00 S 15,330.	9	Furnish and install Pitless Adapter, Lift Pipe, & 4'x4' concrete pad around well seal pursuant to NMED requirements, CIP		н		4.47.00	19,300.00	13,243.14		15,000.00	15,000.00		CONTRACTOR OF THE	13,500.00	温泉 生物	SESSION OF SES	16,439.00		\$ 65	14,531.59
Exemplation and install 30 HP Submersible Pump w/ Controls, EA 1 5 20,000.00 5 25,533.48 5 15,000.00 5 15,000.00 5 15,000.00 5 15,200.00 5 1	99	Construct wellhead (Indudes concrete, sampling port with backflow preventer, air release valve, conduit, steel plates and all other appurtenances not included on bid form), CIP	SI	H	A CONTRACTOR		14,000.00	9,042.00		12,000.00	12,000.00		The state of the s	4,800.00			1,915.00		\$ 86	2,256.98
State Proparation and cashing from the designing and cashing from the designing completion, placement of grave bedding for transf, profiled grave bedding for transf, profiled grave bedding for transfer cashing from the destruction colors and marked statement of grave bedding for transfer and marked statement of grave bedding for transfer and marked statement of grave brown graved from the definition. The markets shedring from the definition of transfer and markets shedring from the definition. The markets shedring from the definition. The markets shedring from the definition. The markets shedring completion, placement of systems of the definition. The markets shedring completion, placement of systems of the definition. The markets shedring completion, placement of systems of the definition. The markets shedring completion, placement of systems of the definition. The markets shedring completion, placement of systems of the definition. The markets shedring completion, placement of systems of the definition of the definition. The markets shedring completion, placement of systems of the definition. The markets shedring completion, placement of systems of the definition of the definit	67	Furnish and Install 30 HP Submersible Pump w/ Controls, CIP	Æ	1	100000	_	4.0			15,000.00	15,000.00			26,000.00	3 133		15,320.00		\$ 06	13,541.90
Remove and Replace Grave Roadway (Includes labor and markets) and replace Grave Roadway (Includes labor and markets) and related appuremance not separately listed on the well and tank find the market space for some find the well and tank find the market parameter) (Includes removal and disposal of existing ferre, well of the well and tank find the well and tank find the well of the well and tank find the well and tank find the well of the well and tank find the well and the well	89	Site Preparation and Grading (Includes Clearing and grubbing, grading of task location, readway to task, pording completion, placement of gavel bedding for task, excavation of task discharge pond and installation of infinian contents.	য	1			25,000.00	152,131.65	152,131.65	140,000.00	140,000.00			162,500.00			175,270.00		\$ 66	66,257.99
Furnish and Intall 12-foot vehicle gate on existing fence, which greet for existing fence, which greet can be statisfied to existing fence, which greet can be statisfied and related and displayed and related and trainal states on the bid form.), CPA statisfied and Intall statisfied a	69	Remove and Replace Gravel Roadway (Includes labor and material required to construct roadway to the well and tank site and related appurtenance not separately listed on the bid form) (IP	λS	5216	1/1		52,160.00	8.235	42,953.76		STATES OF THE PARTY OF			83,456.00		TO SEE SEE SEE SEE SEE	54,768.00		\$ 00	52,160.00
Furnish and install 8 and C900 PVC DR-18 Waterline, [Industrial Jaho, publicity, page 10 performs, page 11 performs, page 11 performs, page 11 performs, page 12 performs, page 12 performs, page 12 performs, page 13 performs, page 14 performs, page 14 performs, page 14 performs, page 15 performs, page 15 performs, page 15 performs, page 15 performs, page 16 performs, page 16 performs, page 17 performs, pag	70	Furnish and Install 12-foot vehicle gate on existing fence (Includes removal and disposal or existing fence, vehicle gate, and related appurtenances not separately listed on the bid form) CIP	EA	m	₩.		1,188.00		\$ 5,425.20				The second second	4,800.00			4,593.00		\$	5,918.31
Furnish and install 8-inch Gate Valve in Cast Iron Valve Box, (Includes all labor, materials and related appurtenances not EA 1 5 1,900.00 \$ 1,700.51 \$ 1,700.51 \$ 2,200.00 \$ 1,600.00 \$ 1,500.00 \$ 1,700.00 \$ 1,700.00 \$ 2,200.00 \$ 1,700.00 \$ 1,	7.1	Furmish and Install 8-inch C900 PVC DR-18 Waterline, Inducated an material, Jacop, pothology, rearching, type D Dedding, removal of waste excavator, joint restraints, fittings, warning tape, tracer wire, above ground pipe markers, backfilling, compaction, disinfaction, site restoration, hydrostatic pressure testing and all related appurtenances not separately listed on the bid form), CIP	4	15	v,			86.105		75,00	1,125.00		THE RESERVE OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN TW	2,655.00			1,376.25		\$ 55	2,753.25
	72	Furnish and Install 8-inch Gate Valve in Cast Iron Valve Box, (Includes all labor, materials and related appurtenances not separately listed on Bid Form), CIP	EA	1	75.00		1,900.00		\$ 1,700.51	2,200.00	2,200.00			1,600.00	100000		1,760.00		\$ 68	2,272.39

1.6 fitt	Re (In 1.5 lat En	1.4 mi	1.3 foo	1.2 Box, appu	Fu (in the contract of the con	Item No.			78 Sia	77 loo rel	76 gee	75 Gr	74 bee	73 be tee
Tie to existing yard piping (Includes all other required fittings and appurtenances for a complete working installation), CIP	Remove and Replace 6-inch thick Concrete Driveway pad (includes disposal of existing concrete to an approve site, alabor, equipment, coordination of removal locations with Engineer and Owner and all related appurtenances not separately listed on the bid form), CIP	Locate and tie to existing 6-inch Waterline (Includes tee, megallugs, caps, reducers, removal of existing valves & fittings and all other required fittings and appurtenances for a complete working installation), CIP	Remove and replace existing roadway with 3-mch HMAC, 8: inch hase course and 12-mch subgrade prep; assumed 12- foot max width (includes removal and disposal to an approved site of the existing asphalt and gravel, tack cost, asphalt and restoration of pavement markings to preconstruction configuration), CIP	Furnish and Install 10-inch Gate Valve in Cast Iron Valve Box, (Includes all labor, materials and related appurtenances not separately listed on Bid Form), CIP	Furnish and Install 10-inch C900 PVC DR-18 Waterline, (Includes all material, labor, potholing, trenching type D bedding, removal of waste excavation, joint restraints, fittings, warning tape, tracer wire, above ground pipe markers, backfilling, compaction, disnfection, site restoration, hydrostatic pressure resting and all related appurtenances not separately listed on the bid form), CIP	Description		Wr	Install 122-feet wide x 60-feet long x 8-indt thick concrete slab (4,000 psi) low water crossing. (Include all labor, materials, and related appurtenances not separately listed on Bid Form), CIP.	runch & Install 1.5-feet thick layer of 6" to 6" Fractured Rock for slope protection at the inlet and outlet side of the flow water crossings, (Include all labor, materials, and related appurtenances not separately listed on Bid Form), GP	Over excavate existing soil for Tank footings and replace with engineered fill as directed by the completed geotechnical report (incl. removing native materia), processing and compaction of engineered fill, and all related appurtenances not included on Bid Form), CIP	Geotechnical Report for New Tanks Site	Furnish & Install 12-inch Ductile Iron (DI) Pipe (Includes pipe material and fittings, megalugs, trenching, installation, bedding material, backfill, compaction, utility crossings, testing, disinfection, and all related appurtenances not listed in the Bid Form). (IP	Fundsh & Install B-inch Durtile Iron (I)) Pipe (Includes pipe material and fittings, megalugs, trenching, installation, bedding material, backfill, compaction, utility crossings, testing, disinfection, and all related appurtenances not listed in the Bid Form), CIP
E	SY	EA	SY	EA	7	Unit		Written Total:	EA EA	SY	Q	LS	FF.	F
Þ	61	ь	85	2 .	3550	Est. Qty.			2	200	15	1	108	v
\$ 2,500.00	\$ 85.00	\$ 2,500.00	\$ 43.00	\$ 2,600.00	\$ 30.00	Unit Price		\$	\$ 25,000.00	\$ 200.00	\$ 90.00	\$ 5,000.00	\$ 60.00	\$ 50.00
\$ 2,500.00	5,185.00	\$ 2,500.00	3,655.00	\$ 5,200.00	0 \$ 106,500.00	Total Price		238,967.50	0 \$ 50,000.00 \$	0 \$ 40,000.00	0 \$ 1,350.00	0 \$ 5,000.00	0 \$ 6,480.00	0 \$ 250.00
\$ 1,709.770	\$ 94.530	\$ 2,388.940	\$ 113.025	\$ 2,606.770	\$ 31.056	Unit Price	ADDITI	v.	\$ 15,350.85 \$	\$ 51.17	\$ 57.026	\$ 8,476.88) \$ 62,369	\$ 170.156
\$ 1,709.77	\$ 5,766.33	\$ 2,388.94	\$ 9,607.13	\$ 5,213.54	\$ 110,248.80	Total Price	ADDITIVE ALTERNATIVE NO. 1	2,717,251.77	\$ 30,701.70	\$ 10,234.00	\$ 855.39	\$ 8,476.88	\$ 6,735.85	\$ 850.78
\$ 3,500.00	\$ 100.00	\$ 3,500.00	\$ 65.00	\$ 3,000.00	\$ 25.00	Unit Price	1	S	\$ 25,000.00	\$ 60.00	\$ 125.00	\$ 6,000.00	5 \$ 85.00	\$ 150.00
\$ 3,500.00	\$ 6,100.00	\$ 3,500.00	\$ 5,525.00	\$ 6,000.00	\$ 88,750.00	Total Price			\$ 50,000.00	\$ 12,000.00	\$ 1,875.00	\$ 6,000.00	00.081'6 \$	\$ 750.00
\$ 1,565.90	\$ 94.70	\$ 4,207.90	\$ 66.70	\$ 2,379.20	\$ 34.50	Unit Price		\$	15,425.00	\$ 45.00	\$ 350.00	\$ 5,100.00	\$ 77.00	\$ 290.00
\$ 1,565.90	\$ 5,776.70	\$ 4,207.90	\$ 5,669.50	\$ 4,758.40	\$ 122,475.00	Total Price		2,799,427.00		\$ 9,000.00	\$ 5,250.00	\$ 5,100.00	\$ 8,316.00	\$ 1,450.00
\$ 2,325.00	\$ 103.50	\$ 4,257.00	\$ 55.65	\$ 2,463.00	\$ 23.68	Unit Price		w u	\$ 26,787.25	\$ 49.79	\$ 1,005.00	\$ 3,635.00	\$ 46.75	\$ 215.00
\$ 2,325.00	\$ 6,313.50	\$ 4,257.00	\$ 4,730.25	\$ 4,926.00	\$ 84,064.00	Total Price		2,857,094.05	\$ 53,574.50	\$ 9,958.00	\$ 15,075.00	\$ 3,635.00	\$ 5,049.00	\$ 1,075.00
\$ 986.39	\$ 140.18	\$ 986.39	\$ 51.00	\$ 2,615.16	5 29.96	Unit Price		\$ 0		0 \$ 49.94	0 \$ 9.21	0 \$ 3,754.43	0 \$ 68.76	\$ 270.52
\$ 986.39	\$ 8,550.98	\$ 986.39	\$ 4,335.00	\$ 5,230.32	\$ 106,358.00	Total Price		3,087,769.82	ts.	4 \$ 9,988.00	v	3 \$ 3,754.43	6 \$ 7,426.08	2 \$ 1,352.60

				SMA	Spartan	Spartan Constructors of Texas, Inc	exas, Inc	Smithco Construction, Inc.	ruction, Inc.	File C	File Construction	JZ	J29 Enterprises, LLC.	TIC.	Highland	Highland Enterprises, LLC	S, LLC
1.7	Remove and Replace Gravel Roadway (Includes labor and material required to construct roadway to the well and tank site and related appurtenance not separately listed on the bid form) (In	λS	4284	\$ 20.00 \$ 85,680.00	\$	8.122 \$	34,794.65 \$	10.00 \$	42,840.00	13.00	\$ 55,692.00	\$ 00	10.45 \$	44,767.80	\$ 7.31	10	31,316.04
	Total of Additive Alternative	/e No. 1:		\$ 211,220.00	\$ (\$ 51.627,691		156,215.00	10	200,145.	\$ 04	ない は は は	151,383.55	\$	15	157,763.12
	Written	en Total:		\$ 211,220.00	\$ (169,731.48		156,215.00	5	200,145.	40 \$	The second second	151.383.55	\$	15	57,763.12

Percentage and install material to rehabilitate existing well Carl Org. Unit Price Total Pri				_	SMA	A	Spartan Constructors of Texas, Inc	tors of Texas, Inc	Smithco Construction, Inc.	truction, Inc.	File Cons	File Construction	J29 Enterp	129 Enterprises, LLC.	Highland Enterprises, LLC	erprises, LLC
Unit Est City Unit Price Total Price Total Price Total Price Total Price Total Price Total Price Unit Pric							ADDITIVE	ALTERNATIVE NO. 2						100		
Furnish and install material to sand blast and repaint intension for existing went from the bulk of the standard season where strong tension of existing went from control existing went from the bulk of the standard went from t	No.		Onit	Est. Qty.	Unit Price	Total Price	Unit Price	Total Price	Unit Price	Total Price	Unit Price	Total Price	Unit Price	Total Price	Unit Price	Total Price
Total of Additive Alternative No.2 S 70,000 00 S 113,590.13 S 110,000.00 S	н	Furnish and install material to sand blast and repaint intentor of existing water storage tank (inc.) adding tank vent shroud, add security hatch locking device and related appurteances not separately listed on the bid form) (IP	S	H	\$ 70,000.00	\$ 70,000.00 \$	\$ 081.590.130 \$		\$ 110,000.00		\$ 110,046.10		\$ 121,712.00		\$ 107,592.68	\$ 107,592.68
Month of the proper control of additive Alternative No. 3. 13,500.000 5 13,600.000 5		Total of Additive Alterna	ative No. 2:		\$	70,000.00		113,590.13	\$	110,000.00	\$	110,046,10	\$	121.712.00	\$	107.592.68
Description Unit Est. Qty, Unit Price Total Price Total Price Total Price Unit Price Total Price Unit	ı	Wri	itten Total:		\$	70,000.00		113,590.13	\$	110,000.00	\$	110,046.10	\$	121,712.00	S	107,592.68
Particle Description Dutic Est Qty. Unit Price Total Price Total Price Total Price Unit Price Total Price Unit Price Unit Price Total Price Unit P		14					ADDITIVE	ALTERNATIVE NO. 3	15		70					
Furnish and install material to rehabilitate existing well (incremoval and disposal of existing buttle from the bid from class of the bid (incremoval and disposal of existing buttle from the bid (incremoval and disposal	8		Unit	Est. Qty.	Unit Price	Total Price	Unit Price	Total Price	Unit Price	Total Price	Unit Price	Total Price	Unit Price	Total Price	Unit Price	Total Price
33. \$ 25,000.00 \$ 18,649.13 \$ 110,000.00 \$ 8,000.00 \$ 18,649.13 \$ 110,000.00 \$ 8,000.00 \$ \$		Furnish and Install material to rehabilitate existing well (incl. removal and disposal of existing Ductile Iron pipe and related appurtenances not separately listed on the bid form) CIP	S	н	\$ 25,000.00 \$	\$ 25,000.00 \$	18,649.130 \$		\$ 110,000.00	\$ 110,000.00			\$ 51,632.00		\$ 46,155.32	\$ 46,155.32
\$ 25,000.00 \$ 18,649.13 \$ 110,000,00 \$ 8,000.00 \$		Total of Additive Alterna	ative No. 3:		\$	25,000.00 \$		18,649.13	\$	110,000.00	\$	8,000.00	S	51,632.00	\$	46,155.32
		Wri	itten Total:		\$	25,000.00 \$		18,649.13	\$	110,000.00	\$	8,000.00	\$	51.632.00	\$	46.155.32
		Bid Total (Base Bid + Alternative 1 + Alternative 2 + Alternative	ernative 3):		\$	2,764,921.00 \$	\$	3,020,401.32	\$	3,125,675.00	\$	3,118,925.30	\$	3,181,821.60	\$	3,399,280,94

*Cells with red text indicate that a correction has been made in accordance with Article 14.01.C of the Instructions to Bidders (C-200)

CERTIFICATION:
I certify that the above figures are the evaluated bid prices from those submitted in the Bid Form.

Marty Howell, P.E. Souder, Miller & Associates

August 20, 2019 Date

	SINIA	Spartan Constructors of Texas, Inc		Smithco Construction, Inc.	tion, Inc.	File Cor	File Construction	J29 Enterprises,	rprises, LLC.	Highland	Enterprises,	S, LLL
EA 8 \$ 3,200.00	\$ 25,600.00 \$	2,750.104 \$	22,000.83 \$	3,000.00 \$	24,000.00 \$	2,783.70	\$ 22,269.60	\$ 2,900.00	\$ 23,200.00	0 \$ 2,708.86	•	21,670.88
EA 14 \$ 4,800.00	\$ 67,200.00 \$	4,310,363 \$	60,345.08 \$	5,500.00 \$	77,000.00 \$	4,082.90	\$ 57,160.60	\$ 4,446.00	\$ 62,244.00	0 \$ 4,046.65	v,	56,653.10
EA 1 \$ 2,500.00	\$ 2,500.00 \$	1,824.85 \$	1,824.85 \$	3,500.00 \$	3,500.00 \$	1,953.70	\$ 1,953.70	\$ 1,910.00	\$ 1,910.00	0 \$ 1,048.03	vs.	1,048.03
LF 60 \$ 10.00	\$ 600.00	15.402 \$	924.12 \$	15.00 \$	900.00	21.10	\$ 1,266.00	\$ 19.81	\$ 1,188.60	0 \$ 20.24	•	1,214.40
EA 4 \$ 1,000.00	\$ 4,000.00	625.748 \$	2,502.99 \$	1,250.00 \$	5,000.000 \$	1,457.00	\$ 5,828.00	\$ 871.25	\$ 3,485.00	\$ 659.65	v.	2,638.60
LF 20 \$ 15.00	\$ 300.00	20.551 \$	411.02 \$	18.00 \$	360.00 \$	51.40	\$ 1,028.00	\$ 21.90	\$ 438.00	0 \$ 40.69	\$	813.80
LF 160 \$ 320.00	\$ 51,200.00 \$	497.474 \$	79,595.84 \$	220.00 \$	35,200.00 \$	295.80	\$ 47,328.00	\$ 458.06	\$ 73,289.60	0 \$ 369.32	·s	59,091.20
LF 20 \$ 200.00	\$ 4,000.00 \$	85.489 \$	1,709.78 \$	150.00 \$	3,000.000 \$	380.10	\$ 7,602.00	\$ 152.02	\$ 3,040.40	0 \$ 135.87	v.	2,717.40
LF 22 \$ 200.00	\$ 4,400.00 \$	96.79 \$	2,129.38 \$	150.00 \$	3,300.00 \$	411.00	\$ 9,042.00	\$ 151.40	\$ 3,330.80	0 \$ 139.10	vs.	3,060.20
LF 65 \$ 350.00	\$ 22,750.00 \$	444.353 \$	28,882.95 \$	250.00 \$	16,250.00 \$	341.70	\$ 22,210.50	\$ \$57.12	\$ 36,212.80	5 572.67	•	37,223.55
LF 20 \$ 350.00	\$ 7,000.00 \$	452.265 \$	9,045.30 \$	450.00 \$	\$ 00.000,6	557.40	\$ 11,148.00	\$ 436.77	\$ 8,735.40	5 483.33	vs.	09'999'6
LF 20 \$ 320.00	\$ 6,400.00 \$	452.265 \$	9,045.30 \$	450.00 \$	\$ 00.000.00 \$	557.40	\$ 11,148.00	\$ 924.58	\$ 18,491.60	\$ 483.33	s,	09'999'6
LF 20 \$ 320.00	\$ 6,400.00 \$	153.304 \$	3,066.08 \$	450.00 \$	9,000.00 \$	384.20	\$ 7,684.00	\$ 156.75	\$ 3,135.00	5 139.88	vs.	2,797.60

											·					T-
	Notes/Comments		According to Operator, Turner Electric added additional windings to pump motor to assist with VFD operation. Info sources Site Visit, Oper: Interview, submittal data, and O&M Manual	Pump No. 2 of Cook St Pump Station. Pump may need additional windings to assist with VFD operation. Info sources Site Visit, Oper. Interview, submittal data, and OSAM Manual	Part of Duplex Booster SNG-Mounted System (Model # TDH-1200-90/System ID # 07-0203). Motor is original and in good condition. Info sources: SIte Visit, Oper: Interview, and 0.8M Manual	Motor Label indicates: motor is 3 Hp, Manfact is WEG in Brazil; Manufact Date is Got 33, 2006. Motor in good condition, info Source: Oper, Interview and Site Visit	Motor in good condition, Info Source: Oper, Interview and Site Visit. Need Info from pump label or O&M Manual.	REPURPOSED - pumps (and motors) were removed from station in approx. 2012 and put into storage at the WWTP site.	REPURPOSED - pumps (and motors) were removed from station in approx. 2012 and put into storage at the WWTP site.	Motor replace in 2001 and in good condition. Info Sources: Operator Interview and Site Visit	Motor replaced in approx, 1985 and is in fair to good condition. Info Source, 2007 MED Sankary Survey, Operator nterview and Site Visit.	Motor replaced in approx. 2001 and is in good condition. Info Source: 2007 NMED Sanitary Survey, Operator Interview and Site Visit.	Motor replaced in 1999 and appears to be in very good condition. Info Sources: 2007 NMED Sanlary Survey, LH 1998-99 As-builts, Operator interview and Site Visit.	Malor appears to be in good condition. Info Sources, 2007 NMED Senitary Survey, LH 1998-99 As- bullts, Operator Interview and Site Visit .	Motor appears to be in very good condition. Info Sources: 2007 NMED Sanitary Survey, tH 1998-99 As- builts, Operator Interview and Site Visit.	Part of Duplex Booster Skid-Mounted System (Model # TDH-1200-90/System ID # 07-0203). Motor is original and in good condition. Info sources: Site Visit, Oper. Interview, and O&M Manual
	Estimated Asset Value (Replacement Value)	ASSET_VALUE	\$15,000	\$15,000	\$5,500	\$500	\$500			\$3,000	\$3,000	\$3,000	\$5,500	\$7,000	\$5,500	\$5,500
	Project No./ System No./ Job No., Etc.	PROJECT_NO	Factory Order #8G6- 10241/ P.O.# 204513	Factory Order #8G6- 10241/ P.O. # 204513	Order # 485692 / System ID # 07-0203			Job# 96-065; PO# 204514 (JC&H)			-					Order # 485692 / System ID # 07-0203
	Replacement/ Rehab Cost	RPC_RHB_CT	000'5\$													
	Year Replaced/ Rehabbed	YR_RPC_RHB	2014							2001	1985	2001	1999			
	Initial Cost	INIT_COST														
ors	Year Installed	YR_INSTL	1996	1996	2007	2006	2006	1996	1992	1991	1945	1958	1976	1999	1999	2007
T or C Water System Inventory: Pump Systems - Motors	Model No. / Serial No. / ID No.	MOD_NO	H08289	H08289	Model# R366	Mod No. 1UTO1CGNXJ003020							ID No. D10-S329A-M C2			Model# R366 Cat # D60P1BS-C
em Inventory: Pur	Manufacturer	MANUFACT	US Electric Motors	US Electric Motors	Motor: Emerson Motor Comp.; System: Canarlis Corp.	WEG		US Electric Motors	US Electric Motors		US Electric Motors		US Electric Motors	US Electric Motors	US Electric Motors	Motor: Emerson Motor Comp.; System: Canarils Corp.
Water Syste	Motor Current (Amps)	MTR_AMPS	284	284	8	8.42		8					69			89
TorC	Motor Phase	MTR_PHASE	m	m	m	м		m			m		m	m	m	er .
	Motor Freq. (Hz)	MTR_FREQ	09	. 8	09	99		99			8		09	09	09	09
	Motor Voltage (V)	MTR_VOLT	460	460	460	208		230/460			220/440		230/460	230/460	460	460
	Motor	MTR_RPM	1780	1780	3500	3450		3600					1750	1800	1775	3500
	Motor Horse - power (HP)	MTR_HP	250	250	6	m	ਦ	75	27.	30	52	30	09	2 7	8	. 09
	Pump Type	PMP_TYPE	Vertical Turbine	Vertical Turbine	Aurora Horizontal Spilt Case Model 411 {Duplex System}	Vertical Split Case	Vertical Split Case	Horizontal Split Case HSC-410	Horizontal Split Case HSC-410	Submersible	Vertical Turbine	Submersible	Vertical Turbine	Vertical Turbine	Vertical Turbine	Aurora Horizontal Split Case Model 411 (Duplex System)
	Location/Description	LOC_DESC	Cook St. Treatment Facility on Cook St: Pump No. 1 Motor	Cook St. Treatment Facility on Cook St; Pump No. 2 Motor	Booster Pump Station #2 (Morgan PS); Pump No. 1 Motor	Cielo Vista Booster Pump System at the end of Camino De Cielo; Pump No. 1 Motor	Cielo Vista Booster Pump System at the end of Camino De Cielo; Pump No. 2 Motor	Old Pershing St. Booster Pump Station Pump No. 1 Motor	Old Pershing St. Booster Pump Station Pump No. 2 Motor	Well #1. at Cook St Water Treatment Facility (Southern part of Property)	Well #2 Pump House (inside) at Cook St. about 600 ft west of Water Treatment Facility	Well #4 Pump Shed (inside) at Approx. 250 feet SW of the Intersection of Ridge Rd and Central Ave.	Well #6 Pump Building (Inside) at Approx. 380 feet NW of the Intersection of S Broadway St. and Rio Grande Ave.	Well #7 Pump Building (inside) at NE corner of the Intersection of Veater St. and Hyde Ave.	Well #8 Pump Building (inside) at NW corner of City Utility Yard Property approx 300 due west of Michigan St.	Booster Pump Station #2 (Morgan PS); Pump No. 2 Motor
	Easting		772209.7	772211,0	774978.8	774996.1	774948,4	778224.5	778207.1	772121.0	772281.6	771213.6	770884.1	770850.2	771776.5	774978.8
	Northing		2890327.4	2890361.3	2897282.9	2897491.2	2897484.7	2901216.7	2901215.3	2890346,2	2890382.7	2887196.2	2889348.7	2890775.8	2888378.2	2897282.9
	Asset ID (GIS Point No./ID)		0	Ħ	2	3	4	S.	·	7	60	6	10	Ħ	12	13

APPENDIX B

T or C Water System Inventory: Pump Systems - Manifolds and Flow Meters

Notes/Comments	NOTES_COM/M	Manfiold piping is 12° for large pumps, 8° for small pumps (not yet installed) and 0° for auxiliany piping. Associated Components: 4 check valves (2x.12°, 2x.8°), 10 isolation (legis) valves (3x.12°, 3x.8°, 4x.6°), 4x.4° (3x/al control valves, 8.48Vs, many press, gauges. All components in good condition except one 12° CV that has a partial rate of the properties of the properties of the foreign of the properties of the producing and the properties of manifold need to be replaced/repaired - Priority in AMP Report. Pumps observed producing 100 PSI in pipe manifold. Info sources: Q&M Manual, Oper: Interview, and Site Visit.	One flow meter on discharge pipe coming from chlorination system to 0.200 MG stongs tank fits going in to the side of storage tank is 1.8°, but appears to choke down to 12° pipe section for the flow meter, inside the FM access walt. Inside the FM access value. The flow of the FM access walt. Inside the FM access walt. Inside the FM access walt. Inside the FM access of the form of the flow of th	Manifold for Duplex Booster Skid-Mounted System (Model # TDH-1200- 19) includes one TG-US-VIS System (Anticipator (angle, flanged) and four 8° lociston waves (externity). Info sources: O&M Manual, Oper, Interview, and Site Visit.	Badger Magnetoflo FM for Duplex Booster Skild-Mounted System on separate discharge pipe inside FM vaults outside of building. Info sources: O&M Manual, Oper, Interview, and Ste Visit.	Associated Components: Two Clayals. No Flow Meter. Manifold in good condition. Info sources: Oper, interview and Site Visit.	REPURPOSED - Piping Manifold remains in service with new PRV as a Pressure Reducing Station that separates the high and low pressure associated Components: two 8" isolation (Gata) relives and PRV. Overall the manifold priping appeared in fair-to-good condition. "FRV Installed new in 2012. "*Gate valves appear to be from 2000. Info sources: Oper. Interview and Site Visit.	Associated Components: Isolation valve, ARV, sampling tap. All components in good condition, except a portion of pipe that is old and corroded. Interview, Site Visit, and 2007 MMED Sanitary Survey.	Flow observed at 500 GPM using brand new flow meter (McCrometer Propeller FM - 2014). FM in excellent condition. Info Sources: Operator interview and Site Visit	Associated Components: 6" check valve, two 6" isolation gate valves, ARV, sampling tap. Check valve, appears to be original. One gate valve iooks to be 30 years old; "One gate valve replaced in 2004. Manifold infinite components in fair to good condition. "Prortions of manifold plping replaced with schedule 80 plps. Into Source: Operator Interview, Site Visit, 2007 NMED Sanitary Survey,
Estimated Asset Value (Replacement Value)	ASSET_VALUE	\$120,000	\$20,000	\$30,000	\$10,000	\$2,500	\$20,000	\$10,000	\$3,000	\$15,000
Project No./ System No./ Job No., Etc.	ON_80L	Factory Order #866-10241 / P.O. # 204513		Canariis Corp. System ID # 07-0203	Canarlis Corp. System ID # 07-0203	EFi Project #85092				
Manufacturer	MFG	Check Valwes; APCC); Control Valwes: GdVal Co.; ARVs; APCO: Gate Valwes; American Flow Control	Water Specialities (McCrometer)	Butterfly Valves: NiBCO; Surge Anticipator: ClaVal Co.; System: Canariis Corp.	FM: Badger; System: Canariis Corp.	EFI (Engineered Fluid)	PRV: claval Co.		McCrometer (Water Specialties)	ARV. APCO?
Replacement/ Rehab Cost	RPC_RHB_CT	\$2000\$							\$2,900	
Year Replaced/ Rehabbed	YR_RPC_RHB	2002*					2012*;		2014	2004*;
Initial	NIT_COS		-					-		
Year Installed	YR_INSTL INIT_COS	1996	1996	2007	2007	5006	1945	1991	1991	1945
Flow Meter Size (Inches)	FM_SIZE		12		60				9	
Flow Meter Type	FM_TYPE		Propeller		Magnetic				Propeller	
Material	MATERIAL	Ductile Iron	Ductile Iron; Sch 80 PVC	Ductile Iron		Ductlle Iron	Ductile fron	Ductile fron	Sch 80 PVC	Ductile Iron; Sch 80 PVC
Manifold Pipe Size (Dia) (Inches)	PIPE_SIZE	12;8,6		00		4	. 00	9		w
Equipment Description	EQUIP_DESC	Cook St. Treatment Facility bump System Manfold	Cook St. Treatment Facility Pump System Flow Meter	Booster Pump Station #2 Pump System Manifold	Booster Pump Station #2. Pump System Flow Meter	Cielo Vista Above Ground Booster Pump System Manifold	Pershing St. PRV Station Manifold	Well No. 1 Manifold	Well No. 1 Flow Meter	Well No. 2 Manifold
Location Description	LOC_DESC	Cook St. Treatment Facility on Cook St	Cook St. Treatment Fadility on Cook St.	Booster Pump Station #2 (Morgan PS);	Booster Pump Station #2 (Morgan PS)	Cielo Vista Above Ground Booster Pump System at the end of Camino De Cielo	Pershing St. PRV Station at SE corner of N Pershing St. and W Eighth Ave.	Well No. 1 at Cook St Water Treatment Facility (Southern part of Property)	Well No. 1 at Cook St Water Treatment Facility (Southern part of Property)	Well #2 Pump House (Inside) at Cook St. about 600 ft west of Water Treatment Facility
Easting		77.2289	772287.3	774923.7	774769.5	774867	769497.1	774839.9	772121	772109.4
Northing		2890363.7	2890375.8	2897124.8	2897422,4	2897125.8	2891899	2897127.2	2890346,2	2890343,4
Asset ID (GIS Point No./ID)		0	et et	2	m	4	. u	٠	7	œ

APPENDIX B

T or C Water System Inventory: Pump Systems - Manifolds and Flow Meters

	Τ		ifold Site	si b	Wes,	uo	", 6", iifold good		es 1 1 ator	
Notes/Comments	NOTES_COMM	Flow meter located on north side of building. *FM Estimated to be approx 15 years old and is in good condition. Info Sources: Operator Interview and Site Visit	Associated Components: 6" check valve, two 6" isolation (Gate) valves, sampling tap, Manifold components in fair to good condition. Manifold concrete and metal pipe supports in good condition. "Gate Valves were installed in 1992." Info Source: 2007 NMED Senitary Survey, Operator Interview, and Site Viles.	*Flow Meter is in fair condition - looks to be about 10 years old and is weathered. Info Sources: Operator interview and Site Visit	Associated Components. 8" check valve, two 8" isolation (Gate) valves, pressure gauges, 1" ARV, sampling tap, Mardiold components are all original and in good condition. Flow observed at 850 GPM at 30 PSI (pressure gauge may be wrong).	FM in good condition and appears to be from the original installation because the pipe component is Di. Info Sources; 2001 LH As-builts, Operator interview, and Site Visit.	Assoc Components: 8° check valve, three isolation (Gaze) valves (3°, 6°, and 8°), pressure gauges, two ARVS (1° and 3°), sampling tap, Manfold components inside building and concrete and metal suppors all in good condition. *The 6° gare valve was replaced in 2010. *The 6° gare valve Was replaced in 2010. Info Sources: 2001 LH As builts, Operator interview, and Site Visit.	FM in good condition and appears to be original. Info Sources: 2001 LH As-builts, Operator Interview, and Site Visit.	Associated Components. 8" theck valve, three isolation (Gate) valves (13', 6', and 8'), pressure gauges, two ARVs (1" and 31'), sampling tamela jible supportents all in good condition. Manifold concrete and meta jible supports in good condition. Manifold concrete and info Sources: 2007 NMBD Sanitary Survey, 2001 LH As-bulls, Operator interview and Site Visit.	FM in good condition and appears to be original.
Estimated Asset Value (Replacement Value)	ASSET_VALUE	000(8\$	\$15,000	000'e\$	\$22,500	\$5,000	\$22,500	\$5,000	\$22,500	15 00 00
Project No./ System No./ Job No., Etc.	ON_BOL									
Manufacturer	MFG	McCrometer (Water Specialties)?	Gate Valves: American Flow Control	McCrometer (Water Specialties)	Gate Valves: Mueller; Check Valve: Flomatic; ARV: APCO	McCrometer (Water Specialties)	Gate Valves: Mueller; Check Valve: APCO; ARVS: APCO	McCrometer (Water Specialties)	Gate Valves: Mueller; Check Valve: APCO; ARVS: APCO	McCrometer
Replacement/ Rehab Cost	RPC_RHB_CT									
Year Replaced/ Rehabbed	YR_RPC_RHB	2000*	1992*	2004*			2010*			
Initial	INIT_COS									
Year Installed	YR_INSTL	1945	1958	1958	1999	1989	1999	1999	1999	1999
Flow Meter Size (Inches)	FM_SIZE	9		9		œ		80		00
Flow Meter Type	FM_TYPE	Propeller				Propeller		Propeller		Propeller
Material	MATERIAL	Ductile Iron	Ductile Iron	Sch 80 PVC	Ductile Iron	Ductile Iron; Sch 80 PVC	Ductile Iron	Sch 80 PVC	Ductile Iron	Sch 80 PVC
Manifold Pipe Size (Dia) (Inches)	PIPE_SIZE N	J	u u			3 **	œ	•	∞	
Equipment Description	EQUIP_DESC	Well No. 2 Flow Meter	Well No. 4 Manifold	Well No. 4 Flow Meter	Well No. 6 Manifold	Well No. 6 Flow Meter	Well No. 7 Manifold	Well No. 7 Flow Meter	Well No. 8 Manifold	Well No. 8
Location Description	LOC_DESC	Well #2 Pump House (inside) at Cook 8t. about 600 ft west of Water Treatment Facility	Well #4 Pump Shed (Inside) at Approx. 250 feet SW of the Intersection of Ridge Rd and Central Ave.	Well #4 Pump Shed (inside) at Approx. 250 feet SW of the Intersection of Ridge Rd and Central Ave.	Well #6 Pump Building (Inside) at Approx. 380 771775.7 feet NW of the Intersection of S Broadway St. and Bio Grande Ave.	Well #6 Pump Building (inside) at Approx, 380 774900.5 feet NW of the Intersection of S Broadway St. and Rio Grande Ave.	Well #7 Pump Building (Inside) at NE comer of the Intersection of Veater St. and Hyde Ave.	Well #7 Pump Building (Inside) at NE corner of the Intersection of Veater St. and Hyde Ave.	Well #8 Pump Building (Inside) at tWW corner of City Utility Yard Property approx 300 due west of Michigan St.	Well #8 Pump Building (inside) at NW corner of City Utility Yard Property approx 300 due
Easting		771309.4	771116.1	770842.9	771775.7		771309.3	_	770838.3	77177.2
Northing		2887377.1	2889366.5	2890720.6	2888358.2	2897124.8	2887383	2889366.6	2890720.5	2888358.2
Asset ID (GIS Point No./ID)	Ī	ø	10	п	12	13	14	£1	16	17

	nd Control Systems
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Notes/Comments		Elec. equipment appears to be in good condition overall. New control panels for the pumps, including new soft-starters, were installed in spring 2014. Approx. 40' of electrical wires, Incl ground wire, were replaced for Pump No. 1 at cost of \$2500 in fall 2014. Info sources: Oper. Interview and Site Visit.	All elec equipment is original and in good condition. *Transfer switch was replace in 2013. Info Sources: Canariis O&M Manual, Operator interview, and Site Visit.	All elec equipment is original and in good condition. Info Sources: Operator Interview and Site Visit.	REPURPOSED - Piping Manifold and PRV remain in service. All Electrical components (not in use) to be scrapped.	All elec equipment is in good to fair condition- doesn't need replacement in the next 5 years but should be housed in weather proof structure. Info Sources: Operator nterview and Site Visit.	All elec equipment is from approx. 1975 and in poor condition - Should be replaced in the next 3 years Info Sources: Operator nterview and Site Visit.	All elec equipment is from approx. 1985 and in fair condition - Should be replaced in the next 10 years. Info Sources: Operator nterview and Site Visit.	All elec equipment is original and in good condition. Info Sources: 2001 LH As-builts, Operator Interview and Site Visit.	All elec equipment is original and in good condition. Info Sources: 2001 LH As-builts, Operator Interview and Site Visit.	All elec equipment is original and in good condition. Info Sources: 2001 LH As-bullts, Operator interview and Site Visit.	Generator is original and in good condition. Manufactured by Onan, Model/Type: QuiteSite II.
Estimated Asset Value (Replacement Value)	ASSET_VALUE	\$120,600	000'55\$	\$5,000	A .	\$30,000	\$35,000	\$35,000	\$50,000	\$50,000	\$50,000	\$100,000
Project No./ System No./ Job No., etc.	ON_80t	Factory Order #8G6- 10241 / P.O. # 204513	Canarlis Corp. System ID # 07-0203	EFI Project #85092							:	
Manufacturer	MANUFACT		System: Canariis Corporation;	System: EFI (Engineered Fluid)								Onan
Replacement/ Rehab Cost	RPC_RHB_CT		3500*									
Year Replaced/ Rehabbed	YR_RPC_RHB	2014	2013								· · · · · · · · · · · · · · · · · · ·	
Initial Cost	INIT_COST											
Year Installed	YR_INSTL	1996	2007	2006	1996	1991.	1975	1985	1999	1999	1999	2007
Equipment Description	EQUIP_DESC	Cook St. Treatment Facility Pump System	Booster Pump Station #2 Pump System	Cielo Vista Above Ground Booster Pump System	Pershing St. PRV Station	Well No. 1	Well No. 2	Well No. 4	Well No. 6	Well No. 7	Well No. 8	Booster Pump Station #2 Back-up Generator
Location Description	LOC_DESC	Cook St. Treatment Facility on Cook St	Booster Pump Station #2 (Morgan PS)	Cielo Vista Above Ground Booster Pump System at the end of Camino De Cielo	Pershing St. PRV Station at SE corner of N Pershing St. and W Eighth Ave.	Well No. 1 at Cook St Water Treatment Facility (Southern part of Property)	Well #2 Pump House (inside) at Cook St. about 600 ft west of Water Treatment Facility	Well #4 Pump Shed (Inside) at Approx. 250 feet SW of the Intersection of Ridge Rd and Central Ave.	Well #6 Pump Building (inside) at Approx. 380 feet NW of the Intersection of S Broadway St. and Rio Grande Ave.	Well #7 Pump Building (inside) at NE corner of the Intersection of Veater St. and Hyde Ave.	Well #8 Pump Building (inside) at NW corner of City Utility Yard Property approx 300 due west of Michigan St.	Booster Pump Station #2 (Morgan PS)
Easting		772209.7	774978.8	774948.4	778224.5	772121.0	772281.6	771213.6	770884.1	770850.2	771776.5	774978.8
Northing		2890327.4	2897282.9	2897484.7	2901216.7	2890346.2	2890382.7	2887196.2	2889348.7	2890775.8	2888378,2	2897282.9
Asset ID (GIS Point No./ID)	₽	0	τ.	2	m	4	25	9	7	∞	6	10

APPENDIX B

T or C Water System Inventory: Pump Systems- SCADA System

					-	C water syst	em Inventor)	I of C water System Inventory: Pump Systems- SCADA System	ADA SYSTEM		
Asset ID [GIS Point No./ID]	Northing	Easting	Location Description	Year Installed	Initial V Cost	Year Replaced/ Replacement Rehabbed / Rehab Cost	Replacement / Rehab Cost	Manufacturer	Serial No./Shop Order No.	Estimated Asset Value (Replacement Value)	Description/Comments
윤			LOC_DESC	YR_INSTL	YR_INSTL INIT_COST	YR_RPC_RHB	RPC_RHB_CT	MANUFACT	SERIAL_NO	ASSET_VALUE	NOTES_COM
0	2887656.6	785234.3	WWTP Facility at the south end of Radium St.	1997		2001*	\$17,000*	US Filter/ Consolidated Electric	Shop Order # 62879 (James, Cooke & Hobson)	\$35,000	Originally Installed in Sept 1997 - *Software for entire system upgraded in 2001. The Water System SCADA is intergrated with WWTP System SCADA. The Master Control CTU (Central Telemetry Unit) is located at the WWTP Office.
H	2888135.8	785359.3	WWTP Facility at the south end of Radium St.	1997		2001*		US Filter/ Consolldated Electric	Serial # 62879-A	NA	Existing Master Control/Transceiver Updates- Softeware for Master Controller and communication RTU.
2	2888760.8	785338.5	WWTP Facility at the south end of Radium St.	1997		2001*		US Filter/ Consolidated Electric	Serial # 62879-B	NA	Existing Desktop Computer Software Updates- Software updates for graphics display, security, alarm/event logging, historical data trending, tank level control, pump input output control, diagnostics etc.
m	2889365.0	785401.0	Sierra County Central Dispatch Station at 1502 N. Date St.	1997		2001*		US Filter/ Consolidated Electric	Serial # 62879-C	NA	Existing Sierra County Dispatch Alarm Monitoring Transceiver Updates- Software for alarm monitoring for water sytem locations.
4	2889885.8	785338,5	Storage Tank located at Broad St. and Corona St., next to Booster Pump Station No. 2	1997		2001*		US Filter/ Consolidated Electric	Serial # 62879-D	NA	Existing Lower Zone Repeater Updates- Softeware for communication RTU; control panel and probes; NEMA 12 enclosure.
īΛ	2890573.3	785317.7	Storage Tank located at Broad St. and Corona St., next to Booster Pump Station No. 2	1997				US Filter/ Consolidated Electric	Serial # 62879-E	\$13,000	Lower Zone Tank Level Transducer- Model 221 GCD Pressure/level Transducer in figerglass enclosure.
. 9	2887615.0	784546.8	Storage Tanks at 2900 Cemetery Rd (North Date St), west of I-25	1997				US Filter/ Consolidated Electric	Serial # 62879-F	\$2,000	Upper Zone Tank Transceiver RTU 12- Model SBC2 Radio Telemetry Transceiver in NEMA 3R enclosure.
7	2888240.0	784630.2	Storage Tanks at 2900 Cemetery Rd (North Date St), west of I-25	1997				US Filter/ Consolidated Electric	Serial # 62879-G	\$2,000	Upper Zone Tank Level Transducer_ Model 221 GCD Pressure/level Transducer in figerglass enclosure.
· ∞	2888719,1	784630.2	Well No. 1 at Cook St Water Treatment Facility (Southern part of Property)	1997				US Filter/ Consolidated Electric	Serial # 62879-H1	\$13,000	Well # 1 RTU 13- Model SBC2 Radio Telemetry Transceiver in a NEMA 3R enclosure.
ø.	2889427.5	784609.3	Well #2 Pump House at Cook St. about 600 ft west of Water Treatment Facility	1997				US Filter/ Consolidated Electric	Serial # 62879-H2	\$13,000	Well # 2 RTU 14- Model SBC2 Radio Telemetry Transceiver in a NEMA 3R endosure.
10	2889969.1	784588.5	Well #4 Building at approx. 250 southwest of Intersection of Ridge Rd and Central Ave.	1997				US Filter/ Consolidated Electric	Serial # 62879-H3	\$13,000	Old Well # 3 RTU 15 - Repurpused as New Well #4 RTU in 1999 - Model SBC2 Radio Telemetry Transceiver in a NEMA 3R enclosure.
11	2890719.1	784630.2	Well #7 Building at NE corner of the Intersection of Veater St. and Hyde Ave.	1997				US Filter/ Consolidated Electric	Serial # 62879-H4	\$13,000	Old Well # 4 RTU 16 - Repurpused as New Well #7 RTU in 1999. Model SBC2 Radio Telemetry Transceiver in a NEMA 3R enclosure.
12	2887635.8	783880.2	Well #8 Building at Approx. 380 feet NW of the Intersection of S Broadway St. and Rio Grande Ave.	1997				US Filter/ Consolidated Electric	Serial # 62879-H5	\$13,000	Old Well # S RTU 17 - Repurpused as New Well #8 RTU in 1999. Model SBC2 Radio Telemetry Transceiver in a NEMA 3R enclosure.
13	2888198.3	783901.0	Well #6 Building at Approx. 380 feet NW of the Intersection of S Broadway St. and Rio Grande Ave.	1997				US Filter/ Consolidated Electric	Serial # 62879-H6	\$13,000	Old Well # 6 RTU 18 - Repurpused as New Well #6 RTU in 1999. Model SBC2 Radio Telemetry Transceiver in a NEMA 3R enclosure.

T or C Water System Inventory: Plump Systems- SCADA System

					°	r C Water Sys	tem Inventor	I or C Water System Inventory: Pump Systems- SCADA System	ADA System			г
Asset ID	Asset ID Northing Easting	Easting	Location	Year		rear Replaced/	Year Replaced/ Replacement	Manufacturer	Serial No./Shop	Estimated	Description/Comments	
(GIS Point			Description	Installed	Cost	Rehabbed	`		Order No.	Asset Value		
No./ID)							Rehab Cost			(Replacement Value)		
FID			LOC_DESC	YR_INSTL	INIT_COST	YR_INSTL INIT_COST YR_RPC_RHB RPC_RHB_CT	RPC_RHB_CT	MANUFACT	SERIAL_NO	ASSET_VALUE	NOTES_COM	
14	2888698.3	783942.7	Cook St. Treatment Facility Building on Cook St	1997				US Filter/ Consolidated Electric	Serial # 62879-I	\$13,000	Cook St. Booster Pump Station/Reservoir Transceiver RTU 19- Model SBC2 Radio Telemetry Transceiver in a NEMA 3R enclosure.	
15	2889260.8	783963.5	Cook St. Treatment Facility Building on Cook St	1997				US Filter/ Consolidated Electric	Serial # 62879-J	\$2,000	Cook St Reservoir Level Transducer Model 221 GCD Pressure/level Transducer in figerglass enclosure.	
16	2889969.1	783963.5	Pershing St. PRV Station Building at SE corner of N Pershing St. and W Eighth Ave.	1997				US Filter/ Consolidated Electric	Serial # 62879-K	NA	Pershing St. Booster Pump Station/Reservoir Transceiver RTU 20- Model SBC2 Radio Telemetry Transceiver in a NEMA 3R enclosure. No Ionger in service and still attached to Pershing PRV Building.	
Source for		Alia ali venitare	monday of adams to make bearings of the Manager of the make and the ma	4004								

Source for all SCADA information: US Filter O&M Manual provided James, Cooke, & Hobson.

separates the "High" and "Low" pressure zones of the distrib, system. Building in fair condition: walls, windows, floor okay; Roof must be replaced - Priority tem. acture is a small shed used to house only pump controls. Well is located to the NE of the shed. Shed is part of old wind mill struct, orig instit of in 1939 with well. Shed is in good condition and had the roof and walls rehabbed about 5 years ago. Shed should be replaced in 5-10 years, but okay for at least 3 more years, Info Building in very good condition overall - the walls, roof, doors, celling, foundation Bullding in very good condition overall -the walls, roof, doors, ceiling, foundation, Structure is small shed used to house pump controls. Well is located NE of shed. Shed should be replaced in 3-5 years, but good for approx. 3 more years, info Source: Site Visit and Operator Interview double doors; cooler unit on concrete slab (4 SF by 4" thick); heater; removable skylight; Control Panel, RTU w/ant, various elec panels/endosures, flour, lights. Info Source: 1998 Leedhill Herkenhoff Record Drawings (As-bults), verified by Building constructed specifically to house pre-fab pump skid in 2007. It is in very REPURPOSED - pumps were removed from station in approx. 2012 and put into storage. Building is currently only used to house and operate the PRV that under pumps) with 16" wide footers on top of 4' of eng. fill; roll-up door; Metal Building is in good condition overall - the walls, roof, doors, ceiling, foundation, electrical, and HVAC are all okay. The AC Unit on the west side of the building. foundation, electrical, and AC unit are all okay. Info Source: 1998 Leedhill-Herkenhoff Record Drawings (As-builts), verified by electrical, and AC unit are all okay. Building Includes: 6" concrete slab (thicker Old building next to the Cook St. Infrastructure. Building looks to be about 40 The orig. Metal building was demo'ed and replace in 1998-99. Exist. building is in good condition overall - the walls, roof, doors, ceiling, good condition overall. Info Source: Site Visit and Operator Interview ears old, Info Source: Site Visit and Operator Interview NOTES_COMM needs to be replaced - priority item for Report, info Source: Site Visits and Pperator Interviews Chlorination Shed is in good condition. Info Source: Site Visits and Operator Interviews nfo Source: Site Visit and Operator Interview ource; Site Visit and Operator Interview or report Operator Value (Replacement Estimated Asse ASSET VALUE \$100,000 \$130,000 \$100,000 \$100,000 \$100,000 \$100,000 \$100,000 Value) \$10,000 RPC RHB CT Rehab Cost N/A Ϋ́ YR RPC RHB Year Replaced/ Rehabbed 2010 ۷ N ¥/× 1999 INIT COS nitial Year YR_INSTE 1940s 1996 1996 1974 2007 1945 1939 1999 1999 thickness FND THK (Inches) 12 T or C Water System Inventory: Buildings and Stuctures 4" Metal Skid on Concrete FND_MAT Concrete Concrete Concrete Concrete Concrete Concrete Concrete Wood frame with metal Wood frame with metal siding exterior Wood and metal Wood and metal Roof ROOF MAT Wood and metal Wood and metal Wood and metal iding exterlo Wood and metal Fiberglass Wood APPENDIX B Wall Width (Inches) WALL WD IN œ 00 00 Brick and Nood Frame/ Wood rame/stucco Wood frame with metal siding exterior WALL MAT Wall Material Brick with stucco exterior stucco CMC CMC ŝ CMD CMC Floor FEEF 4237.26 4237.26 4266.5 HGHT_FT Height (TOW) (Feet) 11.33 11,33 11.33 13 17 7 Width (Feet) 14.67 14,67 14.67 9 38 7 10.5 10,5 30 8 LNGTH FT Length (Feet) 18,67 24.67 24.67 8 œ 20 30 20 10.5 12.5 (Area) (Square Feet) interior Size SIZE_FT_SQ 1110 1400 30 120 20 480 180 252 336 336 Pershing St. Pressure Reducing Valve (PRV) Building Pump Station #2 Pump Building Well #2 Pump Control System Well #4 Pump Control System Cook St. Treatment Facility Pump Building Cook St. Treatment Facility Cook St. Treatment Facility Well #6 Pump Building Well #7 Pump Building Wel! #8 Pump Building Building Name/ Description Chlorination Shed Storage Building BLDG_DESC Shed Shed Approx, 380 feet NW of the ntersectin of S Broadway St. and Rio Grande Ave. South side of Cook St. about 600 feet from Cook St. Water Treatment Facility Approx. 235 feet due E very end (dead end) of Foster Ave. NW corner of City Utility Yard roperty approx 300 due west of SE corner of N Pershing St. and NE corner of the Intersection of Veater St. and Hyde Ave. Cook St. Water Treatment Facility Cook St. Water Treatment Facility Cook St. Water Treatment Facility Broad St. and Corona St. cocation Description W Eighth Ave. LOC_DESC Michigan St. 772282.6 774923,7 778200.0 771309.4 771116.1 770843.5 771774.1 Easting 772312.3 772322.0 771994.8 Northing 2890367.1 2890317.5 2890445.8 2897124,8 2901191.5 2889685,8 2887377.1 2889366.5 2890733.3 2888356.8 (GIS Point No./ID) Asset ID 윤 М m 'n 9 ^ 00

Structure in fair to poor condition. Does not protect equipment from weather or vandalism. Priority to install new Pre-fab Building to house centrols and manifold. Info Source: Site Visit and Operator Interview Structure is a small fiberglass dome housing for the Pre-fab pump system. It is In good condition, info Source: Site Visit and Operator Interview double doors; cooler unit on concrete slab (4 SF by 4" thick); heater; removable skylight; Control Panel, RTU w/ant, various elec panels/enclosures, flour. lights. nder pumps) with 16" wide footers on top of 4" of eng. fill; roll-up door; Metal electrical, and AC unit are all okay. Building includes: 6" concrete slab (thicker nfo Source: 1998 Leedhill-Herkenhoff Record Drawings (As-builts), verified by \$100,000 \$2,000 2006 1991 0.25 Fiberglass 분 Wood frame with metal siding top Fiberglass 0,25 Wood Posts ^ m 00 9 12 Clelo Vista Above Ground coster Pump System Housing Well #1 Shade Structure at the end of Camino De Cielo Southern portion of Cook St. Water Treatment Facility Cielo Vista Booster Pump System 772395,9 774763.9 2897423.8 2890438.6 6 19 Ξ

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	APPENDIX B	T or C Water System Inventory: Storage Tanks	Capacity/
			city/
			Capa
			Asset ID

ment/ Rehab Manufacturer Job No. (I) Cost NPG JOB_NO A RPC_RHB_C MFG JOB_NO A D&R Tank, Co. Job # 12-84520 Job # 03-50401 N/A D&R Tank, Co. Job # 03-50420 N/A D&R Tank, Co. Job # 03-50420	T or C Water Syst						APPEI T or C Water System In	APPEI T or C Water System In	AppEl Water System In	APPEI tem In	APPENDIX B	B ory: Storage Replace-	e Tanks		Estimated		
MFG 108_NO ASSET VANUE Thank is part of Cook St. Water Treatment Facility. Tank reabab in 2012 included; New went hatch and assembly, new access ladders with D&R Tank, Co. Job # 12-84501 Tank in excellent condition. Major Rebab of old tank - Sand Macual Catholic Forestion (internal and external) was added. Tank As builts 2012, WHPacific O&M Manual 2012; Site Visit and Operator Interview. Major Rebab of old tank - Sand Macual Catholic Forestion(intermal and external) was added. Tank As builts 2012, WHPacific O&M Manual 2012; Site Visit and Operator Interview. D&R Tank, Co. Job # 03-50401 Tank In excellent condition. Tank he excellent condition. Constructed in May 2004, Made of steel panels (sides and bottom) on concrete footing. Provides pressure for Upper Pressure Zone. No Purms and Catholic Potention(internal and external) was and bottom) on concrete footing. Provides pressure for Upper Pressure Zone. No Purms Sandon 2012; Site Visit and Operator Interview. Tank nexellent condition. Tank was ABANDONED in 2012, WH Pacific O&M Manual Globes Thessure Zone. Tank was ABANDONED in 2012 when other tanks were rehabbed. Tank was ABANDONED in 2012 when other tanks were rehabbed. Tanks was ABANDONED in 2012 when other tanks were rehabbed. Tanks was an asset of the Water's system due to historical and artists treatine.	(GIS Point No./ID)	Northing	Easting	Location/Description		Diameter (Feet)	Height (Feet)	Material	Year	Initial Cost	rear Replaced/ Rehabbed	ment/ Rehab Cost	Manufacturer	Job No.	Asset Value (Replacement Value)	Notes/Comments	<u> </u>
This condition This				10C_DESC	CAPTY_MG	L	НСНТ					RPC_RHB_C	MFG	JOB_NO	ASSET_VALUE		<u> </u>
2000 Conversion No. 124	l	2890340.7		Cook St. Treatment Facility	0.200	47	16.17	Steel	1997		2012		D&R Tank, Co.	Job # 12-84501	000'00E\$	Tank is part of Cook St. Water Treatment Facility. Tank rehab in 2012 included: New vent hatch and assembly; new access ladders with cages; Cathodic Protection (internal and external) was added. Tank in excellent condition. Info Sources: D&R Tank As-builts 2012; WHPacific O&M Manual 2012; Site Visit and Operator Interview.	
Total Brief	i	2899022.2		2900 Cemetery Rd (North Date St), west of I: 25		114	40	Steel	1978		2013		D&R Tank, Co.	Job # 12-84520	\$1,500,000	Major Rehab of old tank - Sand blasting and repriming/repainting of tank occurred early 2013 and was done by Luckinbill Enterprise, LLC out of Enid Oklahoma. New roof hatch, vent hatches, access ladders with cages and Cathodic Protection[internal and external) were added. Tank Provides pressure for Upper Pressure Zone. No Pump Station at tank. Tank in excellent condition. Info Sources: D&R Tank As-builts 2012; WHPacific O&M Manual 1012; Site Visit and Operator Interview.	<u> </u>
Production of new 1.2 Modern and Stand Corona St. Product	ĺ	2900954.2				73	40	Steel	2004	\$414,823	N/A	N/A	D&R Tank, Co.	Job # 03-50401	000'058\$	Constructed in May 2004, Made of steel panels (sides and bottom) on concrete footing, Provides pressure for Upper Pressure Zone. No Pump Station at tank. Tank in excellent condition. Info Sources: D&R Tank Submittals approved by Sullivan Design Group 2010; Site Visit and Operator Interview.	<u> </u>
West 2nd Ave. on top of Tank was ABANDONED in 2012 when other tanks were rehabbed. 775998.0 Chambers, Library, and Aver Dept. Building West 2nd Ave. on top of Tank was ABANDONED in 2012 when other tanks were rehabbed. NA Tank was ABANDONED in 2012 when other tanks were rehabbed. Aster Dept. Building Tank was ABANDONED in 2012 when other tanks were rehabbed. And Tank was ABANDONED in 2012 when other tanks were rehabbed.	1	2897153.3		Broad St. and Corona St., in between I-25 and the north part of City. Located next to Morgan Pump Station		102	20	Steel		\$643,392		,	D&R Tank, Co.	Job # 03-50420	\$1,500,000	Tank was built in May 2004 with the construction of new 1.2 MG tank (located on Cemetery Rd). Made of steel panels (sides and bottom) on concrete footing. Provides pressure for Lower Pressure Zone. Tank in excellent condition. Info Sources: D&R Tank Submittals approved by Sullivan Design Group 2010; Site Visit and Operator Interview.	
	l	2900674.8		West 2nd Ave. on top of hill south of Commission Chambers, Library, and Water Dept. Building				Steel	1948		N/A	N/A			NA	Tank was ABANDONED in 2012 when other tanks were rehabbed. Tank remains an asset of the Water System due to historical and artistic value.	

	Life Expectancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Useful Life Score Rounded	Field Visit Assessment Score	Operator Interview Score	Condition Score	Consequence Score	Risk Score	CIP Scheduled Year	Estimated Asset Replacement Value (as of 2015)	Current Value (Based on Remaining Useful Life as of 2015)
Pershing St. PRV Station Building	20	69	0	13.8	14	10	10	34	∞	272	2016	\$100,000	0\$
Cook St. Treatment Facility Pump No. 2 Motor	ω	82	0	22.5	23	ю	2	28	7	196	2016	\$15,000	0\$
Gas-Chlorination System	15	18	0	12.0	12	2	2	16	12	192	2016	\$140,000	0\$
Pershing PRV Station Manifold	35	69	0	19.7	20	2	-	23	8	184	2016	\$20,000	\$0
Well No. 2 Pump	15	69	0	46.0	46	9	4	56	3	168	2017	\$15,000	\$0
Cook St. Treatment Facility Pump Manifold	35	18	17	5.1	S	ε	5	10	14	140	2017	\$120,000	\$58,286
Well No. 7 Pump Motor	80	15	0	18.8	19	т	-	23	9	138	2017	\$7,000	0\$
Well No. 2 Pump Motor	8	29	0	36.3	36	4	м	43	£	129	2017	\$3,000	\$0
SCADA System RTUs	15	17	0	11.3	£	10	10	31	4	124	2017	\$110,000	0\$
SCADA System Software	15	13	2	8.7	ō	10	10	59	4	116	2017	\$35,000	\$4,667
Well No. 8 Pump	15	15	0	10.0	10	ю	10	23	r.	115	2017	\$25,000	\$0
Cook St. Treatment Facility Pump No. 1	15	18	0	12.0	12	m	-	16	7	112	2019	\$120,000	\$0
Cook St. Treatment Facility Pump No. 2	15	18	0	12.0	12	m	-	16	7	112	2020	\$120,000	\$0
Well No. 6 Pump Motor	8	15	0	18.8	19	21	, -	22	Ŋ	110	2020	\$5,500	\$0
Well No. 8 Pump Motor	ω	15	0	18.8	19	2	-	22	50	110	2020	\$5,500	0\$
Well No. 1 Pump System Shade Structure	20	23	27	4.6	Ŋ	10	10	25	4	100	2021	\$100,000	\$2,000
Gas-Chlorination System Fiberglass Shed	30	18	12	6.0	ω	N	0	ω	12	96	2016	\$10,000	\$4,000
Well No. 2 Pump Electrical System	15	39	0	26.0	26	4	-	31	ю	83	2022	\$35,000	0\$
Well No. 1 Pump Electrical System	15	23	0	15.3	15	Ŋ	ю	23	4	92	2021	\$30,000	\$0
Well No. 2	25	69	0	27.6	. 28			28	က	84	2022	\$25,000	0\$

APPENDIX B

Well No. 7 Pump 15 15 0 100 10 7 Well No. 4 Pump System Building 50 75 0 14.0 14.0 14 Well No. 4 Pump System Building 50 75 0 15.0 15.0 15 6 Well No. 1 Pump Manifold 25 38 0 15.2 15 6 Well No. 4 Pump Building 50 18 32 16 7 8 Well No. 4 Pump Electrical System 15 29 0 16.3 16 0 Well No. 4 Pump Electrical System 15 15 0 10.0 10 0 1 Well No. 5 Pump Manifold 35 35 0 10.0 10 0 1 0 1 0 1 1 0 1 0 1 0 1 0 1 0 1 0 0 1 0 1 0 1 0 1 1 0 1 <t< th=""><th>10.0</th><th></th><th></th><th></th><th></th><th></th><th></th><th>Oseiai Lile as Ol 2013)</th></t<>	10.0							Oseiai Lile as Ol 2013)
Building 50 70 0 140 14 Harman 50 75 0 150 15 Harman 55 23 12 6.6 7 Imp Building 50 18 32 6.6 7 Imp Building 50 18 32 15 16 cal System 15 29 0 16.3 16 7 cal System 15 0 10.0 10 10 10 cal System 15 0 10.0 10 10 10 cal System 15 0 10.0 10 10 10 cal System 15 0 10.0 10 10 10 10 cal System 15 0 10.0 10 10 10 10 10 10 cal System 15 15 0 10.0 10 10 10 10 10 <t< td=""><td>14.0</td><td>- 8</td><td>14</td><td>9</td><td>84</td><td>2023</td><td>\$30,000</td><td>\$0</td></t<>	14.0	- 8	14	9	84	2023	\$30,000	\$0
suilding 50 75 0 15.0 15 15 state 25 38 12 6.6 7 15 mp Euriding 25 38 0 15.2 15 15 cal System 15 29 0 10.3 16 10 cal System 15 15 0 10.0 10 10 as System 15 15 0 10.0 10 10 10 <t< td=""><td>15.0</td><td>7 7</td><td>28</td><td>ĸ</td><td>84</td><td>2024</td><td>\$100,000</td><td>\$0</td></t<>	15.0	7 7	28	ĸ	84	2024	\$100,000	\$0
4 25 28 0 15.2 15 15 mip Building 50 18 32 36 4 call System 15 29 0 16.3 16 1 28 8 0 16.3 16 2 13 0 16.3 16 3 25 56 0 10.0 10 4 35 35 0 10.0 10 4 35 35 0 10.0 10 4 35 35 0 10.0 10 4 35 35 0 10.0 10 4 35 35 0 10.0 10 4 35 35 0 10.0 10 6 8 9 10.0 10.0 10 6 15 0 10.0 10 10 6 15 0		Θ	27	3	81	2022	\$100,000	\$0
and System 15 25 38 0 0 15.2 15 15 15 15 15 15 15 15 15 15 15 15 15	9.9	ω	20	4	80	2021	\$10,000	\$3,429
mp Building 50 18 32 36 4 cal System 15 29 0 19.3 19 8 13 0 16.3 16 25 56 0 10.0 10 cal System 15 15 0 10.0 10 1 35 35 0 10.0 10 10 1 35 35 0 10.0 10 10 10 1 35 35 0 10.0 10<	15.2		15	5	75	2023	\$30,000	\$0
cal System 15 29 0 19.3 19 19. 8 13 0 16.3 16 16. 8 8 0 10.0 10. 10.0 10 10 10.0 10 10 10.0 10 10 10.0 10 10 10.0 10 10 10 10.0 10 10 10 10.0 10 10 10 10 10 10 10 10 10 10 10 10 10	3.6	1 0	ĸ	14	0.2		\$300,000	\$192,000
cal System 15 0 16.3 16 cal System 15 0 10.0 10 1 35 35 0 10.0 10 1 35 35 0 10.0 10 1 35 35 0 10.0 10 1 35 35 0 10.0 10 1 35 35 0 10.0 10 cal System 15 0 10.0 10 8 13 0 10.0 10 8 15 0 10.0 10 8 15 0 10.0 10 8 15 0 10.0 10 8 15 0 10.0 10 8 15 0 10.0 10 8 15 0 10.0 10 16 16.3 16 16	19.3	т	23	ю	69	2024	\$35,000	0\$
al System 15 56 0 10.0 10.0 10 al 35 56 0 22.4 22 al 35 35 0 10.0 10 al System 15 15 0 10.0 10 al 35 35 35 35 35 35 35 al 40 40 40 40 40 al 40 40 40 al 40 40 40 40 al 40 40 4	16.3	0	17	4	89	2021	\$3,000	0\$
cal System 25 56 0 22.4 22 4 35 15 0 10.0 10 1 15 15 0 10.0 10 .2 8 7 1 8.8 9 1 35 35 0 10.0 10 cal System 15 0 10.0 10 as System 15 0 10.0 10 B 13 0 16.3 16	10.0	5	17	4	89	2024	\$500	\$0
cal System 15 15 0 10.0 10 4 35 35 0 10.0 10 4 15 1 8.8 9 4 35 35 0 10.0 10 cal System 15 15 0 10.0 10 cal System 15 15 0 10.0 10 8 13 0 10.0 10 8 15 0 10.0 10 8 15 0 10.0 10 8 15 0 10.0 10 8 13 0 16.0 10	22.4		22	က	99	2024	\$25,000	\$0
1 15 15 0 10.0 10 1.2 8 7 1 8.8 9 1.3 35 0 10.0 10 10 cal System 15 15 0 10.0 10 10 cal System 15 15 0 10.0 10 10 cal System 15 15 0 10.0 10 10 10.0 10	10.0	0	F	9	99	2024	\$50,000	\$0
15 15 0 10.0 10 10 10 10 10 10 10 10 10 10 10 10 10	10.0	4 4	21	æ	63	2022	\$15,000	\$0
.2 8 7 1 8.8 9 1 35 35 0 10.0 10 cal System 15 15 0 10.0 10 cal System 15 15 0 10.0 10 8 13 0 16.3 16	10.0	-	12	S	09	2025	\$25,000	\$0
4 35 35 0 10.0 10 cal System 15 15 0 10.0 10 cal System 15 15 0 10.0 10 8 13 0 16.3 16	8.8	2	12	2	09	2025	\$5,500	\$688
cal System 15 15 0 10.0 10 10 cal System 15 15 0 10.0 10 10 10 10 10 10 10 10 10 10 10 10 10	10.0	9	20	Е	09	2024	\$15,000	0\$
Electrical System 15 15 0 10.0 10 Electrical System 15 15 0 10.0 10 Adoor 8 13 0 16.3 16	10.0	2 2	14	4	26	2025	\$500	0\$
Electrical System 15 15 0 10.0 10 Adoor 8 13 0 16.3 16	10.0	0	F	5	55	2025	\$50,000	0\$
8 13 0 16.3 16	10.0	1	11	S	22	2025	\$50,000	\$0
	16.3	0	17	8	51	2024	\$3,000	0\$
Booster Pump Station No. 2 Electrical 15 7 8 4.7 5 0 0 System	4.7	0 0	5	10	20	1	\$55,000	\$29,333
Booster Pump Station No. 2 Back-up 15 7 8 4.7 5 0 Generator Generator 15 7 6 0	4.7	0 0	ĸ	10	20	1	\$100,000	\$53,333

Asset	Life Expectancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Useful Life Score Rounded	Field Visit Assessment Score	Operator Interview Score	Condition Score	Consequence Score	Risk Score	CIP Scheduled Year	Estimated Asset Replacement Value (as of 2015)	Estimated Asset Current Value (Based on Remaining Useful Life as of 2015)
Well No. 7 Pump Manifold	35	23	12	9.9	7	0	0	7	9	42		\$22,500	\$7,714
Well No. 1	25	24	0	9.6	10			10	4	40	ı	\$25,000	\$1,000
Well No. 1 Pump	15	13	2	8.7	o.	0	-	10	4	40		\$18,000	\$2,400
Booster Pump Station No. 2 Pump No. 1	15	7	8	4.7	2	2	-	ω	5	40		\$25,000	\$13,333
Booster Pump Station No. 2 Pump No. 2	15	7 /	8	4.7	2	2	·	æ	5	40		\$25,000	\$13,333
Booster Pump Station No. 2 Pump Manifold	35	7	28	2.0	2	2	0	4	10	40	1	\$30,000	\$24,000
3.0 MG Storage Tank next to Booster Pump Station No. 2	50	10	40	2.0	2	0	0	2	20	40	1	\$1,500,000	\$1,200,000
Cielo Vista Pump Station Pump No. 1	15	80	7	5.3	2	2	2	6	4	36		\$1,000	\$467
Cielo Vista Pump Station Pump No. 2	15	ω	7	5.3	5	2	2	6	4	36	,	\$500	\$233
Cook St. Treatment Facility Pump No. 1 Motor	ε	⊽	ω	1.0	-	е	-	5	7	35		\$15,000	\$15,000
Well No. 6 Pump Manifold	32	23	12	9.9	7	0	0	7	5	35		\$22,500	\$7,714
Well No. 8 Pump Manifold	35	23	12	9.9	7	0	0	7	5	35		\$22,500	\$7,714
Well No. 4 Pump	15	13	2	8.7	თ	0	-	10	3	30	ī	\$15,000	\$2,000
Booster Pump Station No. 2 Pump No. 2 Motor	80	-	7	1.3	-	0	5	9	5	30	ı	\$5,500	0\$
Well No. 8	25	15	10	6.0	Q	- ()		9	2	30	1	\$35,000	\$14,000
Cook St. Treat. Facility Storage Building	50	40	10	8.0	80	4	က	15	2	30	,	\$100,000	\$20,000
1.2 MG Storage Tank on Cemetery Rd.	50	10	40	2.0	2	0	0	2	15	30	3	\$850,000	\$680,000
Cook St. Treatment Facility Electrical System	15	-	14	1.0	-	-	0	2	14	28	1	\$120,000	\$112,000
Gielo Vista Pump Station Electrical System	15	ω	7	5.3	5	-	0	9	4	24	,	\$5,000	\$2,333
Cielo Vista Pump Station Manifold	35	7	28	2.0	2	တ	-	9	4	24	,	\$2,500	\$2,000
Cook St. Treatment Facility Flow Meter	12	18	0	15.0	15	4	4	23	1	23	16	\$20,000	0\$

APPENDIX B

Asset	Life Expectancy (Years)	Age (Years)	Estimated Remaining Useful Life (Years)	Useful Life Score	Useful Life Score Rounded	Field Visit Assessment Score	Operator Interview Score	Condition Score	Consequence Score	Risk Score	CIP Scheduled Year	Estimated Asset Replacement Value (as of 2015)	Estimated Asset Current Value (Based on Remaining Useful Life as of 2015)
Cielo Vista Pump System Fiberglass Housing	30	8	22	2.7	3	2	0	5	4	20	1	\$2,000	\$1,467
0.2 MG Storage Tank at Cook St. Facility	50	2	48	1.0	1	0	0	-	20	20	ı	\$300,000	\$288,000
Well No. 7 Pump System Building	20	15	35	3.0	ဧ	0	0	3	9	18	1	\$100,000	\$70,000
3.0 MG Storage Tank on Cemetery Rd.	50	Ť	49	1.0	-	0	0	1	18	18	-	\$1,500,000	\$1,470,000
Well No. 2 Flow Meter	12	14	0	11.7	12	2	-	15	1	15	_	\$3,000	\$0
Well No. 6 Pump System Building	50	15	35	3.0	က	0	0	3	2	15	-	\$100,000	\$70,000
Well No. 8 Pump System Building	20	15	35	3.0	ဇ	0	0	3	2	15	-	\$100,000	\$70,000
Well No. 7 Flow Meter	12	15	0	12.5	13	-	0	14	1	14	-	\$5,000	\$0
Well No. 8 Flow Meter	12	15	0	12.5	13	•	0	14	1	14	-	\$5,000	\$0
Well No. 6 Flow Meter	12	15	0	12.5	13	0	0	13	1	13	E.	\$5,000	0\$
Well No. 4 Flow Meter	12	10	2	8.3	ω	ю	-	12	1	12	1	\$3,000	\$500
Booster Pump Station No. 2 Building	50	7	43	1.4		0	0	ŀ	10	10	-	\$130,000	\$111,800
Booster Pump Station No. 2 Flow Meter	12	7	S	5.8	9	0	0	9	1	9		\$10,000	\$4,167
Well No. 7	25	⊽	25	1.0				1	9	9		\$35,000	\$35,000
Well No. 1 Flow Meter	12	₽	12	1.0	i.	0	0	•	1	-	ï	\$3,000	\$3,000
NEW ITEM- New 0.25 MG Storage/ Transfer Tank at Cook St. Treatment Facility	25 2									NA	2016	\$400,000	ΝΑ
NEW ITEM- Cook St. Treatment Facility Back-up Generator										NA	2018	\$220,000	NA

APPENDIX C

Water System Historical Financial Data, Existing Loan Information, and Energy Use Costs

			-		- TANK		ALL LANDLY C. Match Distant Line Lent Anstorner Land Land						
	FISCAL YEAR	20	2009/10	70	2010/11	20	2011/12		2012/13	75	2013/14		Five-Year Averages (Typical)
	Revenue from V Service Connect		\$ 974,560 \$ 1,028,892	\$ 1	,028,892	40.	\$ 282,676	\$	966,738 \$ 908,993	45	908,993	40	971,693
OPERA SVEVE	Revenue from Other Water System Services	45	19,767	\$	25,839	\$	22,597	₩.	\$ 67,798	···	58,914 \$	\$	38,983
	TOTAL REVENUE		\$ 994,327		\$ 1,054,731 \$ 1,001,879 \$ 1,034,536	\$ 1	628,100,	\$ 1	,034,536	\$	\$ 967,907	\$	\$ 1,010,676
S	Personnel/Administration	\$	416,414	s	411,944	\$	362,624	\$	380,285	\$	421,839	\$	398,621
ENS	Utility	\$	138,164	S	138,057	\$	148,429	\$	147,269	\$	\$ 262,681	s	142,343
EXP	Supplies/Laboratory	\$	10,926	\$	7,493	\$	11,770	❖	8,124	\$	10,762	\$	9,815
ОРЕВАТІИС	Maintenance and Repair or Replacement of System Components/Equipment and Vehicles	\$	153,839	₩.	120,073	φ.	188,590	<>>	188,590 \$ 131,799 \$ 171,123 \$	\$	171,123	₩.	153,085
	TOTAL OPERATING EXPENSES \$ 719,343	\$	719,343	\$	595,779	\$	711,412	\$	667,477	\$	\$ 743,518	*	707,469
	NET INCOME (LOSS) \$ 274,984 \$	S	274,984	\$	377,166 \$		290,467 \$ 367,059 \$ 224,389 \$	\$	367,059	5	224,389	\$	293,972

[1] Compiled from Revenue and Expenses data reported by the City of T or C for FY 2009/10 -FY 2013/14

Hppen	OIX C - LUV. J. Frank Gl		
TORC 2	WATER TANK LOAN	Year that Loan Torm Ends	
Principal Interest Admin Fee	6103905 \$101,205.86 6103910 \$ 17,173.42 6103915 \$ 1.862.30 \$120,241.58	2021	
TORC 17	WTB-229		
Principal Interest	6103905 \$ 12,593.00 6103910 \$ 546.04 \$ 13,139.04 } Annual Payment Amount	2031	
TORC 18 Principal	#2613-PP GROUND STORAGE TANKS 6103905 \$ 6,905.84 Annual Paymont Amount	2032	
TORC 19	REFINANCED 95,96,98 UTIL BONDS		partion of pay-
Principal Interest	6103905 \$ 47,746.66 6103910 \$ 28,241.14 \$ 75,987.80 \ Z Annual Paymont Amount	2033	(some partial of pay- mont hardled in www system Budget
TORE 22	WTB-292		
Principal Interest	6103910 \$ 3,224.00 6103910 \$ 156.62 \$ (3,38062) \(\text{Annucl Payment Amount}	2033	
Loan 94-10			
Principal Interest	6103905 \$ 30,344.26 6103910 \$ 3,808.47 \$ 34,152.73 Annual Paymen + Amount	2017	
Loan 95-16	POTABLE WATER SYSTEM	•	
Principal Interest	6103905 \$ 26,768.22 6103910 \$ 7.140.96 \$ 33,909.18	2022	

APPENDIX C

Water System Major Pump Systems- Current Energy Use and Power Costs

Asset	Pump Motor Total (Rated) Horse Power (HP)	Pump Motor Actual Horse Power (at Design Point) (HP)	Pump Motor Actual Power (at Design Point) (Kw)	Total Daily Equipment Use (Hours)	Total Power Usage (Kw-Hr)	Daily Power Cost (at \$0.108/Kw-Hr)	Pump Capacity (at Design Point) (GPM)	Total Gallons Pumped from Wells (Production)	Total Gallons Pumped from Cook St. Facility to Lower 3.0 MG Tank	Total Gallons Pumped from Booster Pump Station No. 2 to Upper 1.2 MG and 3.0 MG Tanks
Well No. 1 Pump	30	24	17,9	8,00	143.2	\$15.46	250	120000		
Well No. 2 Pump.	25	20	14.9	8,00	119.3	\$12.89	400	192000		
Well No. 4 Pump	30	24	17.9	8.00	143.2	\$15.46	225	108000		
Well No. 6 Pump	09	49	36.5	8:00	292,3	\$31.57	200	240000		
Well No. 7 Pump	75	09	44.7	8.00	357.9	\$38.66	029	312000		
Well No. 8 Pump	09	49	36.5	8.00	292.3	\$31.57	200	240000		
Cook St. Treatment Facility Pump No. 1	250	225	167.8	3,35	562,1	\$60.70	3000		000009	
Cook St. Treatment Facility Pump No. 2	250	225	167.8	3,35	562.1	\$60.70	3000		000009	
Booster Pump Station No. 2 Pump No. 1	09	42	31.3	10.00	313.2	\$33.82	009			360000
Booster Pump Station No. 2 Pump No. 2	09	42	31.3	10.00	313.2	\$33.82	009		,	360000
				Daily Totals	3098.8	\$334.67		1212000	1206000	720000

The 0.2 MG Tank at Cook St is pump out entirely 6 times per day

Assuming an average of 1.2 MG/day of Production for Upper Zone assumed to use 60% of the Municipal Water System (based on WHPacific 40-Year Water Plan)

\$122,152.98

1131046.1

Annual Totals

APPENDIX D

WATER METERING REQUIREMENTS

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City of Truth or Consequences

505 Sims Street

Truth or Consequences, New Mexico 87901

City (575) 894-6673 Fax (575) 894-0363

March 7, 2017

Re: Water Users and Diversions

As written and adopted in the City of Truth or Consequences Municipal Code

Sec. 14-38. - Water rates - Generally (b) Meters Required:

herefore attached and made a part of this letter,

let it be known that the Governing Body of the City of Truth or Consequences shall require consumers to install water meters with the exception of services installed for fire suppression.

Sincerely,

Juan A. (Fuentes City Manager) Sec. 14-38. - Water rates-Generally.

(a) Rates to residential and commercial customers within the City limits and the Village of Williamsburg. The rates for water are as follows:

Customer charge:	\$8.15 per month
Usage charge:	
0—7,000 gallons:	\$1.75 per each 1,000 gallons
Next 22,000 gallons:	1.93 per each 1,000 gallons
Next 20,000 gallons:	2.12 per each 1,000 gallons
Over 50,000 gallons:	2.33 per each 1,000 gallons

- (b) Meters required. The Governing Body shall require consumers to install water meters with the exception of services installed for fire suppression.
- (c) Shortage of water. When there is a shortage of water supply of the City, the Governing Body reserves the right to control the manner of use and time of use by consumers of water supplied to them by the system, and to make such rules and regulations therefore by proclamation of the Governing Body. Any consumer taking or using water contrary to such regulations and rules, as may be promulgated in such proclamation, shall be deemed guilty of a misdemeanor.
- (d) Rates to consumers outside the City limits. Rates charged to consumers outside the City limits are the same as those rates set forth in section 14-38(a), Water rates—Generally.
 - (1) Reference: As provided in section 14-28(b) Service—Applications; extension: All requests and proposals to supply water to consumers outside the City limits and all proposed water system plans, designs, and specifications to deliver water beyond the City limits require prior approval by the City Commission.
 - (2) Reference: As provided in section 14-28(a) Service—Applications; extension: Applicants who wish to have water supplied beyond the City limits shall be responsible for "payment for necessary line extensions."

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The City reserves the right to deny applications and requests to supply water beyond the corporate limits of the City when in the opinion of the City Manager and by vote of the City Commission such requests would impair the supply of water to existing customers located within the corporate limits of the City.

- (e) Industrial fire plug meter. The minimum monthly charge shall be \$91.91, which shall entitle the consumer up to 50,000 gallons per month. The second 50,000 gallons consumed shall be charged at the rate of \$1.84 per 1,000 gallons. The third 50,000 gallons consumed shall be charged at the rate of \$2.02 per 1,000 gallons. All water consumed in excess of 200,000 gallons per month shall be charged at the rate of \$2.22 per 1,000 gallons.
- (f) Effluent water/recycled water. The charge shall be \$1.35 per 1,000 gallons. If a meter is used, the minimum monthly charge shall be \$25.00 plus actual usage.

(Code 1962, § 3-3-12; Ord. No. 371, 5-14-90; Ord. No. 387, 11-12-91; Ord. No. 405, 2-8-93; Ord. No. 411, 1-10-94, Ord. No. 459, § 1, 7-13-98; Ord. No. 475, § 1, 7-24-00; Ord. No. 491, § 1, 3-26-01; Ord. No. 497, §§ 1, 2, 7-9-01; Ord. No. 521, § 1, 10-16-02; Ord. No. 526, § 1, 5-27-03; Ord. No. 530, § 1, 8-25-03; Ord. No. 572, 6-20-07)

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APPENDIX 12- UTILITY RATES

CONTENT:

o UTILITY RATES TABLE



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OLD WATER RATES PRIOR JULY 1,2020

Residential Rates – City of T or C & Village of Williamsburg	
Base Customer Charge (Minimum)	\$8.15
Rate per 1000 gallons for Level 1 Usage (1 - 7,000 gallons)	\$1.75
Rate per 1000 gallons for Level 2 Usage (7,000 - 30,000 gallons)	\$1.93
Rate per 1000 gallons for Level 3 Usage (30,000 – 50,000 gallons)	\$2.12
Rate per 1000 gallons for Level 4 Usage (Above 50,000 gallons)	\$2.33
Commercial Rates – City of T or C & Village of Williamsburg	
Base Customer Charge (Minimum)	\$8.15
Rate per 1000 gallons for Level 1 Usage (1 - 7,000 gallons)	\$1.75
Rate per 1000 gallons for Level 2 Usage (7,000 - 30,000 gallons)	\$1.93
Rate per 1000 gallons for Level 3 Usage (30,000 – 50,000 gallons)	
Rate per 1000 gallons for Level 4 Usage (Above 50,000 gallons)	\$2.33
Industrial Rates – City of T or C	
Base Customer Charge (Minimum and for Usage 1 – 50,000 gallons)	\$91.91
Rate per 1000 gallons for Level 2 Usage (50,001 – 100,000 gallons)	\$1.84
Rate per 1000 gallons for Level 3 Usage (100,001 – 150,000 gallons)	\$2.02
Rate per 1000 gallons for Level 4 Usage (Above 150,000 gallons)	\$2.22



NEW WATER RATES EFFECTIVE JULY 1,2020

Res	idential Rates - City of T or C & Village of Williamsburg
15.5	Base Customer Charge (Minimum)
2.71	per 1,000 gallons for first 7,000 gallons
3.07	per 1,000 gallons from 7001 gallons to 29,000 gallons
3.45	per 1,000 gallons from 29,001 gallons to 50,000 gallons
3.88	per 1,000 gallons for amount over 50,000 gallons
Com	mercial Rates - City of T or C & Village of Williamsburg
15.5	Base Customer Charge (Minimum)
2.71	per 1,000 gallons for first 7,000 gallons
3.07	per 1,000 gallons from 7001 gallons to 29,000 gallons
3.45	per 1,000 gallons from 29,001 gallons to 50,000 gallons
3.88	per 1,000 gallons for amount over 50,000 gallons
	Industrial Rates – City of T or C
91.91	Customer Charge for first 50,000 gallons
3.07	per 1,000 gallons from 50,001 gallons to 100,000 gallons
3.45	per 1,000 gallons from 100,001 gallons to 150,000 gallons
3.88	per 1,000 gallons for amount over 150,000 gallons
	Effluent Water
100	Deposit
\$1.35 pe	er 1,000 gallons + \$25.00 if used

The NEW water rates will affect the City's revenue from July 1, 2020.

